DOWNOWN FOR EVERYONE

A REIMAGINED DOWNTOWN BLOOMINGTON EXPERIENCE

STREETSCAPE PROGRAM REPORT VOLUME 3

Adopted April 8th, 2024

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Core Team Meeting Minutes



KICKOFF MEETING September 28, 2022 – 11:00 AM



MEETING MINUTES

1) **OPENING REMARKS**

2) COUNCIL RECAP / FINAL SCOPE DISCUSSION

- Noted that despite some objections on contract technicalities, Council appears supportive of investment Downtown. Appears to be significant community support for Downtown improvements as well.

- City Admin (Tim/Billy) will lead efforts to keep Council informed/involved, supported by CMT Team.

1. CRITICAL SUCCESS FACTORS DISCUSSION

- Admin validates program approach: (1) Focus on infrastructure modifications to prioritize "walkability" Downtown, not merely a cosmetic program; (2) Everything is on the table going from building face to building face; (3) Acquisitions not preferred unless a unique opportunity presents itself (such as adjacent private surface parking lots); and (4) No specific design element preferences at this time – would like the process to play itself out and see what develops out of it.

- Admin verifies approach to public involvement: public will be informed and consulted at various points during life of program design via public meetings and use of City's Communications Manager. Team will meet at least quarterly with "Steering Committee" including community leaders and cross-section of downtown users.

3) SCHEDULE DISCUSSION

- 12-month design schedule. CMT authorized to engage subs.

- Core Team meetings to be scheduled every two weeks starting Tuesday 10/11/22 at 9:00am.

4) IMPLEMENTATION PLAN UPDATE:

- a) City To Do List:
 - i) Define CORE team members (to meet bi-weekly) CoB has already identified Craig and Billy as City Core Team members with other City staff brought in to meetings as needed.
 - ii) Define STEERING committee team member (to meet quarterly) Craig, Billy, and Tim to assemble this team.
 - iii) Define Community Stakeholder team members (to meet quarterly) To simplify things, there will not be a separate Community Stakeholder Team. The Steering Committee will include representatives of the community.
 - iv) Schedule meeting with County regarding courthouse square **Billy will set something up with the County and then let CMT know when a meeting can occur.**
- b) CMT To Do List:

- i) Schedule bi-monthly meetings (with City) Mike Sewell to send out Outlook invites.
- ii) Schedule internal kickoff meetings (traffic, funding, etc.)
- iii) Obtain and review City GIS data determine need for supplemental information (videos, etc.) Craig to provide Mike with Troy and Craig's availability.

5) CRITICAL CITY DECISION POINTS:

- a) Provide "ok" to start traffic counts (commence immediately) City approval given.
- b) Provide "ok" to start pavement cores (wait until traffic counts are complete) City approval given.

6) ONGOING TASK ITEMS:

- a) CMT/MMA Develop detailed project schedule and QA plan
- b) CMT/MMA Review existing studies / reports
- c) CMT/MMA Data collection (utilities, GIS data, aerial imagery) CMT has shared propose traffic data collection plan with City Traffic Engineer.
- d) CMT Developing Existing Traffic Model

7) UPCOMING TASK ITEMS:

- a) CMT Schedule 1st quarterly meeting with Steering Committee / Core teams (vision & goals session)
- b) CMT Schedule onsite investigation & recording trip
- c) City Define community stakeholders
- d) City Develop external communication plan (stakeholders, council, staff, etc.) CMT to add standing agenda item to Core Team meeings: "STORIES", intended to provide ongoing, compelling content to public to keep them in the loop.

8) COMPLETED TASK ITEMS:

- a) CMT Executed sub agreements with MMA & Workbench
- b) City Executed agreement with CMT & provided NTP

9) OPEN DISCUSSION / QUESTIONS





CORE TEAM MEETING October 10, 2022 – 10:00 AM

MEETING MINUTES

1) DISCUSSION ITEMS:

a) Project "Stories" Approach – Katherine Murphy and CMT Team Intro - Katherine will now attend every Core Team Meeting.

- Katherine and Mike will work together on publishing stories: Katherine will provide suggestions on format/frequency/etc and Mike will keep "ear to the ground" on content.

- b) Traffic Counts Status Update for Katherine
- c) Illinois Main Street Conference CoB Involvement
 - Kent Massie and C.J. Baker will attend from CMT Team. Billy to let CMT know if any sort of content/participation is needed, but unlikely.
- d) Project Schedule Review Milestones
 - See attached schedule.

- Notable milestones: City to deliver GIS info to CMT by 10/21. Steering Committee Meeting #1 tentatively scheduled for week before Thanksgiving. City to deliver "critical utilities" data by Thanksgiving.

e) GIS Data – Next Steps

2) IMPLEMENTATION PLAN UPDATE:

- a) City To Do List:
 - City to provide CMT with downtown maps for vaults and steam tunnels.
 - i) Define STEERING committee team member (to meet quarterly)
 - ii) Schedule meeting with County regarding courthouse square
 Meeting has been scheduled for 10/27.
 - iii) Develop first wave of "easy" data to be shared from GIS and share availability for meeting with CMT Team
- b) CMT To Do List:
 - CMT to prepare less detailed schedule for general use.
 - i) See Ongoing Task Items below...

3) CRITICAL CITY DECISION POINTS:

a) None at this time...

4) ONGOING TASK ITEMS:

- a) CMT/MMA Develop QA plan
- b) CMT/MMA Review existing studies / reports
- c) CMT/MMA Data collection (utilities, GIS data, aerial imagery)
 Traffic counts to be collected 10/25.
- d) CMT Developing Existing Traffic Model
- 5) UPCOMING TASK ITEMS:
 - a) CMT Schedule 1st quarterly meeting with Steering Committee / Core teams (vision & goals session)
 - b) CMT Schedule onsite investigation & recording trip
 - Data collection beginning week of 10/17.

6) COMPLETED TASK ITEMS:

- a) CMT Developed Project Schedule
- 7) "STORIES" TO TELL
- 8) OPEN DISCUSSION / QUESTIONS

ID	0	Task Mode	Task Name	Duration	Start	Finish	Predecessors	22 Sep 4, '22	Sep 11, '22	Sep 18, '22	Sep 25, '22 0	ct 2, '22 C	Oct 9, '22	Oct 16, '22 C	Ct 23, '22	Dct 30, '22 Nov	6, '22	Nov 13, '22	Iov 20, '22 Nov	27, '22 D	ec 4, '22 M T W T F S	Dec 11, '22	Dec 18, '22	Dec 25, '22								
1		*	Overall Program Design	262 days?	Thu 9/1/22	Fri 9/1/23																										
2		->	Task #A - PROGRAM	262 days	Thu 9/1/22	Fri 9/1/23		-	-	-	-		-	-	-		-	-		-	-	-	-									
3	-	-4	Bi-Monthly	262 days	Thu 9/1/22	Fri 9/1/23			-	_	_		_	_	_	_	_	_	_	-	_	_	-	_								
4			Meetings Ouarterly	0 days	Tue 11/1/22	Tue 11/1/22		_								♠ 11/1																
		->	Steering Committee Meeting #1	U days	100 11/1/22	100 11/1/22										•																
5		•	Quarterly Steering Committee Meeting #2	0 days	Tue 2/7/23	Tue 2/7/23																										
6		-	Quarterly Steering Committee Meeting #3	0 days	Tue 5/2/23	Tue 5/2/23																										
7		•	Quarterly Steering Committee Meeting #4	0 days	Tue 8/1/23	Tue 8/1/23		_																								
8		-	TASK #B - DATA COLLECTION	35 days	Mon 10/3/22	Fri 11/18/22							-																			
9		•	Subtask #B.1 - Analyze Data from Previous Studies & Reports	15 days	Mon 10/10/22	Fri 10/28/22																										
10		-	Subtask #B.3 - Coordinate City GIS Data & Aerial Imagery	15 days	Mon 10/3/22	Fri 10/21/22																										
11		-,	Subtask #B.2 - Review Available Record Plans & Drawings	20 days	Mon 10/24/22	Fri 11/18/22	10																									
12		•	Subtask #B.4 - Set Up Base Planning Drawings	20 days	Mon 10/24/22	Fri 11/18/22	10,18							1																		
13		•	Subtask #B.5 - On-Site Investigation & Recording	25 days	Mon 10/17/22	Fri 11/18/22																										
14	i	-	Subtask #B.6 - ADA Compliance Review	25 days	Mon 10/17/22	Fri 11/18/22																										
15		->	TASK #C - UTILITY COORDINATION	88 days	Thu 9/1/22	Mon 1/2/23							-				-	-	_		-											
															-			•														
Proj	ect: Do	wntown 0/8/22	CoB Streets Split			Summary Project Summ	ary	Inactive M	Vilestone 🔶 Summary 🛙		Duration-only Manual Summary Ro	ollup	Sta Fin	art-only iish-only	с Э	External Mil Deadline	estone	♥ ♣	Manual Progre	is in the second se												
Jat		-, 0, 22	Milestone	•		Inactive Task		Manual T	ask	N	Manual Summary	-	Ext	ernal Tasks		Progress			-													
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ID	A	Task Mode	Task Name	Duration	Start	Finish Predecessors	2 Sep 4, '22 Sep 11, '22 Sep 18, '22 Sep 18, '22 Sep 25, '22 Oct,
16		-	Subtask #C.1 - Confirm & Analyze Existing City Utility Data	19 days	Tue 10/25/22	Fri 11/18/22 10	
17		-	Subtask #C.2 - Confirm & Analyze Vaults/Stair Wells Data	25 days	Mon 10/17/22	Fri 11/18/22	
18	CIE.	-	Subtask #C.3 - Coordinate with Various Utility Companies	37 days	Thu 9/1/22	Fri 10/21/22	
19		-	Subtask #C.4 - Perform CCTV Televising of Sewers	50 days	Tue 10/25/22	Mon 1/2/23 10	
20		-	Subtask #C.5 - Drainage Assessment	19 days	Tue 10/25/22	Fri 11/18/22 10	
21		-	City Delivers List of "Critical" Utilities	0 days	Wed 11/23/22	Wed 11/23/22	<u>♦ 11/23</u>
22		-	Subtask #C.6 - Utility Risk Assessment	25 days	Mon 11/28/22	Fri 12/30/22 21	
23		-	TASK #D - TRAFFIC ANALYSIS	158 days?	Mon 10/24/22	Wed 5/31/23	
24		-	Subtask #D.1 - Analyze Existing Conditions Including Vehicle Traffic Counts	20 days	Mon 10/24/22	Fri 11/18/22	
25	i:	÷	Subtask #D.2 - Analyze Data from Parking Analysis Study Done by Others	15 days	Mon 10/31/22	Fri 11/18/22	
26		÷	Subtask #D.3 - Model Traffic Alternatives	138 days?	Mon 11/21/22	Wed 5/31/23	
27		÷	Subtask #D.4 - Traffic Meetings with City Traffic Engineer	138 days?	Mon 11/21/22	Wed 5/31/23	
28		->	Subtask #D.5 - Bicycle Analysis	138 days?	Mon 11/21/22	Wed 5/31/23	
29		-	Subtask #D.6 - Pedestrian Analysis	138 days?	Mon 11/21/22	Wed 5/31/23	
			Task			Summary	Inactive Milestone 👌 Duration-only
Proje Date:	ct: Do Sat 10	wntown 0/8/22	LOB Streets Split			Project Summary	Inactive Summary Manual Summary Rollup Finish-only Deadline
\vdash			Milestone	•	7	Inactive Task	Manual lask Manual Summary External Tasks Progress

ID	0	Task Mode	Task Name	Duration	Start	Finish	Predecessors 22	Sep 4, '22	Sep 11, '22 Sep) 18, '22	Sep 25, '22 Oct 2, '	22 Oct 9, '22	Oct 16, '22	Oct 23, '22 Oc	ct 30, '22 Nov 6, '22	Nov 13, '22	Nov 20, '22 Nov 27, '22	Dec 4, '22	Dec 11, '22 De	:18,'22 Dec.	25, '22 Ja
30		-	TASK #E - ASSISTANCE WITH PUBLIC COORDINATION	138 days?	Mon 11/21/22	Wed 5/31/23															
31	ö	-	TASK #F - US 51B COORDINATION	138 days?	Mon 11/21/22	Wed 5/31/23											_	-	-	-	_
32		-	TASK #G - PAVEMENT ANALYSIS	55 days?	Mon 10/17/22	Fri 12/30/22							-	-	-	-		-		-	-
33		-	TASK #H - CONCEPT	138 days?	Mon 11/21/22	Wed 5/31/23											_	-		-	_
34		-,	DEVELOPMENT TASK #I - PREPARATION OF "STANDARDS FOR DOWNTOWN STREETSCAPE" DOCUMENT	67 days?	Thu 6/1/23	Fri 9/1/23															
35		-	TASK #J - FUNDING OPPORTUNITIES	1 day?	Fri 9/1/23	Fri 9/1/23															
36		•	TASK #K - HISTORICAL CLEARANCES & COMMUNITY RESEARCH	138 days?	Mon 11/21/22	Wed 5/31/23												Г			T
37		-,	TASK #L - PROGRAM COST OPINION	67 days?	Thu 6/1/23	Fri 9/1/23															
38		-	TASK #M - PROGRAM PHASING ANALYSIS	67 days?	Thu 6/1/23	Fri 9/1/23															
39	iii	-	TASK #N - MISCELLANEOUS TASKS	138 days?	Mon 11/21/22	Wed 5/31/23												-	-	-	
40		->	TASK #O - QUALITY ASSURANCE	1 day?	Fri 9/1/23	Fri 9/1/23															
41		-	TASK #P - ADMINISTRATION MANAGEMENT	262 days	Thu 9/1/22	Fri 9/1/23		-	-				-		_				-	-	
42		-	TASK #Q - FINAL "DOWNTOWN STREETSCAPE PROJECT - CONCEPT DESIGN" DELIVERABLE	67 days?	Thu 6/1/23	Fri 9/1/23															
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Proj Date	ect: Do ect: Sat 1	wntown C)/8/22	oB Streets Split			Summary Project Summa	ry I	Inactive M Inactive Su	ilestone 🔶 Immary I		Duration-only Manual Summary Rollup		Start-only Finish-only	с э	External Milestone Deadline	♦	Manual Progress		-		
			Milestone	•		Inactive Task		Manual Ta	sk		Manual Summary	Page 2	External Tasks		Progress		-				





CORE TEAM MEETING October 25, 2022 – 9:00 AM

MEETING MINUTES

1) DISCUSSION ITEMS:

- a) Program Funding Opportunities Approach Ally Fields (CMT Funding Specialist) and Melissa Hon (CoB Economic and Community Development Director)
 - i) Current Funding Opportunities Identified by Ally
 - ii) Existing City Approach to Pursuing Funding Opportunities
 - iii) Strategy for Integrating Program Elements into City's Ongoing Pursuits of Funding

 Ally directed to look for grants/funding opportunities that are good candidates for the
 Streetscaping Program and let CoB know when they're coming up so that they can prepare for them.
- b) Illinois Main Street Conference Quick Recap from Kent
- c) County Meeting Scheduled for 10/27. Minimal prep likely required.
 Core Team subsequently met with County and had productive conversations. Next step is for City, Mike, and Museum to meet with Cassy Taylor (McLean County) likely the week of 11/14 to introduce some concepts to the County for consideration.

2) IMPLEMENTATION PLAN UPDATE:

- a) City To Do List:
 - i) Define STEERING committee team member (to meet ~ quarterly).
 Billy believes composition of committee will be finalized by 10/28/22.
 - ii) Provide CMT with Downtown vault and steam tunnel maps.
 CoB has now provided CMT with this information.
 - Develop first wave of "easy" data to be shared from GIS and share availability for meeting with CMT Team – by 10/21.

- Mike directed by Craig to reach out directly to Troy Olson. Mike and others from CMT subsequently met with Troy and Joe Palma and have begun receiving access to data.

- b) CMT To Do List:
 - i) Prepare less detailed schedule for general use.
 - ii) Develop QA Plan.
 - iii) Data collection (utilities, GIS data, aerial imagery) ongoing.
 - iv) Develop existing traffic model ongoing. Counts occurring 10/25/22.
 Counts have now been completed.

3) CRITICAL CITY DECISION POINTS:

a) None at this time ...

4) UPCOMING TASK ITEMS:

- a) City Tasks:
 - i) Deliver "critical utilities" data by Thanksgiving.
- b) CMT Tasks:

- CMT to draft a first list of upcoming grants/funding opportunities for CoB to review. This will be a "living document", updated as warranted by Ally and her team who have their ear to the ground for us.

- i) Schedule 1st quarterly meeting with Steering Committee (tentatively week before Thanksgiving)
- ii) Develop base map

5) COMPLETED TASK ITEMS:

a) CMT – Developed Project Schedule

6) "STORIES" TO TELL

- a) How often? What format?
- b) Traffic modeling "story". What content needed from CMT Team?

- Katherine will take lead on determining frequency and format of messaging, now that she is attending all Core Team meetings and since she will now be in the loop on what elements of the design are being focused on. CMT will assist with providing content.

7) OPEN DISCUSSION / QUESTIONS

- Discussion on next steps needed to allow Design Team to begin preparing concepts. Consensus was that Design Team needs some additional input first, particularly from the Steering Committee once established.





CORE TEAM MEETING November 8, 2022 – 9:00 AM

MEETING MINUTES

1) DISCUSSION ITEMS:

- a) US-51B Walkability Strategy Phil Allyn (CoB Traffic Engineer) and Roger Driskell (CMT Surface Transportation Director) in attendance
 - Recap by Craig Shonkwiler on conversations recently had with IDOT and Hutchison
 Craig recounts that IDOT is giving consideration to outside bike lane but perhaps skeptical on further lane reductions due to volume of traffic.
 - ii) Review of Massie sketches and general discussion on walkability options
 - iii) Discuss strategy for engaging IDOT, next steps, etc.
 - Kent and Roger discuss possibility of speed reduction either by City Ordinance (change speed limit) or perhaps more effectively by implementing traffic calming such as lane reduction.
 Phil notes that City has an adopted Bicycle Master Plan that includes bike lanes on US-51B and advises we not do anything to take that off the board.

- Discussion amongst team that walkability must also be emphasized, particularly ability to safely cross US-51B.

- Next Steps: Roger to reach out informally to his Hutchison contacts to find out status of Phase I, prior to engaging IDOT. Depending on what we learn, likely set up meeting for CMT and IDOT to develop specific strategy for IDOT meeting.

- b) McLean County Museum of History Meeting (held 10/27/22) Debrief
 - i) Scharnett proposed plan for main entrance (south entrance)
 - ii) War memorial clutter and other ideas...
 - iii) Upcoming meeting with Cassy Taylor, County Administrator
 Discussion amongst team that we would like to further explore some of Scharnett's ideas prior to presentation to County.
- c) Johnson Controls Meeting (held 11/07/22) Debrief -
 - City open to working with Johnson Controls to the extent lighting will be a piece of Design Team's efforts. Kent confirms lighting will be a major component of program.
- d) First Steering Committee Meeting Tentative Schedule –
 New plan is to move up first committee meeting to week of December 5th. Morning meeting and no
 - more than 2 hours likely best option.
 - i) 11/11/22 Finalize Steering Committee members and schedule first meeting
 - ii) Week of 11/21/22 Send out questionnaire to committee members
 - iii) 12/01/22 Questionnaire due back from members
 - iv) 12/06/22 Review questionnaire results and finalize game plan for first steering committee meeting
 - v) Week of 12/12/22 First Steering Committee Meeting
- e) Quick Update from Kent on Friday's Wine/Art Walk
 Amongst other things, Kent remarks on very strong Visual Arts community based in Downtown Bloomington.

2) IMPLEMENTATION PLAN UPDATE:

a) City To Do List:

- Craig and Phil to provide any draft IDS that may have been provided them by IDOT for the Phase I.

- Billy to cancel upcoming meeting with County and instead set up meeting with Scharnett for further discussion of Museum Square options.

- Billy to make contact with prospective Steering Committee members and inform them of their selection, then notify Mike and provide him with their contact info.

- i) Define STEERING committee team member (to meet ~ quarterly). To be finalized this week.
- ii) Determine "critical utilities", ideally by Thanksgiving.
- b) CMT To Do List:

- CMT (Roger) to reach out to Hutchison to determine status of Phase I study and gauge IDOT's stance on walkability improvements.

- Once Billy has made initial contact with Steering Committee members, Mike to reach out and schedule first meeting for week of December 5th.

- i) Prepare less detailed schedule for general use. Done prior to next Core Team meeting.
- ii) Prepare list of promising grants/funding opportunities ongoing. Will be presented by Ally at next Core Team meeting.
- iii) Data collection (utilities, GIS data, aerial imagery) ongoing.
- iv) Develop base map ongoing. Should be completed this week.
- v) Develop existing traffic model ongoing. Counts have been completed.
- vi) Develop narrative for planned downtown improvements.

3) CRITICAL CITY DECISION POINTS:

a) Appointment of Steering Committee – To be finalized this week.

4) "STORIES" TO TELL

a) Upcoming publications/tweets/etc

- Katherine developing piece on underground vaults, then turning to a piece on traffic counts/modeling.

b) Content needed from CMT Team

5) OPEN DISCUSSION / QUESTIONS





CORE TEAM MEETING November 29, 2022 – 9:00 AM

MEETING MINUTES

1) DISCUSSION ITEMS:

- a) Streetscape Grants/Funding Opportunities Ally Fields (CMT Government Affairs Manager) in attendance
 - i) Upcoming Grants/Funding Opportunities List
 - November '22 List included with these meeting minutes.
 - ii) Next Steps

Consensus is that design is not yet developed enough to warrant applying for funding.
City is working on FY'24 budget and will internally strategize about adding a streetscape project to their capital improvements plan as well as engaging Town of Normal and McLean County in discussions so that the project can be included on MPO TIPs. CMT to provide support if asked.
Streetscaping project is a great candidate for Bloomington-Normal Economic Development Council "One Voice Task Force" one year from now.

- b) US-51B Meeting scheduled for Monday, 12/05
 - i) Roger's discussion with Hutchison

- Learned that IDOT has tasked Hutchison with primarily keeping improvements between the curb, with a focus on mill & fill and curb ramp improvements; but IDOT likely to at least consider proposals from City for additional improvements.

- Hutchison is performing 26 Intersection Design Studies along the corridor as part of this Phase I.

- ii) Upcoming meeting strategy
 - Agenda to be prepared by CMT to ensure we discuss everything we want to be covered.

- Need to determine IDOT's schedule for US-51B, both design and construction. When do we need to have things to them?

- Site visit after meeting: Goal is to have IDOT appreciate the economic impact of US-51B choking off Downtown.

- Back pocket strategy: IDOT likely to be more amenable to CoB proposed improvements if the City is willing to consider bearing costs or sharing costs.

- c) Steering Committee Meetings First meeting scheduled for Wednesday, 12/07
 - i) Tentative Overall Meeting Schedule:
 - (1) Early December '22 Big Picture Discussion/Listening
 - (2) Late February '23 Discussing Specific Ideas/Strategy
 - (3) Late April '23 Reviewing Specific Improvements, Colors, Amenities, Themes, etc
 - (4) Late June '23 Plan Review/Final Buy-Off from Committee
 - ii) Tentative First Meeting Format:
 - (1) 1st 30 minutes Intro from Billy/Tim. Brief presentation from Design Team
 - (2) 2nd 45 minutes Break into three focus groups (Local Business Interests, Traffic & Parking, and Cultural/Public Spaces). Each group will spend ~15 minutes considering the *past* (challenges/lessons learned), 15 minutes on *present* conditions (challenges and opportunities), and 15 minutes <u>dreaming big</u> about *future* solutions.
 - (3) 3rd 30 minutes 10 minutes per focus group reporting back the Top 3-5 Takeaways from their discussions. Quick wrap-up.
 - iii) Questionnaire Results -
 - (1) Due back by 12/01/22
 - (2) Results will be compiled prior to 12/06 Core Team Meeting

d) Connect Transit/Market Street Parking Garage -

- i) Update from Billy & Craig
- ii) Overall Downtown Parking Impact due to Displacement of Garage Parking Spaces
- iii) Next Steps

2) IMPLEMENTATION PLAN UPDATE:

a) City To Do List:

- City to internally strategize about how to integrate City Council updates and public meetings into the program design schedule.

- Craig to provide CMT with updated Intersection Design Studies or, if he can't get them from Hutchison in a timely manner, provide us with the older IDS from March.

- i) Data on sewer televising status by 11/25
- ii) Determine "critical utilities", ideally by Thanksgiving.
- b) CMT To Do List:

- Prepare agenda for 12/05 IDOT meeting.

- i) Prepare less detailed schedule for general use. Done prior to next Core Team meeting.
 This schedule summary is being developed for the First Steering Committee Meeting and will be shared with City to aid in City's internal strategy about updating Council and public meetings. (See 2(a) above.)
- ii) Data collection (utilities, GIS data, aerial imagery) complete for now.
- iii) Develop base map complete.
- iv) Develop existing traffic model ongoing. Counts have been completed.
- v) Develop narrative for planned downtown improvements.

3) CRITICAL CITY DECISION POINTS:

a) None at this time

4) "STORIES" TO TELL

- a) Upcoming publications/tweets/etc
- b) Content needed from CMT Team
- 5) OPEN DISCUSSION / QUESTIONS





Downtown Streetscape Potential Funding Opportunities November 2022

Federal:

Rebuilding American Infrastructure with Sustainability and Equity (RAISE) Program

- The RAISE program, included in the Bipartisan Infrastructure Law (BIL) of 2021, is designed to help communities modernize roads, bridges, transit, and other transportation facilities to make the transportation systems safer, more accessible, more affordable, and more sustainable. Projects are evaluated on several criteria, including safety, environmental sustainability, quality of life, economic competitiveness and opportunity, partnership and collaboration, innovation, state of good repair, and mobility and community connectivity.
- The RAISE Program will award at least \$1.5 billion per year through FY2026. While the program has typically opened in the first few months of each calendar year, USDOT is trying to shift to a schedule a few months earlier to more closely align with the fiscal year.
- RAISE grants must be matched by a 20% non-federal contribution.
- RAISE grants do not need to be shovel-ready but are scored on project readiness. Ability to articulate a clear project vision and address any possible barriers to project development (permitting, ROW, etc.) is needed.

State:

Illinois Transportation Enhancement Program (ITEP)

- ITEP is designed to allocate resources to well-planned projects that provide and support alternate modes of transportation enhance the transportation system through preservation of visual and cultural resources and improve the quality of life for members of the communities. ITEP can be used for pedestrian or bicycle facilities, streetscapes, conversion of abandoned railroad corridors to trails, historic preservation and rehab of historic transportation facilities, vegetation management in transportation rights-ofway, storm water management, the construction of turnouts, overlooks, and viewing areas, or other specified uses.
- The ITEP is funded by a mix of state and federal funds and is awarded bi-annually, with the application period opening in the fall of each even numbered year. ITEP includes a \$3 million cap per project.
- Planning studies are not eligible for funding under the ITEP. However, preliminary engineering and construction engineering are eligible.





Illinois Main Streets Program

- The Illinois Main Streets Program set out to support improvements and encourage investment in commercial corridors and downtowns that have experienced disinvestment, particularly in communities hardest-hit by COVID-19. The program funded capital projects that make these commercial areas in Illinois more attractive for private investment, generate short-term and long-term employment opportunities, and improve quality of life in the community through high-quality infrastructure and amenities.
- Under the 2022 program, engineering design was an eligible cost.
- The Illinois Main Streets program provided \$56 million worth of grants between \$250,000 and \$3 million and was awarded in the fall of 2022. This program is unlikely to receive further rounds of funding unless a new capital program is passed.

Other:

State or Federal Budget Line-Items

- State or federal elected officials may be able to place an "earmark" or "line-item" for a project in a federal appropriations bill or the state budget. This route of funding, like grants, cannot be guaranteed, but can be worthwhile if a project is significant enough to garner attention of elected officials.
- The FY2022 federal appropriations package included "Congressionally directed spending" for projects. Line-items for individual projects in the Economic Development Initiatives account averaged \$1.5 million. The committee considered requests of up to \$4 million.
 - Rules for these programs may change in the new Congress in 2023.
- Line-items in federal programs follow the appropriations processes. Federal requests would typically need to be submitted in February or March.
- State budget line-items follow a less standardized process. If they are available to members, outreach early in the legislative session will help position a project.

Typical Timelines									
<u>Program</u>	Application Timeframe	Notice of Award							
RAISE Program	Q1	Q3							
Budget line-items/earmarks	Q1	State: Q2 Federal: Q3 or Q4							
Illinois Transportation Enhancement Program (ITEP)	Q3 – EVEN NUMBERED YEARS ONLY	Q2 – ODD NUMBERED YEARS							



CORE TEAM MEETING December 6, 2022 – 9:00 AM



MEETING MINUTES

1) DISCUSSION ITEMS:

a) IDOT/Hutchison Meeting - Confirm Next Steps

Discussion on risk of Design Team getting bogged down with US-51B efforts at the expense of Downtown core. MBS and CJS assure City that Design Team has capacity, that CMT traffic engineers outside of Core Team will mainly be the ones doing this work, and that there is a risk in waiting to resolve US-51B issues til last as there may be constraints discovered that impact the core design.
Discussion about being aggressive pursuing objectives for US-51B ("it's your road, but this is our community") while balancing against threat of damaging working relationship with IDOT or even having them pull out as they have in other communities. Team will solicit input on strategy for negotiations with IDOT from Roger Driskell when/if the alternatives are deemed viable.

- i) Identify Alternatives (CMT to Develop and Get CoB Blessing)
 Design Team to provide this in next couple of weeks
- ii) Perform Modeling of Alternatives and Document Results - To be completed prior to February meeting with IDOT
- iii) Present Alternatives to IDOT for Consideration Week of February 20th (MBS to Schedule)
- b) Steering Committee Meeting Scheduled for Tomorrow
 - i) Review Presentation
 - ii) Review Questionnaire Results
 - iii) Discuss Subgroup Arrangements
- c) Connect Transit/Market Street Parking Garage
 - i) Update from Billy & Craig

- Project is likely moving forward but is delayed due to Section 106 review. Funding for project will likely include our proposed streetscaping improvements in the vicinity.

- ii) Overall Downtown Parking Impact due to Displacement of Garage Parking Spaces
- iii) Next Steps
- d) Next Meeting Everyone OK with the 20th? Yes.

2) IMPLEMENTATION PLAN UPDATE:

- a) City To Do List:
 - i) Data on sewer televising status by 11/25
 - ii) Determine "critical utilities", ideally by Thanksgiving.
 - iii) Clarify approach/timing for integrating City Council updates and public meetings into the program design schedule.
- b) CMT To Do List:
 - i) Schedule IDOT Meeting for week of February 20th
 - ii) Develop alternatives for US-51B and get CoB blessing before modeling.
 - iii) Prepare less detailed schedule for general use. Amended PowerPoint slide suffice?

3) CRITICAL CITY DECISION POINTS:

a) None at this time

4) "STORIES" TO TELL

- Katherine confirms press release for "Spotlight on Little Known City Structures" and sends to Design Team. - Katherine will lead effort to develop a City website for the Streetscaping project.

- a) Upcoming publications/tweets/etc
- b) Content needed from CMT Team

5) OPEN DISCUSSION / QUESTIONS





CORE TEAM MEETING December 20, 2022 – 9:00 AM

MEETING MINUTES

1) DISCUSSION ITEMS:

- a) Quick Miscellaneous Business
 - i) Meeting with Tim
 - Schedule "sooner rather than later". MBS to make arrangements w/ Amy Overton.
 - ii) Sign Clutter

- Report will include discussion of sign clutter, but project scope will not at this time include detailed sign inventory or document specific solutions. That will be developed at a later point in time.

iii) 1st Steering Committee: Catch-Up Session

- MBS to host catch-up session in January (and following other steering committee meetings) in order to keep everyone in the loop and engaged. No additional participation from other Core Team members necessary.

- iv) Rescheduled County Meeting Status
 Kent relates his recent discussion with Paul and suggests that County is likely OK holding for now.
 As our plan becomes more developed we will re-engage them.
- b) US-51B Alternatives
 - i) Discussion to determine alternatives to model

- City re-affirms priorities: shorten crossing, add buffers for people walking N-S on sidewalk, improve safety, and greenery.

- City OK entertaining relocation of proposed bike lane to adjacent streets, though Craig notes IDOT may take exception since it was the City who first asked them to move it there in accordance with the City's Bicycle Master Plan.

- Phil notes we should be considering actual design vehicles, not just IDOT's policy for design vehicles, when reviewing intersection return radii.

- Kent discusses some big picture ideas for connectivity to the Downtown area. These will be further discussed in subsequent meetings.

- City will provide us feedback by end of day Thursday.
- ii) Schedule for modeling/finalizing alternatives

- CMT will report back on modeling results and work to refine alternatives prior to meeting with IDOT in February.

- Craig notes we may want to feed them information iteratively versus all at once.

c) Next Meeting – January 10th Work Session at CoB

2) IMPLEMENTATION PLAN UPDATE:

- a) City To Do List:
 - i) Data on sewer televising status by 11/25
 - ii) Determine "critical utilities", ideally by Thanksgiving.
- b) CMT To Do List:
 - i) Schedule IDOT Meeting for week of February 20th
 - ii) Develop and model alternatives for US-51B
 - iii) Develop recommendations for integration of broader public into the process.

3) CRITICAL CITY DECISION POINTS:

a) None at this time

4) "STORIES" TO TELL

- a) Upcoming publications/tweets/etc
- b) Content needed from CMT Team

5) OPEN DISCUSSION / QUESTIONS





CORE TEAM WORKSHOP January 10, 2023 – 8:00 AM

MEETING MINUTES

- 1) US-51B CORRIDOR / BIKE LANE CONCEPTS (~1h) - Reviewed preliminary data from traffic model.
- 2) MARKET ST / EAST ST CONCEPTS (~0.5h) - Reviewed preliminary concepts.
- 3) OUTER LOOP CONCEPT (~0.5h)
 Discussed traffic circulation. Discussed need to schedule meeting with Connect Transit to discuss bus traffic/routes.
- 4) JEFFERSON OVERPASS (~0.5h) - Reviewed preliminary concepts.
- 5) 2ND STEERING COMMITTEE MEETING STRATEGY (~0.5h)
 Discussed when we should meet again and what content to present. Billy to discuss with Tim and then provide Design Team direction.
- 6) 2ND IDOT MEETING STRATEGY (~0.25h)
 Design Team needs to run additional simulations and prepare exhibits clarifying alternatives. Will report back at next Core Team Workshop in two weeks.
- 7) INTRO TO 2ND CORE TEAM WORKSHOP (~0.5h)
- 8) MEETING WRAP-UP / NEXT STEPS DISCUSSION (~0.25h)





CORE TEAM WORKSHOP #2 January 23, 2023 – 8:00 AM

MEETING MINUTES

- 1) OLD BUSINESS (~45m)
 - a) US-51B Corridor Preliminary Concepts

- Discussed various alternatives as well as which blocks support which alternatives. Reviewed traffic modeling results.

- b) Bike Lanes Preliminary Concepts - Discussed various alternatives.
- 2) NEW BUSINESS (~2h)
 - a) Main & Center Preliminary Concepts - Discussed various alternatives.
 - b) Museum Square Preliminary Concepts - Discussed various alternatives.
 - N Main Plaza Preliminary Concepts
 Discussed various alternatives.
 - Roundabout Transfer Station Preliminary Concepts
 Discussed various alternatives.

3) MEETING UPDATES (~1h)

- a) Connect Transit 01/25 @ 2:00 PM (in person)
 Kent and Mike to meet with David and discuss options for routing bus traffic out of the downtown core.
- b) County Architect 01/25 @ 3:30 PM (in person)
 Billy to get back with Design Team about additional invitees.
- c) IDOT #2 02/22 @ 10:00 AM (in person)
 Discussed approach for discussing alternatives with IDOT.
- d) Stakeholder #2 TBD
 Mike to schedule meeting for early March.
- e) City Council TBD
 Discussed meeting with Council using "three on ones" or at a Committee of the Whole
 f) Correct Team Workshop (2) 02 (21 0.020 AMA (in memory))
- f) Core Team Workshop #3 02/21 @ 8:00 AM (in person)
- g)—Public Meeting TBD
- h) Utility Meetings TBD
- 4) MEETING WRAP-UP / NEXT STEPS DISCUSSION (~15m)





CORE TEAM MEETING February 7, 2022 – 9:00 AM

MEETING MINUTES

1) DISCUSSION ITEMS:

- a) Engagement Plan
 - i) Review strategy

Discussion includes activities the Design Team will initiate as well as supplemental activities City personnel will initiate. City needs a little time to determine what this should look like.
Design Team to keep Katherine involved during this process, to make sure City's story is being told.

ii) Review schedule and milestones

- Craig and Billy will discuss overall schedule and sequencing of milestones in relation to how soon we can begin a construction project.

iii) Review next steps

- Core Team members from the City will meet to review the engagement plan and decision matrix and then report back to the Design Team.

b) Review Decision Matrix -

- City was issued Decision Matrix comprised of concepts previously presented by the Design Team. Goal is for the City to meet, without any of the members of the Design Team present, to come to a consensus on what direction City wants to go.

- Design Team will be available at any point to clarify decision points/concepts and/or provide additional information.

c) Meeting(s) with Utility Companies – Brief discussion on who should attend from City

- Craig states that he would like to participate in these meetings. CMT will begin to set these meetings up.

- d) Upcoming Meetings -
 - February 8th Meeting with County
 Design Team will present ideas/concepts previously presented to Core Team with caveat that these ideas are very preliminary and have not yet been vetted past Council or any outside bodies.
 - ii) February 21st Workshop at CoB
 - iii) February 22nd IDOT Meeting
 - iv) March 6th 7th Steering Committee Meeting

2) IMPLEMENTATION PLAN UPDATE:

- a) City To Do List
 - i) Determine "critical utilities", ideally by Thanksgiving.

- Craig reports that we will begin receiving this information yet this month.

- b) CMT To Do List
 - i) Arrange for meetings with utility companies, accessibility advocacy group, and Historic Preservation Commission
 - ii) Ongoing work on report

3) CRITICAL CITY DECISION POINTS:

a) Approve or modify Engagement Plan by 2/21

- Return Preliminary "Group A Decisions" to Design Team by 2/21. Returning sooner may allow CMT to engage Workbench in preparing renderings prior to the March 7th Steering Committee meeting.

- Because of the upcoming IDOT meeting, City will prioritize decisions related to US-51B by the end of the week.

- 4) "STORIES" TO TELL
 - a) Pending
- 5) OPEN DISCUSSION / QUESTIONS





CORE TEAM WORKSHOP #3 February 28, 2023 – 8:00 AM

AGENDA ITEMS

1) REVIEW OF ENGAGEMENT PLAN/SCHEDULE

- a) Discussion led by CoB
- b) Next Steps

2) REVIEW OF DECISION MATRIX

- a) Discussion let by CoB
- b) Next Steps

3) BRIEF DISCUSSION ON IDOT

4) DISCUSSION OF UPCOMING MEETINGS

- a) Steering Committee Meeting #2 03/07 @ 2:30 PM
- b) City Council Committee of the Whole 03/20?
- c) Public Meeting TBD
- d) Utility Meetings Ongoing
- e) Interest Group Meetings (Bike BloNo and ADA Advocacy Group) TBD

5) MEETING WRAP-UP / NEXT STEPS DISCUSSION





CORE TEAM MEETING March 6, 2022 – 1:00 PM

MEETING MINUTES

1) DISCUSSION ITEMS:

- a) 2nd Steering Committee Meeting
 - i) Discuss goals of meeting

 Want to make sure we're not just providing the committee with an update but getting plenty of feedback from them. Goal is to vet the concepts past them.
 - ii) Review presentation format and content
 - Received input from City staff on modifications needed to PowerPoint.
 - iii) Get City input on highlighted sections
- b) What's Next?
 - i) IDOT
 - (1) 3^{rd} Meeting scheduled for 3/30
 - (2) Need to schedule meeting with Bike BloNo 3/13?
 Craig will schedule.
 - (3) Get City input on drawings to be shared at meeting
 Craig suggests overall bike route map is sufficient for this meeting and details of highway crossings are not yet needed.
 - (4) Ask IDOT for permission to share concepts?
 - Billy says don't reach out to them about this yet.
 - ii) Engagement Strategy -

- Billy says he will get this info to Design Team soon. Discussed that there are additional activities beyond those proposed by the Design Team that will be necessary for engagement strategy to be successful. Mike clarifies that Decision Matrix returned by the City is helpful, but in order for design rework to be avoided the concepts should be vetted past the Steering Committee/Council/public before Design Team gets too far ahead of themselves.

- (1) Committee of the Whole? (Meet with County first?)
- (2) Broader Public Engagement?
- (3) Renderings
- (4) Websites? Storyteller videos?
- (5) Overall schedule?
- iii) Museum
 - Design meeting with Scharnett

 Billy says hold off on design meeting with Scharnett, but that he will work to schedule meeting with County.
 - (2) Share concepts with the County (and have museum folks in attendance)
- iv) Design -
 - (1) Further exploration of North Main Plaza and Gateway Features
 - (2) Develop preliminary palette of materials/colors/furnishings
 - (3) Individual Street Design ON HOLD?

2) IMPLEMENTATION PLAN UPDATE:

- a) City To Do List
 - i) Determine "critical utilities".
 - ii) Set up meeting with Bike BloNo.

- iii) Determine Engagement Strategy and associated timeline.
- b) CMT To Do List
 - i) Provide City with questions related to sewer televising exhibit.
 - ii) Arrange for meetings with accessibility advocacy group and Historic Preservation Commission.
 - iii) Ongoing work on report.

3) CRITICAL CITY DECISION POINTS:

a) Approve or modify Engagement Plan by 2/21

4) "STORIES" TO TELL

- a) Pending
- 5) OPEN DISCUSSION / QUESTIONS





CORE TEAM MEETING March 21, 2023 – 9:00 PM

MEETING MINUTES

1) DISCUSSION ITEMS:

- a) 2nd Steering Committee Meeting Recap
 - i) General impressions

- Overall consensus is that for the most part all concepts proposed to the Steering Committee were well received.

- ii) Follow-up items:
 - (1) Refuse management & committee member follow-up question
 - Craig and Billy OK with Design Team reaching out to committee for clarification, but also suggest that concerns with a "dirty" Downtown likely stem from (1) overflowing public trash cans out on the sidewalk and (2) unorganized/disjointed dumpster locations for businesses/residents.
 - (2) Museum Square North stage capacity versus existing events

- Design team shares preliminary north stage capacity calculations: ~3,000 people if seated and ~5,000 people if standing. Capacity is less if Main and Center are left open. This capacity is very similar to current attendance at outdoor concerts reported by Kim.

- (3) Committee members reporting back on public feedback for concepts - Design Team will reach out for update.
- iii) Meeting minutes

- Billy will review Mike's minutes.

iv) Make-up session

- Mike will schedule. Nobody from City need attend.

- b) Upcoming IDOT Meeting (3/30)
 - i) Information requested by IDOT:
 - (1) Sign-off on bike lane relocation by Bike BloNo
 - Meeting scheduled with Bike BloNo and Friends of Constitution Trail for 3/22/23.
 - (2) Documented ability to fund local improvements
 - Billy asks at what point in IDOT's process must City make funding commitment. Mike will speak with Roger and report back.
 - ii) Discuss City's implementation alternatives and associated strategy
 - Alternatives include:

- Pushing IDOT to modify their design to incorporate our improvements. (Challenges to meeting IDOT's schedule and possibility of exceeding program funding, even if local portions reimbursed later.)

- Have IDOT remove the downtown section from their plans and City take over design as a permit project. (Would require a commitment from the City to undertake this work and would result in City bearing all costs.)

- Possible hybrid of above two alternatives. (E.g., IDOT re-pave just center two lanes. Craig has some misgivings over this scenario.)

- c) Renderings Discussion
 - i) Discuss rendering "levels" 1, 2, and 3

- Design Team presents spreadsheet with Points of Interest and associated levels of renderings. Core Team will make final decision at upcoming Workshop #4.

- ii) Confirm "point of interest"/locations for renderings
 Craig suggests we likely need to include renderings for US-51B.
- iii) Discuss schedule for delivery
- d) Traffic Modeling Discussion Confirm elements to be modeled.
 Design Team suggests modeling all intersections as stop signs rather than existing signals, except at Washington Street which is a designated emergency access route. Stop signs will further elevate the "walkability" Downtown.

- Design team will begin modeling these alternatives, with goal of presenting findings at Workshop #5.

e) Future Workshops – Tentative schedule: Workshop #4 on 4/04, Workshop #5 on 4/18
 - Mike to send out Outlook invites and Craig to reserve rooms.

2) IMPLEMENTATION PLAN UPDATE:

- a) City To Do List
 - i) Determine Engagement Strategy and associated timeline.
 Billy to send me his thoughts that he's jotted down, then Mike to prepare 2nd version of Engagement Strategy/Plan.
 - ii) Schedule meeting with County.
 - iii) Respond to questions about Right-of-Way irregularities.
 Craig directs Design Team to just use street centerline and fine tune with boundary survey during detailed design.
 - iv) Determine "critical utilities".

- City meeting internally on this next week.

- CMT still owes City feedback on sewer drawings.

- b) CMT To Do List
 - i) Provide City with questions related to sewer televising exhibit.
 - ii) Arrange for meetings with accessibility advocacy group.
 - Mike to prepare draft agenda and share with Craig, then set up meeting.
 iii) Ongoing work on report.

ing ongoing work on report.

3) CRITICAL CITY DECISION POINTS:

a) Approve or modify Engagement Plan by 2/21

4) OPEN DISCUSSION / QUESTIONS





CORE TEAM WORKSHOP #4 April 4, 2023 – 9:00 AM

MEETING MINUTES

1) CONCEPTS: OLD BUSINESS

a) Updated Streetscape Sections

- Reviewed geometrics for typical sections, intersections, and mid-block crossings, with particular emphasis on opportunities for amenities like outdoor dining and public art.

- Also discussed street graphics and other differentiators to give Bloomington its own sense of identity.

- Brief discussion on cost of losing parking in favor of increasing walkability, establishing open space for dining, etc. Additional alternatives for the street sections will be explored by the Design Team in order to validate choices.

- It is desirable to implement green/sustainable features with streetscaping improvements. Is is also desirable to improve storm water management issues.

- b) Refined Museum Square Concept
 - Discussed opportunities for additional activities (oversized checkers, e.g.).
 - Discussed preservation of existing trees.
 - Concerts on the north end of the square could support crowds in the range of 3,000-5,000.
- c) Refined North Main Plaza Concept
 - Discussed potential opportunity for similar alternative in BCPA lawn.

2) CONCEPTS: NEW BUSINESS

- a) Streetscape Materials Introduction
 - Preference is for clay pavers rather than precast concrete pavers that are sensitive to salt degradation.
 - Pavers should not be located in the main traffic lane.
 - Preference is for fixed banners rather than cloth due to their durability.
 - Planters will likely need to be self-watering. Coordination with Parks Department should occur.
 - Tree grates and ash receptacles are not desired.
 - Stained concrete may be explored, but all material choices should be both maintainable and replaceable.

3) BRIEF DISCUSSION ITEMS

- a) Rendering Decisions
- b) Brief Discussion on Program Cost Estimating/Budget
- c) IDOT Follow Up?

4) MEETING UPDATES

- a) Storybook Studios Meeting 04/06 @ 1:30 PM
- b) Core Team Workshop #5 04/18 @ 9:00 AM
- c) Utility Meetings Frontier?
- d) ADA Advocacy Group TBD
- e) City Council Meeting TBD
- f) Public Meeting TBD

5) MEETING WRAP-UP / NEXT STEPS DISCUSSION

- Additional workshops to be scheduled in order to keep making progress on the many elements of this design.





CORE TEAM WORKSHOP #5 April 18, 2023 – 8:30 AM

MEETING MINUTES

1) CONCEPTS: OLD BUSINESS

- a) Updated Street Sections on Museum Square Drawing
- b) North Main Plaza/BCPA Lawn Discussion
 - City directs Design Team to provide new iteration that will perhaps result in more open lawn space.
- c) Main and Center Sections/Downtown Parking Discussion

- Various street sections were presented by the Design Team for Main and Center demonstrating trade-offs between improved pedestrian space/outdoor dining and quantity of on-street parking. Discussed "walkable downtown" concept, which involves moving some spaces off streets and instead consolidating parking into a few larger lots/garages: people need to walk further, but outdoor opportunities for the "public sphere" are increased.

- Group decided we need additional input from business owners and broader public so we can feel confident in what direction we move forward with.

2) CONCEPTS: NEW BUSINESS

- a) None at this time
- b) 5/02 Meeting: Gateway Structures and Refuse Management Options
- c) 5/16 Meeting: Detailed colors/materials/fixtures/etc and Interior Traffic Modeling (stop signs vs signals)
 Discussed that this agenda item should be moved up to the 5/02 meeting, in order to facilitate renderings that are more representative of what we may eventually end up with.

3) BRIEF DISCUSSION ITEMS

a) Rendering Decisions

- In order to help with visualization of street options for the public, a "Level 2" rendering will be prepared for each of the three Main Street options. Renderings of Museum Square will follow.

b) IDOT Follow Up (and Results of HCS Modeling)

- Design Team shared that HCS modeling results in a justifiable argument for reducing lanes to two on US-51B. City authorizes CMT to reach out directly to IDOT's geometrics engineer to discuss assumptions made when preparing the models as well as working out other details necessary to receive from IDOT a preliminary blessing on what has been proposed.

c) Brief Discussion on Program Cost Estimating/Budget

4) MEETING UPDATES

- a) Core Team Workshop #6 05/02 @ 8:30 AM
- b) Utility Meetings Frontier?
- c) ADA Advocacy Group TBD
- d) City Council Meeting TBD
- e) Public Meeting TBD

- Group discussed presenting options to the public in mid-late May. The format will likely be a combination of in-person community input workshop and online tool for soliciting input. Around the same time, City Aldermen will also be consulted. Design Team to develop short presentation, drawings, renderings, etc. for these meetings. M Sewell and B Tyus to discuss further.

5) MEETING WRAP-UP / NEXT STEPS DISCUSSION





CORE TEAM WORKSHOP #6 May 2, 2023 – 8:30 AM

MEETING MINUTES

1) THE PATH FORWARD FOR THIS PROJECT

- a) Review flow chart - See attached.
- b) Discuss "categories of emphasis" and prioritization

- Discussed items that must be resolved prior to Design Team being able to produce streetscaping drawings for Downtown. The two items of highest priority are (1) resolution to the Main/Center parking dilemma and (2) resolution to the US-51B cross-section.

c) Discuss parallel paths: (1) design path and (2) public engagement/momentum building path

2) CONCEPTS TO REVIEW:

- a) Revised BCPA Lawn/Parking Concept - City reaffirmed desire for a surface parking lot, not structure.
- b) Downtown Overall Parking Alternatives Exhibit

- Discussion regarding location of displaced parking (based on proposed Main/Center roadway crosssections) into concentrated locations at the corners of Downtown. Design Team notes that if parking is not available directly in front of Main/Center businesses, people will have to walk no more than two blocks in any direction from concentrated parking locations to arrive at destination. Discussion amongst Core Team on benefits of a transformative, more walkable Downtown versus the cost of asking people to potentially walk further. This question will be put directly to the public for feedback.

c) Detailed colors/materials/fixtures/etc
 - Kent shares various options for pavement and light fixtures. Core Team has interest in exploring suspended lights on cable, similar to Dubuque Millwork Marketplace. Core Team agrees with Design Team's suggestions for a light pole/fixture aesthetic that is more traditional than modern.

3) UPCOMING PUBLIC OUTREACH PHASE:

a) Content: Review Outline

- Discussion on content for presentation that brings public up to speed, presents concepts, and poses specific questions to them for feedback.

- b) Format:
 - Exact format for public outreach to be discussed in upcoming meeting between CMT and City on 5/04/23.
 - i) In-Person Public Input Workshop:
 - (1) Attendees: Business Owners vs Broad Public
 - (2) Venue?
 - (3) Who is Responsible for Setting Up
 - ii) Online Tools:
 - (1) Website?
 - (2) Pinpoint/Engagement HQ/etc?
 - (3) Who is Responsible for Setting Up
 - iii) City Council (three on ones)
- c) Future Public Outreach Items:
 - i) Social Organization Road Tour
 - ii) Storyteller Studios Videos
 - iii) Weekly to Biweekly Videos/Social Media Posts (to provide regular engagement)
- 4) FUTURE MEETING ITEMS:

- a) Gateway Structures
- b) Refuse Management Options
- c) Interior Traffic Modeling (stop signs vs signals)
- d) Brief Discussion on Program Cost Estimating/Budget

5) MEETING UPDATES

- a) Core Team Workshop #7 05/16 @ 8:30 AM
- b) Utility Meetings Frontier?
 Craig agrees we should set up meeting with Frontier, similar to meetings Team has already conducted with Ameren and Nicor.
- c) ADA Advocacy Group TBD
 Discussion that we should get this meeting scheduled for week of 5/08/23 if possible.
- d) City Council Meeting TBD
- e) Public Meeting TBD

6) MEETING WRAP-UP / NEXT STEPS DISCUSSION

Downtown Streetscaping Project Path Forward 4/27/2023




DOWNTOWN STREETSCAPE DESIGN



CORE TEAM WORKSHOP #7 August 22nd, 2023 – 8:30 AM

MEETING MINUTES

- 1) THE PATH FORWARD FOR THIS PROJECT
 - a) Quick recap, for context
 - Discussed critical path items and how to resolve them. See original graphic from 04/27/23 attached.
 - b) Review/discuss overall schedule

- Discussed impact of public outreach campaign on overall design schedule. Core team discussed schedule alternatives to expedite the first streetscaping project. See draft schedule attached.

2) US-51B NEXT STEPS:

a) Review latest response from IDOT

- Despite the Design Team having demonstrated that a two-lane section is acceptable, the District engineers state that they will proceed with a three-lane construction project.

b) Discuss alternatives
 - Billy to discuss with Tim and report back to the team regarding next steps.

3) UPCOMING PUBLIC OUTREACH PHASE:

- a) Social Media:
 - i) Teaser video is live any feedback?
 - ii) Feature video in production will point towards website
- b) Website:
 - i) Discuss content/outline
 - ii) Confirm what specific input we want from public
 - (1) Parking philosophy Design Team instructed to include this
 - (2) Level of investment (too big/too little) Design Team instructed to not include this
 - (3) Review colors/materials/furnishings, etc Design Team instructed to include this
 - (4) Curbs/no curbs? Discussion to be resolved at next Core Team Meeting
- c) In-Person Public Input Workshop:
 - i) Discuss venue, date, and format of workshop
 - ii) Discuss CoB/CMT responsibilities Securing venue, large prints/easels, etc
- d) Future Public Outreach Items:
 - i) Social Organization Road Tour
 - ii) Weekly to Biweekly Videos/Social Media Posts (to provide regular engagement)

4) FUTURE STEERING COMMITTEE MEETING:

- a) Discuss schedule and purpose of future meetings
 - i) Suggestion for 3rd meeting: Meet two weeks after public input to discuss direction related to specific design elements and review of colors, materials, furnishings, etc. Also discuss trash collection alternatives.
 Discussed that an additional meeting should be held sooner rather than later to provide an update to the committee. MBS to schedule.
 - ii) Suggestion for 4th meeting: Meet one more time to discuss phasing/prioritization and also general buy-off on plan (provide draft in advance?)

5) MATERIALS/COLORS/FIXTURES RECOMMENDATIONS:

- a) Review alternatives and recommendations from Design Team
- b) Discuss how selections will ultimately be made.

6) MEETING WRAP-UP / NEXT STEPS DISCUSSION

- Items that were not discussed at this workshop will be pushed to the following Core Team Meeting.

Downtown Streetscaping Project Path Forward 4/27/2023



Downtown Streetscape - Overall Schedule



Updated 8/23/23

Overall Streetscaping Plan			First Streetscape Project		
Prepare for Public Outreach: Develop website, video, secure Public Input Workshop venue, etc. AND host Steering Committee Meeting #3.	1.5 Months				
Milestone: Public Outreach Launch		10/9/2023			
Public Outreach: Solicit public input thru website and Public Input Workshop.	2 Weeks				
Make Critical Overall-Plan Design Decisions: Host Steering Committee Meeting #4 followed by Core Team Meeting.	1 Week		Determine What First Project Will Be – Additional time needed in addition to adjacent "Make Critical Overall-Plan Design Decisions" . Council	3 Weeks	
Milestone: Resume Design Efforts		10/30/2023	3x1s?		
Planning Production Phase: Prepare overall plan drawings and cost estimates. Concurrently, resolve outstanding design issues such as refuse management, gateway features, BCPA, etc.	3 Months		Milestone: 1 st Project Determined and Contract Preparation Begins		11/13/2023
			1 st <i>Project:</i> Contract preparation/ negotiation/getting on council agenda.	2 Months	
			Milestone: City Council Approves 1 st Design Contract		1/8/2024
Programming Phase: Design Team develops recommendations for phasing strategy. Then host Steering Committee Meeting #5 followed by Core Team Workshop to approve phasing strategy and finalize plan.	1 Month		1 st Project – Detailed Design (accelerated) : Survey, prepare construction documents, secure	5 Months	
Milestone: City Council Adopts Plan		3/11/2024	permits, prepare bid documents, etc.		
			Milestone: Bid 1 st Streetscaping Project		June '24
			1 st Project – Bidding Process: Pre-bid meeting, addendum, etc.	2 Months	
			Milestone: Award 1 st Streetscaping Project		August '24
			Milestone: Issue Notice to Proceed for 1 st Project		September '24
			Construct 1 st Streetscaping Project.	3 Months, then break for winter, then 6 Months	A PH
			Milestone: 1 st Streetscaping Project Complete	312023	September '25







August 29, 2023 September 5, 2023 – 9:00 PM

MEETING MINUTES

1) OLD BUSINESS:

- a) Update on US-51B
 - Discussion about each of the options for moving forward with IDOT on US-51B.
 - Discussion to be resumed at following Core Team Meeting.
- b) Updated Schedule
 - Billy states that we will be moving forward with updated schedule. See attached.
- c) Public Outreach
 - i) Quick discussion on curbs
 - Decision made to introduce zero curbs on Main and Center where drainage allows for it.
 - ii) Video/Website development
 - iii) In-Person Public Input Workshop
 MBS to follow up with Katherine and begin coordinating this workshop.
 - (1) Discuss venue, date, and format of workshop
 - (2) Discuss CoB/CMT responsibilities Securing venue, large prints/easels, etc

d) Review Draft Agendas for Upcoming Steering Committee Meetings

2) NEW BUSINESS:

- a) Materials/Colors/Fixtures/Recommendations
 - Review alternatives and recommendations from Design Team

 Design Team directed to research new options for lighting, chairs/tables, and to prepare drawings showing various color/material options that make Downtown Bloomington unique.
 Discussion to be resumed at following Core Team Meeting.
 - ii) Discuss how selections will ultimately be made.

3) IMPLEMENTATION PLAN UPDATE:

- a) City To Do List
 - i) Respond to Clear Design deadlines.
 - ii) Coordinate with CMT on in-person workshop.
- b) CMT To Do List
 - i) Coordinate with City on in-person workshop.
 - ii) Provide cost estimate on Museum Square project.
 - iii) Ongoing work on report.

- a) Clear Design stuff
- 5) OPEN DISCUSSION / QUESTIONS





CORE TEAM MEETING September 8, 2023 – 9:00 PM

MEETING MINUTES

1) OLD BUSINESS:

- a) Resume discussion on US-51B
 - Discussed a phased option of reducing traffic lanes and crossing distances.
 - Billy to get in touch with MBS about final direction.

2) NEW BUSINESS:

- a) Materials/Colors/Fixtures/Recommendations
 - i) Review alternatives and recommendations from Design Team
 - Recommendations from City staff included the following:
 - Charging stations with adjacent seating options and shade.
 - Planters should be smaller in pedestrian walking spaces, so as not to take up valuable real

estate.

- No tree grates.
- Provide digital directory signs in key locations.
- Provide locations for public art that can be on a rotation.
- Promote use of building murals.

- Discussed trash collection system, and how much the proposed street section will impact options. Discussion to be resumed at a future meeting next week.

ii) Discuss how selections will ultimately be made.
 Billy to get in touch with MBS to follow up.

3) IMPLEMENTATION UPDATE:

- a) City To Do List
 - i) Finalize recommendations on materials/colors/fixtures by Weds 9/20
 - ii) Coordinate with CMT on in-person workshop.
- b) CMT To Do List
 - i) Provide City revised materials/colors/fixtures selections by Weds 9/13
 - ii) Coordinate with City on in-person workshop.
 - iii) Provide cost estimate on Museum Square project.
 - iv) Ongoing work on report.

- i) Finalize recommendations on materials/colors/fixtures by Weds 9/20
- ii) Provide final direction on US-51B
- 5) OPEN DISCUSSION / QUESTIONS





CORE TEAM MEETING October 17, 2023 – 9:00 PM

MEETING MINUTES

1) SCHEDULE REVIEW:

a) Discuss upcoming milestones.

- 10/23 – Steering Committee Meeting #4 to provide final recommendations on Streetscape Section and Styles. Committee will also discuss first streetscape project out of the entire Downtown program to be taken on.

- 10/25 – Core Team Meeting to finalize key design decisions (related to section and style). Design Team to then resume design efforts.

- 11/13 – First streetscape project to be determined.

2) NEXT WEEK'S STEERING COMMITTEE MEETING:

- a) Review draft agenda.
- b) Discuss decision making process Schedule follow-up Core Team Meeting?

- Confirmed that decision would be made by the City, not the Steering Committee. The latter is an advisory group only.

3) THORN RUN / CMT COORDINATION:

a) Discuss potential coordination meeting.

- Mike discusses that concepts are developed enough that we can begin pursuing funding opportunities, grants, etc. DowntownforEveryone.com website has content that will show very well.

- Mike to provide Ally Field's contact info to Billy so that this meeting can be scheduled.

4) GARBAGE MANAGEMENT:

a) Discuss next steps, including website update.

- Billy says that City will have an internal meeting on this topic soon.

5) REVIEW PUBLIC FEEDBACK THUS FAR:

- i) Street Section
 - "Alternate B" is the clear favorite thus far.
- ii) Street Theme
 - "Hybrid Blend" is the clear favorite thus far.
- iii) Comments

6) IMPLEMENTATION UPDATE:

- a) City To Do List
 - i) None at this time.
- b) CMT To Do List
 - i) Provide cost estimate on Museum Square project.
 - ii) Traffic modeling of four-way stops.
 - iii) Ongoing work on report.

- i) Finalize street section and street theme decisions by 10/27.
- ii) Determine first street scaping project by 11/10.

8) OPEN DISCUSSION / QUESTIONS





CORE TEAM MEETING November 7, 2023 – 9:00 AM

MEETING MINUTES

1) 1ST STREETSCAPE PROJECT UPDATE

- Mike reports that he met with Billy, Tim, and Melissa on 11/03/23. Potential first project out of the overall Downtown program discussed. CMT to prepare high-level cost estimate to verify that the project budget is acceptable before moving forward.

2) EV CHARGING STATION UPDATE FROM PHIL

- Phil discusses meeting with Ameren representatives to discuss Bloomington's community readiness for electric vehicle charging stations (EVCS).

- There are various funding opportunities available for EVCS installations, though municipalities are not eligible for some of them.

- Mike to reach out to Russ Waller, City Facilities Manager, to learn more info on desired EVCS locations from Russ' consultant.

3) SCHEDULE DISCUSSION

- Following today's discussion, Design Team will work for ~4 weeks to further refine concepts and Core Team will regroup on 12/05/23.

- Steering Committee Meeting #5 tentatively scheduled for 12/18/23. Tentative agenda: (1) Progress/Schedule Update, (2) Review & Discuss "Hybrid Style", (3) Review & Discuss Remaining Design Concepts (East-West Streets, Center Street, US-51B Areas, BCPA Area), (4) Review & Discuss Project Phasing, and (5) Review & Discuss Garbage Management Plan.

4) CONCEPT REVIEW:

a) East-West Streets

- Design Team proposes the following: (1) corner curb extensions to shorten pedestrian crossings, (2) bold graphic pavement striping (similar to the core area) with indicators for on-street parallel parking and driveway locations for no parking, (3) brick amenity strip and/or turf and trees between curb and sidewalks, and (4) uniform sidewalk width and treatment.

- Phil requests an alternate marking for the no-parking locations.

b) Center Street

- Design Team proposes the following: (1) pedestrian space to duplicate what is proposed for Main Street with uniform concrete sidewalks and wide amenity strips for outdoor activities and plantings, (2) curb extensions on the east side only to shorten pedestrian crossings, (3) parallel parking on the east side similar to the existing configuration, and (4) bold graphic pavement striping indicating the flexible use lane on the west side.

c) US-51B

- Design Team proposes the following: (1) modification of the existing four lanes of traffic to two lanes of traffic, (2) crosswalk distances will reduce approximately 50% as a result of the lane reductions, (3) wider green space on both sides for turf, street trees, and the space for lighting and other amenities, (4) maintain a typical 8-foot sidewalk adjacent to the properties on the core side of the streets, (5) create a 10-foot wide shared use path on the outward side of the core area that would be set 5-foot off the property lines to allow for screening and ADA access to adjacent properties, and (6) add sculpture, illuminated walls, wayfinding and landmark signage along the streets to add character.

- Core Team discusses the need to emphasize to the public that the ultimate development of US-51B

into two-lanes is not going to be the next iteration, since IDOT first plans to repave and re-stripe down to three-lanes. This can be communicated on the website, in the report, and in a presentation to Council.

d) BCPA Area

- Design Team proposes the following: (1) Reconfigure the area (based on minimal acquisition of open space) to align Market Street which will reduce the two existing intersections down to one. The new intersection can be developed with just a minor alignment kink if the traffic and parking on Market Street between East and Main Streets is modified. (2) Reconfigure the open lawn space by BCPA to be more linear and to provide a major connection to both the North Main area and to the BCPA. Add sculpture and trees to the space making it more park-like. (3) Develop a major parking area with smaller parking areas to accommodate large events, local business patrons, and the potential for large markets/concerts needing a paved area. Underground storm water detention and other environmentally friendly practices can be incorporated in the design and materials. (4) Douglas Street would be diverted to Market Street, and East Market east of East Street would be converted to just parking. No building removal is required.

5) SIGNALS \rightarrow ALL-WAY STOPS:

a) Review modeling results

- CMT shares results and show how intersection Level of Services greatly improve by changing from signals to all-way stops at Market/Center and Market/Main.

- b) Discuss next steps for integration into plan
 - Phil asks CMT to provide results of queueing lengths.

- CMT to verify with Connect Transit the routing of busses into and out of the new transit center.

6) IMPLEMENTATION UPDATE:

- a) City To Do List
 - i) Social organization road show? How can CMT help?
 Billy says City will handle this.
 - ii) Thorn Run/Allie Fields meeting?

- Billy says City will schedule.

- b) CMT To Do List
 - i) Develop concept of chosen street style, including renderings.
 - ii) Prepare final design elements (BCPA, areas around US-51B, etc).
 - iii) Ongoing work on report.

- i) Determine first street scaping project by 11/10.
- 8) OPEN DISCUSSION / QUESTIONS



CORE TEAM MEETING December 6, 2023 – 1:00 PM



MEETING MINUTES

1) CONCEPT REVIEW:

a) Hybrid Style Draft and Materials

- Reviewed options for styles, including colors, light fixtures, and materials. Narrowed options down to two which will be presented to the Steering Committee and public.

b) North Main Plaza Update

- Design Team presented refined concept which was well received by the Core Team. Design Team will proceed with renderings for this area.

2) REVIEW DRAFT PHASING PLAN

- Design Team presented plan for dividing overall program into logical phases and sequencing them in ways that minimize negative impacts to businesses during construction as well as other criteria. Well received by the Core Team. Acknowledged that phasing plan is subject to change based on available grants and other future unforeseen conditions.

3) DISCUSS SUSTAINABILITY IMPLEMENTATION

- Design Team reviewed various design elements that will be incorporated into the Downtown program that will result in improved public health and/or implementation of sustainable practices.

4) PUBLIC MEETING/SCHEDULE DISCUSSION

- City will follow up with Design Team regarding future opportunity for engaging and updating the public. CMT will provide list of potential topics for meeting to City for consideration.

5) ADDITIONAL RENDERING CANDIDATES

- a) North Main Plaza Decision made to prepare full rendering.
- b) BCPA Area Decision made to prepare an artistic sketch less detailed than full rendering.
- c) US-51B Areas Decision made to prepare an artistic sketch less detailed than full rendering.

6) PUBLIC ART UPDATE

- MBS updated Core Team about meetings and Downtown walk-thru with Doug Johnston, Director of the McLean County Arts Center.

- Final Report will include an exhibit with potential locations for public art (murals, sculptures, etc.) as well as information on how to best plan for and implement public art.

7) IMPLEMENTATION UPDATE:

- a) City To Do List
 - i) All-way stops Input needed for next steps
- b) CMT To Do List
 - i) Provide input to City on RAISE Grant pursuit
 - ii) Develop any remaining renderings
 - iii) Ongoing work on report

- i) Direction on public input
 - Billy to eventually follow up with Design Team on what is required.
- ii) Selection of City PM for first project
 Phil Allyn will be City PM. CMT to set up meeting to coordinate questions.
- 9) OPEN DISCUSSION / QUESTIONS





CORE TEAM MEETING January 9, 2024 – 9:00 AM

MEETING MINUTES

- 1) DOWNTOWN FOR EVERYONE CAMPAIGN:
 - a) Upcoming website updates
 - Updates to correspond with second community open house in February.
 - b) Re-engagement Campaign
 - Need to make sure momentum doesn't stall out if construction doesn't commence this year.

- Josh with Clear encourages a re-engagement campaign, possibly including billboards because not everyone is re-visiting the website. Many outside of our Downtown sphere aren't even aware of the streetscaping program.

2) FINAL REPORT PREVIEW

Design Team shares preview of Chapter 1 to ensure City is OK with look and feel.City staff sign-off on look and feel, believe quality of deliverable is appropriate.

3) DISCUSS OVERALL SCHEDULE

a) Construction Schedule

- Construction could start in very late 2024, but both City and Design Team agree that tearing things up right before the winter would cause more harm than good. Need to find alternate way of demonstrating progress and keeping momentum.

b) Public Update

- Mike is looking to schedule a 2nd Community Open House for early to mid-February at the BCPA.

c) "Streetscape Concept" Final Report

- Design Team propose schedule with final presentation to Council in early May. Billy suggests this needs to happen sooner. Mike will try to develop a different schedule and report back.

d) Design Contract

- Phil, Mike, and Chris have been working on this. Now that construction will get deferred until after council reviews the final report, we have more time to develop this.

4) CONCEPT UPDATE – THE SQUARE

- MIke questions if fountain improvements on west side are truly not desired by Museum.

- Billy suggests Musuem may be expecting to see more than Design Team shows on south side.

- Billy asks if west lawn can be used for activities. Neil suggests that the Square is perhaps not the appropriate place for this, and that Withers Park would be a better candidate.

- Billy is going to set up meeting with Museum so we can discuss these items.

5) IMPLEMENTATION UPDATE:

- a) City To Do List
 - i) Provide clarification on overall schedule

- Done

- b) CMT To Do List
 - i) Refine renderings (N Main Plaza, US-51B Areas, BCPA/Douglas Lots Area)
 - ii) Facilitate working session w/ Billy on streetscape treatments
 - iii) Ongoing work on final report

6) CRITICAL CITY DECISION POINTS:

- i) Direction on public input
 - Done
- ii) Direction on overall schedule

- Done

7) OPEN DISCUSSION / QUESTIONS





CORE TEAM BLITZ MEETING January 23rd, 2024 – 9:00 AM

MEETING MINUTES

1) 2nd Community Open House

- a) Confirm Date: February 13th, 4-6:30
 City confirmed this time will work.
- b) Confirm Content

- City good with content proposed, but Billy suggests adding a station for "Garbage Management".

c) Overall Map Review?
 - City and Design Team will meet in-person on February 5th to review the map and make any changes prior to the public meeting.

2) FINAL REPORT REVIEW

a) Who at CoB Reviews?

- Billy would like us to provide the final report to him and he will distribute to other departments as necessary.

b) Review Schedule

- Discussed getting this to the Council to adopt in March, but we want to make sure they have enough time to review – so probably looking at the first Council meeting in April. This will require an expedient review by the City.

3) 2ND CONTRACT

- a) Contract Date Logistics
 Billy will discuss with others how 2nd contract will be managed.
- b) DFE Extended Campaign

- Mike to set up meeting with Clear and Katherine to discuss next steps.

IDOT Meeting Minutes



IDOT/CoB – US-51B MEETING

December 5, 2022 – 2:00 PM



MEETING MINUTES

1) INTRODUCTIONS:

- a) City of Bloomington Billy Tyus, Kimberly Smith, Craig Shonkwiler, Phil Allyn
- b) CMT Roger Driskell, Chris Stritzel, Mike Sewell
- c) Massie Kent Massie
- d) IDOT Scott Neihart, Jeff Allen, Ryan Carroll, Jason Stults
- e) Hutchison Jim Burke, Brian Borgman

2) DOWNTOWN STREETSCAPE PROGRAM UPDATE:

- a) Opening Remarks from City Engineer/Assistant Public Works Director
- b) What We're Trying to Do
 - i) Overall streetscape program objective: Revitalize Downtown Bloomington
 - ii) Related to US-51B:
 - (1) Provide safe access for pedestrians and bicyclists into and out of the Downtown core
 - (2) Integrate community assets that are outside the "beltloop" into the Downtown environment
 - (3) Restore economic well-being to Downtown that is currently being choked off by highway

3) IDOT PHASE I UPDATE:

a) IDOT/Hutchison Update

- IDOT staff discuss status of Phase I and overall project. Hutchison is working on finalizing Intersection Design Studies (IDSs), and a public involvement meeting is planned for late 2023.

- IDOT believes cross-section for roadway is fairly "set", but willing to entertain other alternatives from City.

- Due to continued escalation of construction costs, IDOT staff concerned about limits of program funding as City considers introducing additional elements. IDOT can't exceed program funding, even if City driven improvements were to be reimbursed by the City later on.

- b) General Discussion of Opportunities to Team Up on Walkability Improvements
 - CMT discusses possible design opportunities to achieve City goals.
 - City wants to make sure critical community concerns are addressed with US51B improvements.
- c) IDOT's Design Schedule: When do you need things from us?
 CMT will work with City to develop specific concepts for IDOT to consider, and all parties will meet again week of February 20th to discuss further.
- d) IDOT's Construction Schedule: When will the US-51B improvements be implemented?
 Project has a planned January 2025 letting date.

4) NEXT STEPS

a) CoB/CMT action items

- CMT to work with the City on refining design concepts, and traffic models will be developed to ensure that reasonable/responsible alternatives are being proposed to IDOT.

- CMT to schedule follow meeting in February.
- b) IDOT/Hutchison action items
 - None at this time

5) DOWNTOWN WALKING TOUR



IDOT/CoB – US-51B MEETING #2

February 22, 2023 – 10:00 PM



MEETING MINUTES

1) IN ATTENDANCE:

- a) City of Bloomington Billy Tyus, Kimberly Smith, Craig Shonkwiler, Phil Allyn
- b) CMT Roger Driskell, Chris Stritzel, Mike Sewell, Brian Eads
- c) IDOT Clare Dietz, Scott Neihart, Jeff Allen, Ryan Carroll, Jason Stults
- d) Hutchison Jim Burke, Brian Borgman

2) APPROACH TO PROGRAM SCHEDULES

a) IDOT Phase I Program

- IDOT has a rigid schedule for their project they must adhere to, due to the use of Restricted Funding (part of the Capital Plan).

b) City of Bloomington Streetscape Program

- City wishes to respect the approved program that IDOT and Hutchison are committed to and not disrupt their process, but also wish to underscore the importance of their proposed US-51B improvements as part of the overall Downtown revitalization. City's approach would be to work with IDOT on ways we can minimize having to re-do improvements that the other party is planning to construct.

3) PRESENTATION OF CITY PROPOSED CONCEPT

a) Downtown Revitalization Goals

- Goals for Downtown extend beyond a simple "fresh coat of paint". Rather, the City is investing in potential changes to roadway infrastructure that elevate walkability and provide opportunities for increasing the "pubic sphere" (outdoor dining, public outdoor space, etc). US-51B currently severs the Downtown core from the surrounding community and threatens to undermine the economic viability of investments the City is proposing there. Additionally, there are currently legitimate safety concerns for pedestrians and bicyclists wishing to cross the four-lane highway.

b) Bicycle Path Concept

- Discussed preliminary concept proposing that the bike routes be relocated to the "inner loop" of Downtown rather than as proposed on the highway – both for safety reasons and for attracting bicyclists to Downtown.

c) Cross-Section Concept

- Discussed preliminary concepts that involve reducing the number of traffic lanes (in order to establish safer pedestrian and bicyclist crossings) and creating new opportunities for parking. CMT asserts that data provided by their traffic modeling supports consideration of these concepts.

4) INTEGRATION OF IDOT/CITY PLANS: NEXT STEPS

- a) CoB/CMT action items
 CMT to submit traffic modeling data and preliminary concept sketches to IDOT and Hutchison.
 b) IDOT/Hutchison action items
 - After receiving info from CMT, IDOT will consider these proposals, and all parties will then reconvene on March 30th to discuss IDOT's findings.

- Goal is to have IDOT provide a "preliminary blessing" on a concept so that CMT can then perform Intersection Design Studies (IDSs) for IDOT's further evaluation. The IDSs will facilitate IDOT's formal review of what the City is proposing.



IDOT/CoB - US-51B MEETING #3





MEETING MINUTES Revised 4/28/23

1) IN ATTENDANCE:

- a) City of Bloomington Craig Shonkwiler, Phil Allyn
- b) CMT Roger Driskell, Chris Stritzel, Mike Sewell
- c) IDOT Clare Dietz, Scott Neihart, Jeff Allen, Ryan Carroll, Jason Stults
- d) Hutchison Jim Burke, Brian Borgman

2) DISCUSSION ON PROPOSED CITY IMPROVEMENTS

a) Quick review of proposed improvements

- M Sewell summarizes elements proposed by City for US-51B: (1) reduce lanes down to two based on Design Team traffic modeling, (2) provide on-street parking on at least one side of the roadway, and (3) relocate the proposed bike path to the interior of Downtown Bloomington.

b) Relocation of Bike Route off US-51B

- M Sewell shares that Letters of Support from community bicycle organizations for relocation of bike path to the interior of Downtown Bloomington have been received by the City: Bike BloNo, Friends of the Constitution Trail, West Bloomington Revitalization Project Bike Co-op, and McLean County Wheelers.

c) Feedback from IDOT/Hutchison on design elements

- IDOT states that their desire is to stick with the proposed preliminary roadway section previously presented to the City as part of their Phase I effort: three lanes of traffic with an adjacent buffered bicycle lane.

- IDOT believes that a two-lane section would not hold up if modeled using IDOT's preferred Highway Capacity Software (HCS), including queued traffic backing into the previous intersection.

- IDOT states that it is not their policy to allow new on-street parking on State Routes.

- IDOT states that their Central Office bicycle coordinator, Steve Letsky, believes the bicycle lane should be located on US-51B.

- IDOT suggests a Jurisdictional Transfer (JT) could be entertained, but C Shonkwiler says that is not very likely unless it is for a limited section of US-51B. IDOT states that they would not consider a JT unless there is a "logical termini", such as another State Route.

- M Sewell states that the Design Team will need to meet with the City to discuss next steps.

d) Discuss process for design revisions, revised modeling, etc.

3) DISCUSSION ON INTEGRATION OF IDOT/CITY PLANS

a) Discuss scenarios/sequencing for integrating IDOT and City projects

b) Discuss Memorandum of Understanding – Content

- c) Discuss Memorandum of Understanding Timeline
- d) Discuss required IDSs

4) NEXT STEPS

- a) CoB/CMT action items
- b) IDOT/Hutchison action items

Steering Committee Meeting Minutes



DOWNTOWN STREETSCAPE DESIGN

FIRST STEERING COMMITTEE MEETING December 7, 2022 – 1:00 AM



MEETING MINUTES

1) ATTENDEES -

- a) City of Bloomington
 - i) Billy Tyus Deputy City Manager
 - ii) Phil Allyn City Traffic Engineer
 - iii) Kimberly Smith Assistant Economic & Community Development Director
- b) Design Team
 - i) Mike Sewell Project Manager/Point of Contact (Crawford, Murphy, & Tilly)
 - ii) Chris Stritzel Project Principle (Crawford, Murphy, & Tilly)
 - iii) Kent Massie Lead Designer (Massie Massie + Associates)
 - iv) Neil Brumleve Landscape Architect (Massie Massie + Associates)
 - v) Scott Swanson Project Architect (Workbench Architects)
 - vi) Kyle Glandon Project Architect (Workbench Architects)
- c) Steering Committee
 - i) Vicki Tilton Owner, Fox and Hound Day Spa
 - ii) Andy Shirk President, Beer Nuts
 - iii) David Braun General Manager, Connect Transit
 - iv) Andi Whalen Branch Manager, INB (and President, Local Jaycees)
 - v) Jamie Mathy Owner, Red Raccoon Games (and Former Council Member)
 - vi) Jan Lancaster Owner, The Bistro
 - vii) Pam Eaton Gallerist Eaton Studio Gallery
 - viii) Elmo Dowd Production Systems Development Manager, Ferrero
 - ix) Tony Grant Assistant County Administrator, McLean County

2) INTRODUCTION/PRESENTATION -

- a) See Exhibit 1 for PowerPoint presentation.
- b) General Discussion
 - i) See Exhibit 2 for notes on group discussion during presentation.
 - ii) Main Takeaways:
 - (1) Need to improve connections to adjacent neighborhoods and not just focus on downtown.
 - (2) Less planning/talking, more action! Eagerness to see this implemented and not just sit on the shelf once it reaches the City Council.
 - (3) Public involvement:
 - (a) There are now around 1,000 people living downtown and those people (and the larger community) need to be involved in discussions of what is planned for Downtown.
 - (b) That said, most are likely not interested in attending public meetings and consideration should be given to creating a website for this program or other online tools to solicit feedback.

3) SUBGROUP BREAK-OUT SESSIONS -

- a) Traffic & Parking Subgroup
 - i) See Exhibit 3 for notes on Traffic/Parking subgroup discussion.

- ii) Main Takeaways:
 - Perception is that parking downtown is stressful right now access difficult as well as challenging to find a spot on certain blocks. But still a general desire for wider walks and outdoor dining opportunities – even if it means displacing some parking that was immediately in front of businesses.
 - (2) Underground utilities need to be assessed prior to streetscaping improvements being constructed in order to avoid tearing up what was just built. For example, there may be power capacity issues with Ameren service for Downtown or old watermains that need to be replaced right now.
 - (3) Trees in Downtown are scraggly and not robust. They are also not symmetrical across each block. General desire for more greenspace Downtown.
 - (4) Mixed opinions in the Steering Committee on one-way versus two-way streets. One-way with a loading lane accommodates the many delivery trucks downtown, but two-way reduces frustrations with visitors attempting to park Downtown and slows traffic.
- b) Local Business Interests Subgroup
 - i) See Exhibit 4 for notes on Local Business Interests subgroup discussion.
 - ii) Main Takeaways:
 - (1) Outdoor dining has been a huge success, but the dining look is not the greatest.
 - (2) Outdoor concerts are a good addition to the Downtown scene.
 - (3) Concern due to absentee building owners and abandoned buildings contributing to property values going down.
 - (4) Trash collection needs a hard look. City cans are overflowing and dumpsters are in ill-advised locations.
 - (5) Perception of Downtown building unsafe and dirty.
 - (6) Need better notification/marketing of upcoming events.
- c) Cultural/Public Spaces Subgroup
 - i) See Exhibit 5 for notes on Cultural/Public Spaces subgroup discussion.
 - ii) Main Takeaways:
 - (1) Downtown was originally THE retail hub. How do we elevate this again?
 - (2) Downtown currently offers affordable rent and a variety of spaces. But some of the facilities (the Coliseum and Bloomington Center for the Performing Arts that aren't being used as they should – too expensive to rent and need managed differently.
 - (3) Need better utilization of existing green spaces such as the BCPA lawn and Museum Square.

4) MEETING WRAP-UP -

- a) Provide follow-up thoughts to Mike Sewell (CMT): msewell@cmtengr.com
 Cell: (309)386-0679 – call anytime
- b) Second Steering Committee Meeting to be scheduled for March '23. Doodle poll to follow.

5) Exhibits -

- a) Exhibit 1 PowerPoint Presentation
- b) Exhibit 2 Notes on Group Discussion During Presentation
- c) Exhibit 3 Notes on Traffic/Parking Subgroup Discussion
- d) Exhibit 4 Notes on Local Business Interests Subgroup Discussion
- e) Exhibit 5 Notes on Cultural/Public Spaces Subgroup Discussion

FIRST STEERING COMMITTEE MEETING

Downtown Streetscape Program

City of Bloomington, IL | December 7, 2022









WHO YOU'LL BE WORKING WITH





Workbench

Billy Tyus Craig Shonkwiler Melissa Hon Kimberly Smith

Deputy City Manager City Engineer Economic & Community Development Director Assistant Economic & Community Development Director

Mike Sewell Chris Stritzel Project Manager/Point of Contact Project Principle

Kent Massie Neil Brumleve

Lead Designer Landscape Architect

Scott Swanson Kyle Glandon Project Architect Project Architect

WHO WE'LL BE WORKING WITH

STEERING COMMITTEE

Vicki Tilton	Owner, Fox and Hound Day Spa
Andy Shirk	President, Beer Nuts
David Braun	General Manager, Connect Transit
Andi Whalen	Branch Manager, INB (and President, Local Jaycees)
Jamie Mathy	Owner, Red Raccoon Games (and Former Council Member)
Jan Lancaster	Owner, The Bistro
Pam Eaton	Gallerist Eaton Studio Gallery
Brandon Thornton	Learning Behavior Specialist, Bloomington High School
Elmo Dowd	Production Systems Development Manager, Ferrero
Tony Grant	Assistant County Administrator, McLean County



"Downtown Bloomington Strategy" – Adopted by City Council in 2013

Ultimate Goal: Develop an implementable multi-year plan for streetscaping enhancements in order to revitalize Downtown Bloomington

PROGRAM OBJECTIVES

- \checkmark Enhance the Streetscape Aesthetic
- ✓ Create/Enhance/Activate Public Spaces
- \checkmark Improve Walkability and Accessibility
- \checkmark Facilitate Greater use of Public ROW by
 - **Downtown Businesses**
- ✓ Bridge Downtown Core and Surrounding Neighborhoods
- \checkmark Integrate History into the Downtown Experience
- \checkmark Integrate Sustainability where Appropriate



PROGRAM SCHEDULE



WHAT WE'VE HEARD

"What are some of the biggest challenges/obstacles you feel that prevents Downtown Bloomington from reaching its full potential?"

- Government inaction lack of political will and funding
- Aesthetics of entry points into downtown are poor
- Lack of attractive businesses & lighting between parking garages and main strip
- Loitering and pan-handling discouraging to visitors
- Lack of unified sense of place Downtown different areas are disparate
- Downtown's reputation is primarily for nightlife not very family friendly
- Costs of upgrading buildings to ADA scares investors away
- Office traffic has declined, limiting daytime viability of ground floor retail
- Some building owners just "sitting on properties" rather than investing in them
- Poor promotion in advance of Downtown events
- Core services (e.g. trash collection) overlooked

WHAT WE'VE HEARD

"What do you believe would be the most impactful streetscape improvements in terms of generating more visitors to Downtown?"

- U.S. Route 51B road diet elevate pedestrians over vehicles
- Gateway project along Market Street to make Downtown entrance more inviting
- Enlarge the "public sphere" narrower road lanes and wider sidewalks
- Better utilization of alleys as restaurant/green space with festive lighting
- Make certain streets for pedestrians only
- More vibrant décor color, banners, lighting, creative/fun things, etc.
- Less hardscaping and more landscaping large, robust trees e.g.
- Creation of a central space for events/recreation destination for the community
- Multi-modal access improvements (e.g. bicycles) overcome reliance on cars
- Street furniture to encourage visitors to stay longer
- Traffic calming to promote safe walking Downtown

DISCUSSION GROUPS



TRAFFIC & PARKING

Mike Sewell/Craig Shonkwiler (Facilitators) David Braun Anthony Grant Jamie Mathy



LOCAL BUSINESS

Scott Swanson/Chris Stritzel (Facilitators) Elmo Dowd Vicki Tilton Jan Lancaster

SUBGROUP DISCUSSION:

First 10 minutes – "Past"

Second 10 minutes – "Present"

Final 10 minutes – "Future"

15 minute presentation to larger group – 3-5 Takeaways



CULTURAL/PUBLIC SPACES

Neil Brumleve/Kyle Glandon (Facilitators) Pam Eaton Andi Whalen Andy Shirk

WHAT'S NEXT

POINT OF CONTACT:

Mike Sewell (CMT) <u>msewell@cmtengr.com</u> (309)386-0679

- Provide any additional thoughts to Mike in the next week or so.
- We will provide Minutes from the meeting to all participants (including those unable to attend today)
- We will be in touch to schedule the 2nd Steering Committee Meeting (tentatively March '23)

THANK YOU!

Exhibit 2 – General Discussion Meeting Notes

December 7, 2022

Streetscape - Steering Committee Meeting #1

Downtown Bloomington, IL

- Looking to improve connections to neighborhoods just outside the downtown
- Need different touch points with the community (absentee business owners, bar owners vs. etc.). Involve other building owners sooner in the process (30% may have changed hands). Property owners vs. business owners. (Jamie)
- Brought up website idea (Connect)
- Pam Eaton sent out our questionnaire to 20 other people already
- There are now around 1,000 people living downtown and those people (and the larger community) need to be involved in discussions of what is planned for Downtown.
- That said, most are likely not interested in attending public meetings and consideration should be given to creating a website for this program or other online tools to solicit feedback.
- Some residents aren't interested (not an owner attitude)
- Some of the room doesn't think there are any new issues from previous studies
- Over signage
- Communication from City isn't perfect
- Message isn't consistent
- 50% of Connect Transit will be electric by 2024
- Need to future proof (extra conduits, etc.)
- Some pessimism in the room about whether or not we will deliver on our word
- Wayfinding sign tag line was "Dream Big"
- Signage are for visitors to town, not residents
- Pam wants us to get article out to Pantagraph requesting information from constituents

Exhibit 3 – Traffic & Parking Meeting Notes

December 7, 2022

Streetscape - Steering Committee Meeting #1

Downtown Bloomington, IL

Traffic and Parking

Facilitators: Phil Allyn (City of Bloomington), Michael Sewell (Crawford, Murphy, & Tilly) Team Members: David Braun (Connect Transit), Anthony Grant (McLean County), Jamie Mathy (Red Raccoon)

Past

- Mistakes made in past:
 - Abundant Life (flop house) negatively affecting neighborhood.
 - Asymetrical light poles not lined up.
 - Planting little trees vs larger, more robust. Don't provide shade. Lose parking for more shade
 - Four lanes of traffic. Concerns with older folks and also dropping people off on bus lines and having to cross highway.
- Good things in past:
 - One-way good for deliveries

Present

- Walker parking study didn't take into account people that were working remote from County. Anecdotally, Jamie is now having to park on upper level where he didn't have to before.
- Parking right outside front door is clientele dependent
- Most people fine parking within one or two blocks.
- Lot of anecdotally people do not like parking downtown because it's difficult. Stressful and anxiety.
- Weather considerations affect people's appetite for walking.
- Current wayfinding good for people that don't live here.
- Homeless people make visitors feel unsafe.
- Transient people downtown, sketchy west side.
- Blind spot people crossing over to BCPA. Fine line between protecting people from what they shouldn't be doing vs encouraging bad behavior it by putting in a protection.
Future

- Autonomous cars less people own cars. You get a subscription for Ford fleet to come pick you up. Less parking needed perhaps.
- Fix underground infrastructure first. Concern about utilities such as water mains that need replaced. But also Ameren has a capacity issue that needs discussed. Need to be fixing this in advance of big streetscaping work. Fiber providers are also the worst about digging things up.
- More businesses do seem to be coming downtown and giving reasons for people to come back downtown.
- Need digital information boards (wayfinding) not static. Events board. Right now are not dynamic. Need push frame that tells you restaurants.
- Nobody has ever complained about losing parking for the parklets (outdoor dining). Wider sidewalks
- 6 bus "trains" going away and moving lines to the outside highways.
- Frustrated with IDOT being reactive not proactive
- Discourage people from crossing US51
- Takeaways:
 - Parking problem right now (perception for visitors it's not fun and it's stressful)
 - But desire for wider walks and outdoor dining and future parking needs may not be as bad due to autonomous vehicles
 - Make sure underground utilities are thought through

Exhibit 4 – Local Business Interests Meeting Notes

December 7, 2022

Streetscape - Steering Committee Meeting #1

Downtown Bloomington, IL

Local Business Interests

Facilitators: Scott Swanson, Chris Stritzel Team Members: Elmo David, Vicki Tilton, Jan Lancaster

PAST DISCUSSION

- Lack of support for business owners from City
- No clear path for setting up building or business (Rueben Spice Works)
- Need single point of contact for business owners to help walk through the process
- Hard to promote downtown
- Told all the time it's entirely different in Normal (business relations)
- Pub crawl was a huge success no longer a thing
- Couldn't close the street down before
- Jazz & Blues (pub crawl) was for an adult crowd (not students)
- Lost of Osco/CVS, small grocers, etc.
- Parking tickets were ridiculous
- Churches used to have place for homeless

PRESENT DISCUSSION

- Outdoor dining = huge success
- Outdoor dining look is not the greatest
- Festivals on the square
- Student pub crawl students get bused in not notice to business owners (trash)
- Covid restrictions was tough as times
- Lost employees staffing issues especially in service industries
- Business is almost back to growing again
- Recession proof for the most point
- Switching business models (more adaptable)
- Retail outlets need to be good marketers
- Free parking during COVID (sign clutter)
- Bars are open to 1 during the week and 2 on the weekend
- Outdoor concerts are a good addition
- Problem is in the 500-block (college bars)
- Food truck vendors (stay until 5:30 AM)

FUTURE DISCUSSION

- Ordinance (Merl Huff Peoria) abandoned buildings
- Property values going down because of abandoned buildings
- Need to figure out parking (probably more of a perception)

- Need to up services for downtown residences
- Trash no garbage on the street (city cans are overflowing)
- Updated lighting
- Perception of being unsafe code blue stations
- Perception of being dirty
- Welcome wagon for new people in downtown (DBA)
- Would prefer outdoor sidewalk space over parking next to business
- People want to park right in front of the business
- Want downtown to be walkable, want family
- Want flexible use space like uptown Normal circle
- Need to notify owners of upcoming events
- Correct homelessness issues
- Promote mixed-use buildings
- Get the students out of town when bars are done

Exhibit 5 – Cultural/Public Spaces Meeting Notes

December 7, 2022

Streetscape - Steering Committee Meeting #1

Downtown Bloomington, IL

Cultural / Public Spaces

Kyle GlandonWorkbench ArchitectsNeil BrumleveMassie MassiePam EatonEaton GalleryAndi WhalenINB / JayceesAndy ShirkBeer Nuts

Past

- + Originally THE retail hub
- Coliseum, never worked, lack of communication, personal interests driving decisions, not big enough, no hotel, hockey venue without a feeder youth program
- One way streets
- Facade grants misappropriated or poorly utilized, little oversight on the program City made poor decisions
- + Second Pres. big addition / Holy Trinity / First Christian thriving church community downtown acoustics in their facilities are great
- + Library
- + string lights, benches, christmas decorations,
- inconsistent lighting levels
- + painted transformers
- + affordable rent, variety of spaces
- + neighborhood associations as affiliates
- + County Seat
- + Downtown was center of activity

Current

- + Library is filling void of culture for many coming out of COVID
- Mclean County History Museum needs more programming, more events to help operating deficits
- + New lighting is ok although illumination levels are inconsistent String lights are nice
- + The courthouse square serves as an event space this is a positive but can be improved upon
- Churches downtown haven't rebounded and aren't coordinating activities with downtown.
 Second Presbyterian congregation is way down. No longer biggest Youth program and therefore fewer families coming downtown
- No public toilets
- + Jefferson Street as the regular outdoor event space (suspect its the default and not critically evaluated-Kyle). Improve ease of use and supporting amenities.
- + BCPA acoustics are great
- Management of Coliseum and BCPA difficult. too expensive to rent
- + Parklets for restaurants, image of buildings spilling outside are really welcoming

- jersey barriers the City is providing for parklets aren't permitted to be personalized or beautified by businesses. difficult for businesses to get approvals. certain number per block, applications required, etc.
- + Grass around at Museum Square used (although not optimized) for seating
- Museum square World War memorial isn't seating although people need seating and it's an obvious place to sit.
- Safety perception of safety, lighting, storefronts need to be lit, paving uneven
- Recent site furnishing (trash, benches, etc.) were a nice addition

Future

- + Two way streets to slow traffic. Stop signs in lieu of traffic signals. Linked traffic signals allow drivers to speed. Eliminate one-way streets...confusing
- + Improved circulation (I believe this was automobiles-Kyle)
- + Better utilization of existing green spaces. Includes BCPA lawn (needs branding)
- + Warehouse district. Public works area south of project scope has tons of potential and interested parties (Tom Kirk, Hermes) should be emphasized and connected
- + Museum Square can be better optimized, needs a plaza cafe leased by 3rd party to encourage lingering.
- + Better activate roofs. Parking garage roofs should be programmed and landscaped. Pam had several programming suggestions.
- + Family oriented green space as destination and reason to linger. "Give me a reason to bring my family"
- + Connect transit location should be located closer to amenities that riders are using library, courts, jail, government facilities, etc (Pam Eaton suggested they're planning to put Connect in the wrong garage. Should be at the Front Street garage)
- How is new open space going to be used and maintained. Is it just going to be a place for trash to collect and dog poop. can the City manage a new public amenity in a way that it stays beneficial to the downtown experience.
- + Programmed events such as farmers markets, music festivals need to be expanded
- + Management of spaces need to be considered
- + Want more businesses spilling out into the sidewalk spaces
- + Want a consistent look for parklets
- + Shorten crosswalk distances to encourage walkability
- + County Museum space could work better in the future



DOWNTOWN STREETSCAPE DESIGN

2nd STEERING COMMITTEE MEETING March 7, 2023 – 2:30 PM



MEETING MINUTES

1) PROGRAM SCHEDULE UPDATE:

- Design Team has mostly completed the Data Collection Phase and has moved on to the Strategy Development Phase.

- Design program is on schedule for Fall '23 completion.

2) VALIDATION OF PREVIOUS FEEDBACK:

Briefly reviewed "Main Takeaways" from previous Steering Committee meeting in order to ensure that feedback from the committee is informing design decisions. Takeaways from Overall Group, Traffic and Parking Subgroup, Local Business Interests Subgroup, and Cultural/Public Spaces Subgroup were discussed.
Design Team shares how various discussion points have been incorporated into design decisions, such as (1) improving connections to adjacent neighborhoods, (2) greatly increased opportunities for outdoor dining, (3) consistency in parking layout and street sections to make parking less stressful, (4) review of utilities (private and public) to avoid tearing out recent improvements due to utility replacement projects, (5) incorporating more "greenspace" Downtown including tree symmetry, (6) use of streets to facilitate deliveries, (7) better facilities for outdoor concerts, and (8) better utilization of open spaces.

3) DISCUSSION OF PRELIMINARY CONCEPTS:

- Provided update on meetings with IDOT. IDOT appears to be receptive to various improvements proposed by Design Team on US-51B with goal of slowing down traffic, making crossings safer, inviting traffic into Downtown, and establishing "sense of arrival". Possible alternatives include lane reduction, curb bump outs, on-street parallel parking, and streetscape beautification.

Discussed potential location of bike lanes inside the Downtown core, rather than located on US-51B. Team has upcoming meeting planned with Bike BloNo and Friends of the Constitution Trail to further discuss.
Discuss alternatives for Main Street, Center Street, and East/West streets. Proposals include consistent parking layout, flexible use lane for bicycles and deliveries, and wider sidewalks for outdoor amenities.
Discussed alternatives for utilizing open space in Downtown Bloomington to enhance quality of life and attract more visitors.

4) OPEN FLOOR DISCUSSION / WRAP-UP

- 3rd Steering Committee Meeting to be tentatively scheduled for early June.



DOWNTOWN STREETSCAPE DESIGN

3rd STEERING COMMITTEE MEETING September 27th, 2023 – 2:30 PM



MEETING MINUTES

- 1) PROGRAM SCHEDULE UPDATE: - See attached presentation for details.
- 2) UPCOMING PUBLIC OUTREACH CAMPAIGN:

- Discussed goals of the campaign: (1) to inform and update, (2) to solicit feedback for critical design decisions, and (3) to build excitement/engagement in order to attract more feedback and public involvement.

- Discussed different elements of the campaign, including the upcoming community input workshop and project website. See attached presentation for more details.

- Discussed possibility of having a press conference to announce the campaign.

- Discussed plans for making sure all Downtown business owners are invited to the open house, invited to view the project website, and how we could encourage additional feedback from them.

3) UPDATE ON RECENT MEETINGS W/ COUNCIL & COUNTY:

Provided update on meetings with County: two meetings with McLean County Museum of History staff, one with County Administration staff, and one with County Property Committee. Reported that all meetings went very well. We incorporated some feedback from these meetings and concepts have been very well received. Property Committee asked "How can we help you make this happen?"
Provided update on meetings with City Councilmen during three-on-one meetings. With just a couple of exceptions, the Downtown project was very well received and councilmen appear very eager to see this project happen.

- 4) UPDATE ON US-51B: - 3rd Steering Committee Meeting to be tentatively scheduled for early June.
- 5) MATERIALS/COLORS/FIXTURES:

- 3rd Steering Committee Meeting to be tentatively scheduled for early June.

6) STEERING COMMITTEE MEETING #4:

- 3rd Steering Committee Meeting to be tentatively scheduled for early June.

7) OPEN FLOOR DISCUSSION / WRAP-UP:

- Several committee members discussed that there is already adequate parking around Downtown but visitors are unaware of these options. Need better education, parking wayfinding, etc.

- Several committee members discussed the need to communicate to the public that the proposed flex lane will accommodate delivery vehicles.



TODAY'S AGENDA

- Schedule Update
- Upcoming Public Outreach Campain
 - pdate on Recent Meetings w/ Council & C

unty

- Update on US-51B
- Materials/Colors/Fixtures...
- Meeing #4

SCHEDULE UPDATE

- October '23 Public Outreach
 Campaign, 4th Steering Committee
 Meeting, make critical design
 choices, and select 1st streetscape
 project
- March 2024 City adopts
 Streetscape Plan
- June 2024 Bid 1st streetscape project
- Fall 2025 1st project complete

Downtown Streetscape - Overall Schedule Updated 8/23/23					
Overall Streetscaping P	First Streetscape Project				
Prepare for Public Outreach: Develop website, video, secure Public Input Workshop venue, etc. AND host Steering Committee Meeting #3.	1.5 Months				
Milestone: Public Outreach Launch		10/9/2023			
Public Outreach: Solicit public input thru website and Public Input Workshop.	2 Weeks				
Make Critical Overall-Plan Design Decisions: Host Steering Committee Meeting #4 followed by Core Team Meeting.	1 Week		Determine What First Project Will Be – Additional time needed in addition to adjacent "Make 3 Weeks		
Milestone: Resume Design Efforts		10/30/2023	3x1s?		
Planning Production Phose: Prepare overall plan drawings and cost estimates. Concurrently, resolve outstanding design issues such as refuse management, gateway features, BCPA, etc.	3 Months		Milestone: 1 st Project Determined and Contract Preparation Begins	8	11/13/202
			1 ^{ef} Project: Contract preparation/ negotiation/getting on council agenda.	2 Months	
			Milestone: City Council Approves 1 st Design Contract		1/8/2024
Programming Phase: Design Team develops recommendations for phasing strategy. Then host Steering Committee Meeting #5 followed by Core Team Workshop to approve phasing strategy and finalize plan.	1 Month		1 [#] Project – Detailed Design (accelerated) : Survey, prepare construction documents, secure permits, prepare bid documents, etc.		
Milestone: City Council Adopts Plan		3/11/2024			
			Milestone: Bid 1 st Streetscaping Project		June '24
			1 st Project – Bidding Process: Pre-bid meeting, addendum, etc.	2 Months	
			Milestone: Award 1 st Streetscaping Project		August '2
			Milestone: Issue Notice to Proceed for 1 st Project		Septembe '24
			Construct 1 st Streetscaping Project.	3 Months, then break for winter, then 6 Months	AL AND
			Milestone: 1 st Streetscaping Project Complete	23	Septembe

PUBLIC OUTREACH CAMPAIGN

- Branding/Messaging
- Project Website
- "Hype Video"
- Community Input Workshop
- Social Organization Road Show







PROJECT WEBSITE



DOWNTOWN 🔤 EVERYONE



COMMUNITY OPEN HOUSE



Wednesday, October 11th from 4-7PM at the BCPA Ball Room



UPDATE ON US-51B





CLASSIC / TRADITIONAL

MODERN / CONTEMPORARY

HYBRID BLEND

STEERING COMMITTEE MEETING

Week of October 23rd

Draft Agenda:

Review of Public Feedba

Recommendations on c

Recommendation of 1st

esign decisions

reetscaping project

WHAT'S NEXT

POINT OF CONTACT: Mike Sewell (CMT) msewell@cmtengr con (309) 386-2619

to

Provide any additional though

 We will be doing a "make-up join today, so that all are ker

We will be in touch soon to Committee Meeting (tentative like in the next week or

on" with those unable to e loop.

dule the 4th Steering week of October 23rd)

THANK YOU!



DOWNTOWN STREETSCAPE DESIGN



4th STEERING COMMITTEE MEETING October 23rd, 2023 – 10:00 AM

MEETING MINUTES

1) REVIEW RESULTS OF PUBLIC INPUT (AS OF 10/20/23):

Street Section: Alternate B was the clear favorite with more than twice the votes of the second closest option. 84% of the votes favor a section that prioritizes walkability/pedestrian space over parking.
Street Style: Hybrid Blend was the clear favorite with almost twice the votes of the second closest option. Modern/Contemporary only received 11% of the votes.

2) FINAL RECOMMENDATION FROM COMMITTEE ON STREETSCAPE SECTION:

- Steering Committee voted unanimously for Alternate B.

- Alternate A: 0 votes.
- Alternate B: 9 votes.
- Alternate C: 0 votes.

- Design Team clarified that the proposed section for Center Street would be parallel parking on just one side of the road, in order to provide very nearly the same amount of pedestrian space as that provided by Main Street Alternate B.

- Design Team reaffirmed numbers shown on the DowntownforEveryone.com website, that include the aforementioned Center Street parking reductions:

- Current number of on-street parking spaces: 780
- Proposed number of on-street parking spaces: 720
- Current downtown public parking vacancy during peak hours: 64%
- Proposed downtown public parking vacancy during peak hours: 56%

3) FINAL RECOMMENDATION FROM COMMITTEE ON STREETSCAPE SECTION:

- Steering Committee voted unanimously for Hybrid Blend.

- Classic/Traditional: 0 votes.
- Hybrid Blend: 7 votes.
- Modern/Contemporary: 0 votes.

- Committee was told they would have a chance to review the proposed hybrid style once the Design Team develops it.

4) DISCUSSION ON 1ST STREETSCAPE PROJECT:

- No consensus on what the first Streetscaping project should be. Suggestions included the following:

- Focus on beautifying/expanding the Douglas Street lots first so that there is a place for people to park when parking spaces in the Downtown Core are displaced due to construction.

- Center Street needs more attention than Main Street.

- Stay away from Center Street during the construction of the new Transit Center, slated for Fall '24 construction through spring/summer of 2026.

- The Square project will make the biggest "splash".

- Focus on beautifying the corridors between the parking garages and the Square (Center from Monroe to Jefferson and Main from Front to Washington).

- City would have some PR to do if our first project was anything other than the Square or Main Street Alt B improvements, since that is what the public is most excited for.

- Focus on the Market Street corridor from Roosevelt all the way to and including the Douglas Street lots.

5) OPEN FLOOR DISCUSSION / WRAP-UP:

Several committee members discussed that there needed to be more "icing" to the parking in order to make it attractive to visitors: well-lit and change from 90 minute parking to 2 hours. There was some discussion about businesses validating parking. Many business owners prefer metered parking.
There was some discussion that the use of flex lanes precludes the possibility of some customers getting drive-up service (such as parking adjacent to a coffee shop and running in to get a coffee you ordered in advance).

- There was not time to discuss the proposed garbage management plan. This will be deferred to the next Steering Committee Meeting.



A long time ago, in a Downtown not too far away...



Now hold on a minute...



1950s

CMT Get country.

1983

TODAY'S AGENDA

Review Results of Rublic Input

Final Recommendation from Committee on Streetscape Section

nal Recommendation from Commit

Discussion/Recommendation on

Update/Discussion on Garbage Situa

ectscape Project

allo

on

reetscape Style

PUBLIC INPUT RESULTS (as of 10/20/23)



Alternate A	35
Alternate B	127
Alternate C	61



Classic/Traditional	77
Modern/Contemporary	24
Hybrid Blend	130

FINAL RECOMMENDATION FROM STEERING COMMITTEE ON STREET SECTION

Alternate A – Maintains Current Level of Parking/ Least Pedestrian Space



Alternate B – Improves Walkability of Downtown



Alternate C – Establishes the Greatest Amount of Pedestrian Space / Least Parking



Website: https://downtownforeveryone.com/feedback/

FINAL RECOMMENDATION FROM STEERING COMMITTEE ON STREET STYLE

Classic / Traditional



Modern / Contemporary



Hybrid Blend



Website: https://downtownforeveryone.com/feedback/



GARBAGE SITUATION/UPDATE

CONSIDERATIONS / CRITERIA –

- Minimize visibility of trash along streets...
- Minimize smell of trash along streets...
- Maximize screening of dumpsters...
- More laborious pickup process acceptable if results in desired placement...





GARBAGE SITUATION/UPDATE



DOWNTOWN 🔤 EVERYONE

GARBAGE SITUATION/UPDATE






POINT OF CONTACT Mike Sewell (CMT) msewell@cmtengr.co (309) 386-679

Provide any additional the or so.

We will be doing a "make to join today, so that all a

Steering Committee Meeting

sion" with those unable in the loop.

ke in the next week

5 date TBD

S

THANK YOU!



DOWNTOWN STREETSCAPE DESIGN



5th STEERING COMMITTEE MEETING December 19th, 2023 – 1:00 PM

MEETING MINUTES

1) REVIEW & DISCUSS "HYBRID STYLE":

- Steering Committee generally in favor of the hybrid style shown. (See attached presentation.)

- Concerns about material durability. Design Team responds that clay pavers proposed are much more durable than similar materials, such as concrete pavers. Design Team will avoid selecting furnishings that include wood, in order to avoid burdening City staff with maintenance issues.

- Committee member suggests exploring use of planters made of recycled wind turbine blades. This fits in well with Design Team's sustainability initiatives for this project.

- Asked the Committee for their preference on color scheme (see attached presentation).
 - Tan Blend: 3 votes.
 - Red Blend: 7 votes.

2) PRESENT CONCEPT UPDATES:

- Steering Committee in strong support of the concepts shown (see attached presentation):

- Center Street
- East-West Streets
- North Main Plaza revisions
- Bloomington Center for Performing Arts/Douglas Lots Area
- US-51B Areas, including Withers Park

- Some discussion about the large radio tower Downtown (the "Eiffel Tower"). Committee likes the idea of making a community asset out of the tower. Pending owner approval, Committee agrees with Design Team's suggestion of ornamenting the tower with lights and turning it into a valuable landmark.

3) REVIEW & DISCUSS PROJECT PHASING:

- Steering Committee generally in favor of the project phasing shown. (See attached presentation.) Design Team explains that phasing plan is very much subject to change based on unforeseen funding, new building developments, etc.

- Design Team explains rationale of proposed phasing:
 - Stay away from Center Street until after transit center construction.
 - Save US-51B for last (almost), and focus on interior first due to dynamic with IDOT.
 - Prioritize BCPA area in order to establish more parking spaces.
 - Alternate streets so that detours during construction are nevermore than a block.
 - Save side streets outside of the beltloop for last.

4) REVIEW & DISCUSS GARBAGE MANAGEMENT PLAN:

- Design Team explains rationale for proposed garbage management plan:
 - Minimize visibility of trash along streets.
 - Minimize smell of trash along streets.
 - Maximize screening of dumpsters.

- More laborious pickup process acceptable to the City if it results in desired placement of receptacle.

- Steering Committee generally in favor of the garbage management plan shown.

DOWNTOWN FOR EVERYONE Steering Committee Meeting #5 12/19/23





TODAY'S AGENDA

 Review & Discuss "Hybrid Style" Present Concept Updates Review & Discuss Project Phasing Review & Discuss Garbage Management Plan



HYBRID BLEND





HYBRID – Pavement Colors

Option 1 – Tan Blend





Option 2 – Red Blend











HYBRID – Option 1 (Tan Blend)







HYBRID – Option 2 (Red Blend)





HYBRID – Option 1 (Tan Blend)







HYBRID – Option 2 (Red Blend)





HYBRID – Option 1 (Tan Blend)







HYBRID – Option 2 (Red Blend)





HYBRID – Materials





Round

Square

Shapes





Lighting (Repurposed)

Planters



Bicycle Racks

Benches







CONCEPT UPDATES

Center Street Comparison
East-West Streets
North Main Plaza
BCPA/Douglas Lots Area
US-51B Areas





CENTER STREET COMPARISON



Main Street



Center Street



EAST-WEST STREETS

W. JEFFERSON STREET - 300 | Roosevelt St. to Madison St.





Parking Calculations

- 4 Existing Parking Spaces
- 7 Proposed Parking Spaces































BCPA/DOUGLASLOTS









BCPA/DOUGLASLOTS







N. MADISON STREET - 500 | Market St. to Mulberry St.





















S. EAST STREET - 200 | Olive St. to Grove St. | CONCEPT



Bloomington Illinois



STREETSCAPE SUMMARY

Existing

0' 40' 🏈

Proposed

Traffic Lanes = 4 northbound Traffic Lanes = 2 northbound Turn Lanes = None Boulevard = None Pedestrian Zone = 8' wide

Turn Lanes = 1 into garage Boulevard = 10'-14' wide (turf) Pedestrian Zone = 8'-12' wide







Bloomington Parks and Recreation















PROJECT PHASING



PHASING CONSIDERATIONS:

1. Stay away from Center Street until after transit center construction.

2. Save US-51B for last (almost), and focus on interior first due to dynamic with IDOT.

3. Prioritize BCPA area in order to establish more parking spaces.

4. Alternate streets so that detours during construction are never more than a block.

5. Save side streets outside of the beltloop for last.



GARBAGE MANAGEMENT PLAN

CONSIDERATIONS / CRITERIA –

- Minimize visibility of trash along streets...
- Minimize smell of trash along streets...
- Maximize screening of dumpsters...



More laborious pickup process acceptable if results in desired placement...







GARBAGE MANAGEMENT PLAN

TO: 1



Bigbelly

(

Bigbelly Smart Max

FULLY ENCLOSED

() CLEAN™ CONNECTED



TRASH MANAGEMENT SKETCH 2023-11-08 **CMT** Citibin containers placed in alleys where possible Dump truck uses flex lane during hours when bicyclists are minimal Public trash cans to be BigBelly compactors in order to minimize required pickups






GARBAGE MANAGEMENT PLAN







WHAT'S NEXT

 Provide any additional thoughts to Mike in the next week or so.

Future Steering Committee meetings??



POINT OF CONTACT: Mike Sewell (CMT) msewell@cmtengr.com (309) 386-0679

THANK YOU!



5) OPEN FLOOR DISCUSSION / WRAP-UP:

- Billy Tyus discussed need to keep Steering Committee involved beyond adoption of the plan and during design and construction of individual projects. Committee can provide invaluable input and support.

Utility Company Meeting Minutes



DOWNTOWN STREETSCAPE DESIGN



AMEREN-CITY MEETING March 17, 2022 – 10:30 AM

MEETING MINUTES

1) Attendance:

- Craig Shonkwiler City Engineer, City of Bloomington
- Michael Sewell Project Manager, Crawford, Murphy, & Tilly, Inc.
- Abby Helm Community Relations Coordinator, Ameren
- Emily Moore Engineer, Ameren
- Andrue Zulueta Engineer, Ameren
- Dean Thompson Engineer, Ameren
- 2) Overview of Streetscaping Project
 - M Sewell and C Shonkwiler give overview of project and approximate program timeline.

- M Sewell discusses potential impacts to Ameren facilities due to utility work, street construction, and/or ADA grade correction. A Zulueta discusses how City franchise agreement generally requires Ameren to bear costs of relocation/adjustments due to this work, but details to be worked out as design progresses.

3) Review of Upcoming Ameren Projects

- E Moore shares information on upcoming facility replacement projects in Downtown Bloomington over next 5-10 years. Some are manhole replacements (containing splices) while some are vault replacements (containing transformers and venting). It is approximately \$250,000 to replace a manhole. Vault replacement costs are even higher.

- Attendees discuss need for coordinating streetscaping construction schedule with Ameren replacement schedule in order to avoid tearing up recently installed streetscaping improvements.

 4) Review of Potential "Tap-On" Locations for Future Electronic Vehicle (EV) Charging Stations
 D Thompson shares location of two 277/480V spot locations (required for any Level 3 EV stations) Downtown. Remainder of Downtown power grid is on a 208V network.

- A Zulueta discusses that locating Level 3 EV charging stations elsewhere will require extension of 480V service, the cost of which is not covered by the City's franchise agreement. Alternatively, the City can install a dry-mount transformer to step-up voltage anywhere a Level 3 charger is desired but where only 208V service is available. This cost would likewise not be covered by the City's franchise agreement.

5) Review of Potential Light Fixture Replacements

- C Shonkwiler shares that many light fixtures will likely be replaced as part of streetscaping projects.
Majority of lights on US-51B are Ameren owned, while majority inside the "beltway" are City owned.
- A Helm shares that Ameren has a program in place to replace ~1,500 new bulbs with LEDs throughout
Bloomington in June/July '23. But A Zulueta says they will likely proceed with that work and simply salvage the new bulbs whenever a streetscaping project that involves street light replacement occurs.

- A Zulueta states that if new lights are added (not just replacing existing light fixtures in the same location) and/or pigtails added for electrical outlets on poles then this will need to be metered.

6) Review of Potential Relocation of Overhead Power Lines

- M Sewell shares concerns about overhead wire "pollution" downtown and desire to bring underground during streetscape constructions. A Zulueta shares that majority of Ameren's power grid is already underground.

7) Brief Discussion on Solar Buy-Back Opportunities Downtown

- A Zulueta states that Downtown Bloomington is currently not well equipped to accommodate solar buyback because most customer services are linked to a multi-transformer network with protectors that prevent reverse flow. This isn't an insurmountable situation, but is made more difficult due to the system being a linked network versus single spot network.

8) Discuss Nexst Steps

- E Moore will share marked up map(s) with CMT sharing location of facilities and schedule for planned projects.

- Once CMT/City have a more developed design, they will be in contact with Ameren to coordinate schedule for future construction projects.



DOWNTOWN STREETSCAPE DESIGN



NICOR-CITY MEETING March 31, 2023 – 8:30 AM

MEETING MINUTES

1) Attendance

- Craig Shonkwiler City Engineer, City of Bloomington
- Michael Sewell Project Manager, Crawford, Murphy, & Tilly, Inc.
- Bernie Anderson Regional Manager, Nicor Gas
- Jeremy Burton Field Operations Supervisor, Nicor Gas
- Mark Hylton Supervisor Bloomington Field Operations, Nicor Gas
- 2) Overview of Streetscaping Program
 - a) Scope of Project
 - M Sewell provides brief overview of program and typical improvements.
 - b) Anticipated Schedule
 M Sewell provides brief overview of schedule but notes that phasing of individual projects is yet to be determined.
 - c) Phasing TBD

3) Review of Upcoming Nicor Projects

a) 5-10 Year Infrastructure Replacement Plan?

- Nicor notes that most of the mains Downtown have recently been replaced. Other mains, even those that are as old as the 1970s, should last indefinitely, Nicor says. That said, Nicor plans to reach out to others in the company to see if there are any corporate plans for replacing any of these mains and, if so, might these be expedited to avoid conflict with the Streetscaping Program.

b) Aged infrastructure?

- Nicor notes that there should be no capacity issues in Downtown Bloomington. Even should the Downtown Transfer Station for Connect Transit be constructed, there should not be any capacity issues.

4) Integration of Streetscaping Program and Nicor Projects

- a) Discuss franchise agreement and process for dealing with project conflicts
 B Anderson acknowledges cost of relocation of Nicor utilities due to conflict with City infrastructure would be borne by Nicor.
- b) Discuss meters/regulations and vent piping and implications for relocation

- B Anderson states that meters are located in basements, not vaults, and consequently we can't eliminate the vent piping.

c) Discuss typical costs/schedule for relocation of Nicor pipes/facilities

- Nicor notes that mains are at 60 psi pressure and located 2-1/2' to 3' below the surface. Services are about 18" below the surface.

- Nicor notes that valve lids can be easily adjusted but that some valves have been buried by City asphalt paving operations.

5) Discuss Next Steps

- Nicor to let City and CMT know if there are any plans for replacing any of the Downtown mains.

6) Site Visit



DOWNTOWN STREETSCAPE DESIGN



FRONTIER-CITY MEETING June 8, 2023 – 1:00 PM

MEETING MINUTES

1) Attendance

- Mike Sewell Crawford, Murphy, & Tilly, Inc., Project Manager
- Craig Shonkwiler City of Bloomington, Assistant Public Works Director
- Adam Gangloff Frontier, Bloomington Supervisor

2) Overview of Streetscaping Program

- Mike provides an overview of the program to Adam.

- a) Scope of Project
- b) Anticipated Schedule
- c) Phasing TBD

3) Review of Upcoming Frontier Projects

a) 5-10 Year Infrastructure Replacement Plan?

- Adam shares Frontier's plan to provide Fiber To The Home (FTTH) to all homes in Bloomington-Normal, but notes that the schedule is on hold. 1st Phase was to consist of running fiber to single-family homes. The 2nd Phase would include multi-family homes Downtown. All fiber lines come out of Frontier's downtown building on Market and East Street.

- Adam will attempt to provide the City with a copy of the mapping for this plan. (On June 9th, Adam provided a preliminary plan of the FTTH to Mike and Craig.)

b) Aged infrastructure?

- Most of Frontier's infrastructure Downtown is very old, Adam says. Much of it is old copper phone lines, and Frontier does not have much incentive to be proactive in the replacement of those lines due to the business decision to move away from that service in favor of fiber lines. Most old lines are simply abandoned in place.

- Most of the conduit is located in the center of roads and located 3-4' deep. Most infrastructure is below ground and not overhead.

4) Integration of Streetscaping Program and Frontier Projects

- a) Discuss franchise agreement and process for dealing with project conflicts
- b) Discuss typical costs/schedule for relocation of Frontier facilities
 Adam suggests it depends on the specific conflict.
- c) Project specific coordination process

- Besides the aforementioned FTTH program, most of Frontier's work is focused on putting in new services based on orders from new customers.

- Adam asks that we alert Frontier during construction if we notice any issues (such as settlement around Frontier manholes).

- Adam also suggests that many adjustments to Frontier infrastructure due to City construction are relatively easy to implement (such as raising or lowering manhole lids).

5) Discuss Next Steps

- When the detailed design of specific streetscaping projects are initiated, the City will reach back out to Frontier to coordinate any potential conflicts.

Advocacy Groups Meeting Minutes



DOWNTOWN CONCEPT DESIGN

BICYCLE MEETING March 22, 2023 – 10:00 AM



MEETING MINUTES

1) Attendance:

- Craig Shonkwilker City Engineer, City of Bloomington
- Phil Allyn Traffic Engineer, City of Bloomington
- Michael Sewell Project Manager, Crawford, Murphy, & Tilly, Inc.
- Patrick Dullard Treasurer, Bike BloNo
- Dan Steadman Manager, West Bloomington Revitalization Project Bike Co-op
- 2) Overview of Streetscaping Project:
 - M Sewell and C Shonkwiler give overview of project and approximate program timeline.
- 3) Review of Potential US-51B Alternatives:

- M Sewell provides update of conversations with IDOT and shares IDOT's proposal to place a bike lane adjacent to three lanes of traffic on US-51B. P Dullard and D Steadman suggest that this feels unsafe without a physical barrier protecting the bike lane from lanes of traffic.

- M Sewell presents Design Team proposal for reducing to two lanes on US-51B with bike path moved to dedicated "flex lanes" in the Downtown interior. P Dullard and D Steadman are in favor of this idea. They do comment that bike commuters traveling past Downtown Bloomington on US-51B are unlikely to go out of their way to make use of the flex lanes in the Downtown interior, but that they would likely feel comfortable sharing the street lanes on US-51B due to the traffic calming impact of reducing to two lanes.

4) Discuss Next Steps

- M Sewell shares that IDOT has requested letters of support for the proposed bike path relocation off US-51B from the community bike organizations. P Dullard suggests he will likely be able to provide these letters from Bike BloNo, Friends of the Constitution Trail, West Bloomington Revitalization Project Bike Coop, and McLean County Wheelers.



DOWNTOWN STREETSCAPE DESIGN

Life Center for Independent Living Meeting June 8, 2023 – 10:15 AM



MEETING MINUTES

1) INTRODUCTIONS

- a) Mike Sewell Crawford, Murphy, & Tilly, Inc., Project Manager
- b) Craig Shonkwiler City of Bloomington, Assistant Public Works Director
- c) Conan Calhoun LIFE CIL, Advocacy & Advancement Director

2) BRIEF SUMMARY OF THE STREETSCAPING PROJECT

- Conan was already familiar with the plan for the streetscaping project.
- All present view this project as an opportunity to make Downtown ADA accessible.
- a) Goals & Objectives
- b) Project Limits
- c) Process & Schedule

3) DOCUMENTATION OF NON-CONFORMING ADA ELEMENTS

- a) Current Study: High level review only
- b) Future Detailed Design Projects: High accuracy topographic survey

4) INCORPORATION OF ADA ELEMENTS

- a) Standard Design Practice: Title II, 2010 ADA Standards
- b) Approach for Unique/Challenging Situations
 - Conan provides commentary on various options used in other communities, including use of ramps/handrails and steps.

5) OPEN FLOOR TO VOICE ADA CONCERNS

Conan shares that the disabled community view Downtown as very unfriendly to those with disabilities. Many people have never been able to visit certain restaurants and shops due to being inaccessible.
Conan remarks that an accessible outdoor environment is insufficient – accessibility improvements must also be made to the inside of businesses, or Downtown remains inaccessible. Mike remarks that there will need to be a bigger conversation related to these barriers, because the current scope is limited to streetscaping improvements. Conan suggests promotion of and adjustments to the Rust Grant in order to incentivize building improvements.

6) MEETING WRAP-UP / NEXT STEPS DISCUSSION

- Craig notes that we will keep Conan involved, but additional input is likely unnecessary until we move to the detailed design of specific streetscaping projects.

Letters of Support from Bike Advocacy Groups



March 29, 2023

Mr. Craig Shonkwiler, City Engineer City of Bloomington, IL

RE: Proposed Bicycle Lanes in Downtown Bloomington

Dear Mr. Shonkwiler:

During our meeting with you and representatives of the Downtown Bloomington Streetscape Core Team on March 22nd, 2023, you presented our organization with information related to the proposed streetscaping improvements in Downtown Bloomington. This included a discussion on the location of bicycle routes in the area.

We understand that the "City of Bloomington Bicycle Master Plan" (adopted in May 2015) recommends locating a bike route on US-51B. We further understand that the Illinois Department of Transportation (IDOT) intends to establish bike lanes on said highway in accordance with the bicycle master plan. However, at our meeting your team proposed to instead relocate the bike route to the interior of Downtown on proposed flexible use lanes constructed as part of the streetscaping program. We are in agreement that doing so would have several benefits over the original proposal including (1) improved safety and (2) access to bike amenities and points of interest that will be constructed as part of the Downtown streetscaping program.

In short, Bike BloNo is in agreement that relocating the bike route from US-51B to the interior of Downtown would be in the best interest of the bicycle community and support any efforts you may undertake with IDOT in order to see this accomplished.

Sincerely, Patrick Dullard Treasurer Bike BloNo



roots pride vision West Bloomington revitalization project

3/27/2023

Mr. Craig Shonkwiler, City Engineer

City of Bloomington, IL

RE: Proposed Bicycle Lanes in Downtown Bloomington

Dear Mr. Shonkwiler:

During our meeting with you and representatives of the Downtown Bloomington Streetscape Core Team on March 22nd, 2023, you presented our organization with information related to the proposed streetscaping improvements in Downtown Bloomington. This included a discussion on the location of bicycle routes in the area.

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In short, The West Bloomington Revitalization Project Bike Co-op is in agreement that relocating the bike route from US-51B to the interior of Downtown would be in the best interest of the bicycle community and support any efforts you may undertake with IDOT in order to see this accomplished.

Sincerely, Dan Steadman WBRP Bike Co-op



March 29, 2023

Mr. Craig Shonkwiler, City Engineer City of Bloomington, IL

RE: Proposed Bicycle Lanes in Downtown Bloomington

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We understand that the "City of Bloomington Bicycle Master Plan" (adopted in May 2015) recommends locating a bike route on US-51B. We further understand that the Illinois Department of Transportation (IDOT) intends to establish bike lanes on said highway in accordance with the bicycle master plan. However, at our meeting your team proposed to instead relocate the bike route to the interior of Downtown on proposed flexible use lanes constructed as part of the streetscaping program. We are in agreement that doing so would have several benefits over the original proposal including (1) improved safety and (2) access to bike amenities and points of interest that will be constructed as part of the Downtown streetscaping program.

In short, Friends of the Constitution Trail is in agreement that relocating the bike route from US-51B to the interior of Downtown would be in the best interest of the bicycle community and support any efforts you may undertake with IDOT in order to see this accomplished.

Sincerely, Patrick Dullard President Friends of the Constitution Trail



March 29, 2023

Craig Shonkwiler, City Engineer City of Bloomington 109 E Olive St Bloomington, IL 61701

RE: Proposed Bicycle Lanes in Downtown Bloomington

Dear Mr. Shonkwiler:

At a recent meeting of representatives of the Downtown Bloomington Streetscape Core Team on March 22nd, 2023, you presented information about the proposed streetscaping improvements in Downtown Bloomington. This included a discussion on the location of bicycle routes in the area.

We understand that the "City of Bloomington Bicycle Master Plan" (adopted in May 2015) recommends locating a bike route on US-51B. We further understand that the Illinois Department of Transportation (IDOT) plans to establish bike lanes on US-52B in accordance with the bicycle master plan. However, at the recent meeting, your team proposed to relocate the bike route through the interior of Downtown on proposed flexible use lanes, constructed as part of the overall streetscaping program. We are in agreement this plan as doing so would have several benefits over the original proposal: (1) improved safety, and (2) access to bike amenities and points of interest that will be integrated into the Downtown streetscaping program.

The McLean County Wheelers, a cycling club with over 200 McLean County cycling enthusiasts, agrees that relocating the bike route from US-51B to the interior of Downtown would be in the best interest of the bicycle community and we support your efforts with IDOT to see this accomplished. Sincerely,

Kellie Williams, President McLean County Wheelers mcleancountywheelers@gmail.com

McLean County Property Committee Meeting Minutes

Minutes of the Property Committee Meeting

The Property Committee of the McLean County Board met on Thursday, August 10, 2023, at 5:00 p.m. in Room 404 of the Government Center Building, 115 E. Washington Street, Bloomington, Illinois.

Members Present: Chair Val Laymon; Members Jim Soeldner, Geoff Tompkins, Corey Beirne Members Absent: Members Lyndsay Bloomfield **Other Members** Present: None Staff Present: Cassy Taylor, County Administrator; Anthony Grant, Assistant County Administrator; Cathy Dreyer, Assistant County Administrator; Taylor Williams, Assistant State's Attorney; Julie Morlock, Recording Secretary. Department Heads/ Elected Officials Present: Joseph Gaither, Facilities Management; Mike Steffa, Director of Parks and Recreation Others Present: Trevor Sierra, Assistant State's Attorney; Dan Leary, Information Technology

Chair Laymon called the meeting to order at 5:10 p.m. and declared a quorum.

Chair Laymon presented for approval the consent agenda including minutes from the July 6, 2023 regular meeting as well as the bills in the amount of \$275,730.83.

MCLEAN COUNTY BOARD COMMITTEE REPORT PAGE 1 OF 7

AS OF 7/31/2023 EXPENDITURE SUMMARY BY FUND

Property Committee

FUND	FUND TITLE	PENDING TOTAL	PREPAID TOTAL	FUND TOTAL
2000	GENERAL FUND		\$275,730.83	\$275,730.83
			\$275,730.83	\$275,730.83

Motion by Beirne/Tompkins to approve the Consent Agenda including minutes from the July 6, 2023, regular meeting as well as bills in the amount of \$275,730.83. Motion Carried.

Chair Laymon confirmed with Ms. Taylor there were no requests for appearances by members of the public or County employees.

Chair Laymon introduced Mr. Mike Sewell, project manager for the City Streetscape project. He noted they had met with County Museum staff and County Administration but Cassy suggested meeting with the Property Committee to get further input before starting public outreach. Mr. Sewell presented information on the City of Bloomington's Downtown Property Committee August 10, 2023 Page **2** of **3**

Streetscape including an overview of the project, schedule update, concepts and upcoming public outreach campaign.

Mr. Beirne asked for more information about the data they had collected. Mr. Sewell noted they reviewed traffic numbers and information, utility information and consulted with some downtown businesses. He noted the streetscape would involve a collection of all the sidewalks, paths, landscaping, and decorative lighting to enhance the pedestrian experience along the streets. Mr. Sewell indicated they were in-between the strategy phase and design phase and wanted to adopt a design late this year or early next year. Ms. Laymon asked if they were getting input from the public on access to downtown. Mr. Sewell indicated they had set up a steering committee to discuss needs and wants and the questions posed to the public would not be general but more specific.

Mr. Sewell showed concept pictures that illustrated a people centric instead of vehicle centric downtown. Mr. Beirne asked about traffic flow. Mr. Sewell confirmed they were looking at reducing vehicle traffic to one lane with a possible bicycle lane. Ms. Laymon noted the change to make the square street level and asked about ADA compliance. Mr. Sewell confirmed all plans were ADA compliant. Mr. Tompkins noted more pedestrian friendly but Illinois had about 7 months of winter and asked how that played into the planning. Mr. Sewell agreed that people are less likely to walk around in bad weather. Ms. Taylor noted that in previous discussions they had discussed snowman competitions, sleigh rides and other events that would involve cold weather activities that might encourage people to come to the square. Mr. Soeldner asked if security and car access during possible public protests had been considered. Mr. Beirne asked if they had considered closing some of the street's vehicle traffic. Discussion of options for security, deliveries and closures.

Mr. Tompkins asked about the balance between business utilization of the area and citizens' use of the property. Mr. Sewell noted that the City issues permits for outside dining but that they were taking into consideration creating enough space for businesses and people. Ms. Laymon asked how the court house would be affected by extreme rain events with the different landscape grading. Mr. Sewell indicated there would be some challenges but those could be addressed. Ms. Laymon asked if there would be any structural changes to the Courthouse. Mr. Sewell confirmed there would not.

Mr. Sierra asked for the physical parameters of the project. Mr. Sewell showed the boundaries for the project on a map. Mr. Soeldner asked if they had considered restroom facilities for the increased number of people visiting the downtown area. Mr. Sewell agreed there are nicer portables available, but they would need to look at options for permanent facilities. Mr. Tompkins asked how the County could help with the project. Mr. Sewell indicated they appreciated the support and would appreciate members getting information out to the public once they start up the public input portion of the project. Mr. Sewell confirmed the members were ok with them sharing the draft concept drawings with the public. Members concurred. Ms. Laymon thanked him for coming and providing information.

Chair Laymon presented on behalf of Facilities Management a request to approve an Amendment to Agreement for Boiler Replacements. Mr. Gaither went over the project. Ms. Laymon asked him to confirm efficiency would be increased. Mr. Gaither confirmed.

Property Committee August 10, 2023 Page **3** of **3**

> Motion by Tompkins/Beirne to recommend approval of an Amendment to Agreement for Boiler Replacement. Motion Carried.

Mr. Gaither presented the monthly report for the Facilities Department. Mr. Beirne asked about the sewer smell. Mr. Gaither provided an update on steps being taken to eliminate the smell. Mr. Beirne if Mr. Gaither could find out what it would take for the clocks on the museum tower to be operational. Mr. Gaither indicated he would research how the clocks worked and possible repairs. Chair Laymon asked if there were any additional questions or comments; hearing none, she thanked him.

Mike Steffa, Direct of the Parks Department provided his monthly report to the Committee. He provided an update on the bait shop project. Mr. Steffa discussed a long-term project they were starting with the goal of improving the water quality of the lake. Ms. Laymon asked how they would eliminate the invasive plant life. Mr. Steffa went over the process. Mr. Soeldner asked him to confirm mostly mechanical removal and chemicals only on specific spots. Mr. Steffa confirmed and noted the safety of chemicals used. Ms. Laymon asked for an update on the beavers at the park. Mr. Steffa noted he had been in contact with several individuals including the Illinois Beaver Alliance. He noted there are grant opportunities to help with the situation and went over work that could be done to deter the beavers. Ms. Laymon asked about getting volunteers. Mr. Steffa confirmed they would ask for volunteers. Mr. Beirne noted another beaver situation in town. Mr. Steffa indicated he would reach out to get more information on how they were handling that situation. Chair Laymon asked if there were any further questions; hearing nothing, she thanked Mr. Steffa.

Cassy Taylor, County Administrator provided an update on the energy audit. Ms. Laymon asked for more information on high-performance water fixtures. Ms. Taylor noted that would include updated sink and shower fixtures at the nursing home and jail. She provided an example of sinks with automatic turn off features. Ms. Taylor thanked Mr. Grant and Mr. Gaither for their work on the project.

Ms. Taylor noted the Committee would be in this room next month due to delays in the Dias project. Chair Laymon asked if there were questions for Ms. Taylor; hearing none, she thanked her.

Under Other Business Mr. Tompkins thanked Mr. Gaither and Mr. Steffa for their work to maintain both the physical buildings and natural settings for the County. Chair Laymon noted the next meeting would be September 7, 2023. She noted there was no other business to come before the Committee and adjourned the meeting at 6:18 p.m.

Respectfully Submitted,

Julie A. Morlock

Julie Morlock Recording Secretary

Community Open House Attendance Logs

"DOWNTOWN BLOOMINGTON STREETSCAPE" COMMUNITY OPEN HOUSE – REGISTRATION FORM

Yes, please notify me by email of updates to **Email Address** Name DowntownforEveryone.com MIDT Smlot@citigmn.opo Chad Par -ko hadcparkerogmai . (On Joel . a C Zeta. Coffee Joel Hper 6 61 public ford a grant kimberly, inge. smith 29 mail. 14m Smith 8669 Swenaw gimail.com Urena Fish jessica millir @ ourlich. JESSICA Miller ALAN LESSOFF ahusso () jatu. edu Korty bre 24 Qie nos: cm Kat Whisson andi Whaten 21@gmail.com K ndi Whalen 164

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DOWNTOWN **EVERYONE**

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Thank you for registering your contact information so that we know you were here.

Yes, please notify me by email of updates to DowntownforEveryone.com Email Address Name Goddam.B.Q.Y Ref ro col douge mean org beys bjeffreys Omidamericaily.con \mathcal{O} Santino C the hangar art co. com ANTINO LAMANCUSA perumpe m cheryl. majesticdesign@pmsil. Magnuson rery | MERTAN RATION merchall amail.co Tory Othe Custle theatle. Com Kury O' Nonnuy jason@ the castle theatre. com Jason Williams \mathcal{N} amic. mouser@gmail.com Mouser Hmie

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Yes, please notify me by email of updates to **Email Address** Name DowntownforEveryone.com olene USPhabitat mclean.org mechan.oneal @axis 360.00 en O'Neel Joer Neal 65@ Gmail. Com Doel Neal stgill LIC granil.com Silimin Leill charle & mileanischamber realy A long n Porter @ mchistory.org Norris Porter bdirmontaite@gmail.com Barbora Dirmontaite JOE MCDONNELL SOEMAVENDEGMULICOM rlotto 132004 @gmail. com eten daveand cothy 12@ Frontier con X. ave Templ 172

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Janal Siminat	on joinington City	0/m.org
Steve &V	alerie Ambres	
AndyKanSuran	andy a coaldyst cons	Structs.com
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KaySwich	Kayshipswiech	equael. com
Vicki Tilton V	ILTFX@FOX-N-1	Tounds, com
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Thank you for registering your contact information so that we know you were here.

DOWNTOWN

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Name	Email Address	Yes, please notify me by email of updates to DowntownforEveryone.com
JOSH JEFFEREY	Josh@getclear.Co	
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"DOWNTOWN BLOOMINGTON STREETSCAPE" COMMUNITY OPEN HOUSE – REGISTRATION FORM

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Community Open House and Website Feedback

Public Comments (10/11/23 through 3/23/24)

Comment # Comment

- 1 Too much car traffic in one-ways downtown
- 2 Need for rentable e-scooters and e-bikes. Solar charging stations. Rainbow crossings by Bistro.
- 3 Rainbow crosswalk near The Bistro. Need spaces for e-scooters.
- 4 Weird is memorable! Go wild with the modern architecture!
- 5 I still think the idea of a meandering street can make the experience more pleasureable. The real obstacle is the bike lane which has a double purpose for loading/unloading. But I like the idea in general.
- 6 Love the lights overhead!
- 7 Alternate C-bike and parking on East cars will collide / Hybrid blend looks messy / Can nto change most of the building so make big change in streets.
- 8 Let's do this!

Street sections - In all 3 scenarios we'd need a bike lane going south on center st. / Street styles - classic is more in keepign w/ our existing buildings. / Definitely prefer a bike lane

- 9 next to the sidewalk. Otherwise, people exiting from a parallel park lane will have to pull out into the bike lane, plus bikes could hit car doors as they open. Would love to see a water feature that captures runoff, as in Uptown Normal. We'll also need good, safe crosswalks on center and East streets.
- 10 Alternate B-want to prioritize as much pedestrian room/access as possible but value the parking on only one side of the street
- 11 The water feature seems an unneeded waste of water and further reduces green space. There seems to be a kind of public-use tables/seating on the square. I am impress with the parking plan and walkability.
- 12 I love the idea of lots of green space trees, flowers, etc. Good lighting. Don't sacrifice beauty for utility.
- 13 Having a bike path next to parking isn't optional. B provides a much safer and smarter path for bicyclists.
- 14 PARKLETS! Strip bike lanes with a color to improve awarness and safety. Create "canopies" of hanging lights. Curbless around square. Mini parks. More greenspace.
- 15 need alternate accommodations for our homeless neighbors
- 16 What will be done about unhoused population? As a downtown resident, they are a problem that continues to increase. More walkability is great, but where will they go?
- 17 Love everything! Let's do it this time and not shelve it forever!
- 18 Can you do hybrid blend streets but classic/traditional sidewalks
- 19 Completely on board with everything I've seen tonight. Kudos to the city of Bloomington for its dedication to this project!
- 20 Great job!
- 21 Create a welcoming feel for all ages. Family "hang out" feel needed currently adult feel oriented. Features water and art to go visit. Fountain also good for photo ops.
- Please provide a logistical plan from the city of Bloomington outlining the maintenance of this proposed space. This would include jobs and duties of city employees including job descriptions and budget distrubtion for wages.

I think there needs to be more communication with all the downtown stateholders. For those that wore in these spaces, disruptions and change can be welcome, but then needs to

- 23 be more open conversation. I also worry about sustainability. Will the city hire more staff to maintain these new features (cleaning the splash pad, managing projector, weeding between cobblestones)
- 24 As long as there is safe parking...
- 25 let's move forward!
- 26 I love the idea of the MOST sidewalk. I would love to see the protected bike lane in this scenario. As an owner of a building and resident I raise the desire for less cars and more people. START THIS PROJECT
- 27 Major concerns are availability of parking, safety of bicyclists, and stormwater drainage.
- 28 Don't like the concert stage on the square
- 29 Very excited to see this happening in the community. Would definitely be interested in continued opportunities for public input or ways to support as this aligns with what I do for work. Love walkable and bikeable communities!
- 30 Will there be interesting mureals both modern and old? I was in Pontiac last weekend and I was surprised at the number of people admiring all the ROUTE 66 murals!

- I didn't feel overwhelmed with the closeness of the sidewalk in "A" but I could see it being tight with servers at ables, more than one person walking in either direction. Make
- Douglas lots (surface) more appealling looking. Offer limited number of permit parking in 4 hour lots for business and employees.
- 32 Agree with starting with museum square PEOPLE OVER CARS! WALKABILITY! / Street Sections A-not keen on this one. Limites the walking area and when you add in outdoor dining, need space to get a stroller or wheelchair through. But wouldn't be mad about C. / Street Styles-like some of the hybrid features. Lighting! Trees! / parking-consider pay to street park, tree garage. would
- 33 encourage downtown employees to not monopolize street parking. add public restaurants (at least during events) permanent structures to open and close streets. business signage-no banners, no electronic/neon/LED lights. Feel like the water feature/splash pad might be a safety issue. Wet kids on those slippery stairs might be hazardous, it also feels close to the street to encourage kids to be running around (outside of times where the street is closed) overall, love all the ideas. Keep them coming!
- 34 Take care not to just duplicate what Normal is doing. Keep Bloomington identity unique. May be one reason to lean toward more traditional styles. Ideas so far look beautiful and promising! Open house was a great idea!
- 1) Connection/Bridge to local residential areas is very important. 2) Quality and width of sidewalks is important. 3) Ability to easily and safely bike downtown is important.
- 36 The reduction of any parking is detramental to downtown. Bicycles reduce the flow of traffic.
- 37 It's a beautiful concept, but I can't vote for any until I know what you are going to fix prior to this plan. Parking is an issue. You target people who come downtown. You have a homeless issue. You shouldn't paint the house until you powerwash the dirt.
- 38 We need to commit to whichever design we choose. If we half-ass it, we will waste tax payer money.
- 39 If A or C is chosen, switch the bike lane and parking lane. Consider reverse angle parking for increased safety.
- 40 More communication between stakeholders-the people who live and work downtown is needed to move forward. The water feature is concerning, mostly due to the maintenance it will require. The city will have to hire more staff. Too many flags for no reason. Need to include updates to the South side of the square for consistency.
- 41 Question #2 is skewed. I want the greenery of "traditional" but juxtaposed with modern art, structures, energy-but the photos of those options are mostly pavement. Yuck. I want protected bike lanes. Alternate C should swap parking and bike lanes to be perfect. Projected lights are "fun" but light pollution and not an amenity.
 - The museum lawn design doesn't invite staying. I'd like to see more areas just for sitting in the shade with a friend, not shopping or "doing" public restrooms please! Visual
- 42 designation for pedestrian. Also the splash pad maybe doesn't serve the same function that a fountain would kids playing like to get water in unwanted places. Areas (like the brick/concrete distinction) for amenities. All the protected bike lanes!
- 43 please ensure proper bike parking, trash, water fountains, or public restrooms. Would love to see bike path separated or have activated signals i.e. ann arbor
- 44 Don't forget ADA / take out curbs / create design for south side of museum
- 45 more public restrooms and drinking fountains are a must
- 46 It's lovely however you have too many issues to address before you tackle this 1) parking 2) homeless 3) you need to encourage one trip visits. YOU NEED METERED PARKING.
- 47 Close the square to cars, make it 100% foot traffic or cut 1 lane down to allow for more walkbility/greenery/seating space. Alternate A -but not good - eliminate bike lane / Parking is too important to lose any spaces. The amount of people that use the current bike lines is few but because they have a
- 48 loud voice and the city feels like we have to accommodate them. We are not in Chicago!! Let the bikes ride with the traffic. There is never enough parking for the arena. This discourages people to come to any of the shows. Think about adding a hotel in the downtown for conventions and events.
- 49 More residential.
- 50 Safe, well-lit parking is critical. As a downtown resident, I just hope there is some effort to NOT increase the homless loitering.
- More communication with museum staff needed to work out the finer details for implementations. Start referring to the "museum square" in downtown Bloomginton in all
- communication NOT downtown square. I like the several design encerpts. It will be good for downtown
- 52 Very nice presentation!
- 53 Public restrooms. Raised enclosed catwalks. Closed off pedestrian area.
- 54 Would have loved to see an option with no vehicular access and only pedestrian. At least for a few blocks.
- 55 Wish there was a pedestrian only option.

For #1 B because I prefer diagonal parking - question what about deliveries for the businesses? With one lane of parking instead of this that may be a problem logistically. For #2 we have classical/traditional now - I love the addition of payers because it is warmen and adds interest to not have all concrete and asphalt - keeping classical touches in place like

56 street lamps and adding twinkly lights and modern touches makes the space inviting and inclusive for all ages. I look forward to hearing/seeing how mosaics and sculpture and public art will be incorporated.

- 57 Where is wifi? It really would not be difficult to have public wifi covering the walkable areas downtown.
- 58 The water feature needs to be done well. It needs to be on the Jefferson Street side. Close the one Block of Jefferson by the museum. Create water feature, concessions, restrooms, join picnic area, and a shelter for farmers market (and other events) headquarters. Information building.
- 59 At one of your kiosks you had overhead lighthing. Those need to be incorporated all over the downtown area. People want a reason to "hook up". Lighting and safety go together! Stores might be willing to stay open later developing many more visitors downtown.
- 60 Parking and walking spaces can be done better. None of your options are quite correct. The issues around garbage pick up were not addresses here either. More trees/planters/seating could be planned in as well. Talk to Jamie Mathy and truly payattention to what he is saying.
- 61 Street sections prefer none / still the same problems go beyond the main avenue all the problems still exist. FYI in weather people do not walk here and parking needs to be nearer. No ADA shown on rendering. Downtown/surrounding area not supported.
- 62 Need to expand entertainment space behind museum.
- 63 I'm happy that the plan extends to beyond the center streets and connects to the outer neighborhoods. The weste side often feels forgotten and I appreciate the efforts to include those areas. Why so many flags on the square though?
- 64 People do not come downtown for parking. They come for events, shopping, dining. There is plenty parking with the garages.
- 65 Street Sections NO to alternate C / Street Styles DEFINITELY NO to modern/contemporary. Maybe some elements are nice to hybrid blend.
- 66 02-ADA access?? Love northside improvement and outdoor projection. 05 like the off road bike lane. Transportation center is definitely needed.
- 67 Street Styles like modern but history is unique and attractive and US keep historic street lamps! Love the storage not sure about splash pad. ADA (02) with more consolidated parking, we'd like to see more ADA/accessible parking not just minimum. Make sure ADA can get to lawn and stage on square. Keep new way finding sign and arch! :)
- 68 Sustainability and climate residency must be prioritized flooding prep, reducing transmission pollution and car dependency, more pedestrian-friendly streets!
- 69 Pedestrian priority should be the design focus minimize parking to get full use of garages.
- 70 Plan to manage for homeless occupancy.
- 71 Doesn't look like you included the cultural district (MCAC and BCPA) in your plan. Why?
- 72 Why no consideration for the existing art hubs? The "Program Goals" map completely excludes the BCPA and MCAC.
- 73 Downtown needs more trees, decorative grasses, and plants in larger spaces to help provide shade. Etc. sidewalk repairs are a MUST!! Public art will bring more variety of people see Ottawa, Geneva, Urbana

Option 3 - don't do this! BLM has spent way too much on downtown already and is not generating a lot of traffic to DT. The arena was a bust. The BCPA generates some traffic, but continues to run it in the red and most people don't spend more money DT. The return on investment of millions of tax payer money is not there and would be there. These types of

- 74 projects SHOULD be put to a vote in a refferendum. A small committee that is appointed, should not have so much say in tax payer money. Just putting a pretty facade on some buildings i not wanted by the majority.
- 75 The #1 complaint about downtown is lack of parking. People do not want to walk very far unless they are casually strolling. If they have specific errands, they want to pake as close to the business they are visiting.
- 76 Jefferson from Main to Cneter should be a pedestrian-only area. Not too thrilled with water features around the museum. Like CT transfer station design. Wish we could carrow East and Madison NOT ped friendly.
- Please keep shop owners and businesses in the loop about these proposals, or better yet, more involved in these proposals.
- 78 NO WAY! To Alternate C Street Section / I don't think the modern vibes with our historic buildings.
- 79 It's about time, thank you love it and I support it!
- 80 Need lanes for "delivery" trucks garbage is now terrible. All the "new" won't help if the garbage, etc. isn't fixed.
- Of the street section options I prefer Alternate B the most because it separates vehicles, bike lane, and pedestrians the best. I do think it would help to have a more long-term plan
- to include areas for streetscape redesign beyond just the core downtown area, such as library area and around the arena.
- 82 No parking downtown.
- 83 Close street southside of square. Need that for event/gathering space.
- 84 Lighting/Lighting/Lighting! Layers of light improve safety, LOVE water features :), planters/trees. EXCITED! Thank you!
- 85 Consider marking bike lanes with a unique material or color so bikers and pedestrians can stay in the proper lane.

These are very beautiful but lofty ideas. .Don't paint the house until you have power washed the dirt. Issues to resole: parking, and homeless. Questions: what is the incentive for any business to locate downtown? What is the cost and what was the cost of devising this plan? The cart is well before the horse here.

- 87 No water feature on the Museum Square. It would be better placed elsewhere downtown if that is kept in the plan. Prefer traditional, but hybrid is nice too. The modern feels soulless and cold.
- 88 I don't like the water features location. While centralized, I think it cuts off walkability and access, introduces fall risk with the amount of bars, unless constantly maintained

Better placement would be by bcpa

86

- 89 Please consider putting pressure on current building owners to upgrade and maintain the extertiors as well.
- 90 I think that walkability should be the priority, with the exception that there be accessible spaces for those with mobility issues. With the parking garages surrounding the area, there is plenty of space for folks to park. Perhaps a compromise are some spaces for "pick-up" if people are ordering food and just need to swing by and get it from restaurants. Adding permanent biking/walking paths that connect the trail will do wonderful things to the foot traffic of the downtown area. On street bike lanes are at least something but is still
- 91 scary territory for the more novice biker. Barricades or some sort of separation goes along was in increasing viability to more pedestrians. The room may not be available but roundabouts have shown to decrease automotive and pedestrian accidents. Really excited to see what new and interesting uses we can find for this great communal space. I believe this option would be the most equitable to all impacted parties while still accomplishing the mission of improving walkability of Downtown Bloomington. To mitigate the
- 92 safety issue of the flex lane, it should be a priority to avoid any structures or landscaping that would obstruct a driver's line of sight in a way that would be detrimental to a bicyclist in motion.

Streetscaping is all well and good, but what about safety and cleanliness? We have issues with people urinating and vomiting in door ways. What's going to stop them from doing such on the water feature? And we have issues of close calls of pedestrians almost being hit crossing intersections. Will this help or hinder the situation? And is the city committed

- to maintaining this? They aren't committed to tons of trees they plant each year in the parkways a good percentage die from neglect.
 Most of the images shown are for flat areas. How will these designs work for the hills on Main and Center? Will these designs be extended to East and Madison?
 Is the only reason for the water feature is because they have one in Normal? It seems incongruous with the rest of the area.
 These upgrades will be great for downtown. I live downtown and we are very excited for these changes. How will we clean up downtown and maintain to keep it clean? Right now,
- 94 downtowns current state has trash all over (especially north side). As a resident and business owner downtown, I hope the first step is to clean all of downtown up and help the homeless find a new home that isn't downtown Bloomington.
- 95 What are you doing to help the unhoused population? As this is in crisis, none of these improvements will be worth it if they are not dealt with. They will simply be using the night fountains to do exactly what you think they will.
- 96 Wow! I love the options you have presented! I'm looking forward to a more immersive experience downtown. Thank you for all your hard work!
- 97 I like the idea of more green (trees/plants) in the modern design but the look of the traditional look. And walking around town I have never not crossed the street to get to somewhere I want to go that was seen as a potential con to business owners on opposite side of the street

I like the changes called out here. Pedestrian and bike traffic are often under represented in smaller cities like ours. A couple questions and thoughts. How will this change bus

98 routes/bus depot? Im not a fan of the projections on the buildings, not sure how that will be an attraction full time, maybe just for big holidays? I could see this also annoying people in the apartments nearby

I think Downtown is plenty walkable, people don't actually want to walk though. They complain if they have to walk half a block. They might walk if they have decided to take a stroll around the shops. But that's few and far between. Most people have an errand to run and they want to park close to where they are going. In the cold months especially no one will want to walk far.

99 Walk to walk fail.
99 The water feature on the side of the museum. I'm not sure that is a great idea. I just imagine it being a place for drunks to pee. Then children are supposed to play in that? I wonder if anyone in the planning committee ever goes out between 11pm and 2am on the weekends to watch what goes on.

No one seems to think it's their responsibility to pick up trash. The cans don't get emptied enough. There needs to be a plan for basic maintenance and cleanliness.

100 Would love to see that native plants make it into the landscape design

With the amount of parking near downtown (parking garage, multiple lots, and other side streets), improving the pedestrian space and walkability of downtown would bring the most benefit to businesses and overall feel of downtown. Making downtown a pedestrian friendly place would improve opportunities for restaurant outdoor seating and make

- 101 Instruction of the improved process and overlat rector downtown rinking downtown a processing method, processing with the traditional brick and small town style fits Bloomington the best and would go well with the improved pedestrian first approach. A hybrid blend would be okay, but going too modern would remove some of the charm of the downtown area.
- 102 I don't like the idea of having the parallel parking on the other side of the sharrows that's really dangerous when people open their doors.
- 103 The style should include landscape design that is sustainable and regenerative. Collaborate with the Ecology Action Center and the town of normal to explore plantings that are beautiful and ecologically beneficial.
- 104 Let's do this! So much opportunity to improve the experience for everyone in our lovey downtown area, we can make it so much better!
- 105 Love the designs! I don't see how any design would be a down grade. Excited about the walkability and a revamped look! Would really make the downtown area a much more desirable area to go eat or do sop shopping or going to live music.

106 Let's move forward!!!!

- 107 I think the downtown area is so compact that having parking only on one side is worth the additional walking space. Especially if improvements to the parking garage are coming.
- 108 I want to see art, lights, and lots of color!
- 109 The hybrid seems to be more appropriate considering the classic architecture of downtown's buildings while also modernizing the streetscape. Incorporating Perennial plants would be a nice addition to cut down on landscape maintenance.
 - I love these improvements! I would love to see more improvements for cyclists. Many cities have started painting bike lanes green or red and I really think it provides another layer of seperation from cars without many added expenses.

Even better than this would be to incude ballards or planters or some other form of physical separation from cars to improve safety. I understand that this may be difficult in this project as parking is on the other side of the bike lane. However, things like this improve safety for cyclists, it'd be nice to see here but also in places like Front Street. Another improvement, that is perhaps a bit beyond the scope of this project, is improving connectivity in bike lanes all over the city. Bloomington-Normal is just dense enough to be

110 ideal for many to commute via bicycle but a lot of useful connections just aren't there. Constitution Trail is an amazing resource we have here and I think just improving the bike network to go beyond these scenic trails would do wonders for the city!

For this project, just ensuring that people can get to these very nice bike lanes from where they live and to where they want to go is essential in making sure that they are actually used!

Thanks so much for reaching out to the community for input, it really makes me feel connected to everything! I am very lucky to be in a place that values feedback like Bloomington does. Thank you so much!

As someone who is a cyclist, adding specific bike lanes is MUCH better than "Sharrows." After traveling to Europe, the Dutch idea of separate bike lanes really raised the number of bikers. Importantly, we need to add more bike parking facilities and secure a more permanent connection to the Constitution...

What's missing here is the ultimate intent of the new streetscape. If it's to encourage biking and pedestrian access, Design Alternate B would be safest. But where will the parking be? Will there be a safe, structurally sound parking garage nearby, or are there other plans for parking? As for the streetscape style, that also depends on intent. Do we want to

- 112 bet with there be a safe, structurary sound parking garage nearby, or are there ofter plans for parking. As for the streetscope style, that also depends on ment be we want to encourage meeting spaces, contemplative spaces, or something else? I do think that whichever choice of style is decided on, we need to allow for adequate stormwater drainage. That could mean curb cuts to swales or stormwater drains, or raingardens, or other solutions. The more hardscape we keep or add will make stormwater drainage a greater
- 113 I'm so excited this is underway in Bloomington! Improving the built environment will have positive impacts on community well-being and economic vitality for years to come!
- 114 Our community needs this!
- 115 As someone in a wheelchair, I'm excited about downtown becoming more accessible

Prioritize dealing with real needs like empty/dilapidated buildings in downtown, deteriorating parking garages and infrastructure needs before the 'nice to have' items like a water feature and outdoor projection

116 This would be on the nice to have list, but I think Bloomington a community market space like one of these could be a neat addition to downtown:

https://www.newbocitymarket.org/

https://milwaukeepublicmarket.org/

https://midtownglobalmarket.org/

- These redesign plans make so much sense for downtown! I'm really hopeful and excited for follow-through and completion of this work in some capacity. It will transform
- downtown into the jewel of Bloomington.
- 118 This is so great!! I think improving walkability is vital along with more space to be there safely with kids and bikes and dogs.
- 119 I like design C, but it would be better if you switch the bike lane and parking. Place the bike lane curb side and keep the parking left of the bike lane so cyclists will be protected from traffic. That way it'll be safer for everybody.
- 120 How will the unhoused be assisted?
- Please stop this nonsense. Stop wasting money on useless things like this. Fix the roads, or lower our taxes. We don't need any more pet projects. This is really annoying. Stop wasting money!!!!! Feed the homeless, give them a place to stay fix what is already broken like the crappy roads all over town. STOP SPENDING MONEY ON THIS TYPE OF STUFF
- 122 Love these ideas and am so excited to see this implemented! As a downtown resident, my concern before any of this is implemented is the homeless population. These new spaces are beautiful, but don't want to see anything misused.
- 123 I think including more things that reference Route 66 might help draw more travelers to downtown. We should have something that makes people want to visit the downtown area. More inviting eating and shopping places are needed.
- 124 Like option C with the hybrid style!

I don't favor A, B, or C. An ideal plan to me would be to close Main between Washington & where it ends at East, close Jefferson between East & Madison, close Center between Washington & Monroe. Keep Center open from Monroe to where it begins at Madison. Have Center return to Madison at Monroe. This allows access to the Center parking garage and whatever it entails in the future. It also maintains access from Madison and allows continued use for the wayfinding sign. Add a parking garage on the flat (currently privately

125 owned) lot at Market & Main.

Note, I selected C only because no options similar to mine were offered. I definitely believe we should go BIG!!

- No discussion on bus system which now uses Main St. . Water feature not large or interesting enough to be an attraction. Use of pavers is good, would calm traffic. Significant
- investment has been made to erect vintage style lights, why not keep them?
 Tearing up downtown to make changes is going to drive people away from the area so whatever you do make it quick. And, fiscal responsibility isn't one of the City's strong suits be cautious to not overspend please.

Who the heck is complaining about crowded sidewalks? The only things I have to dodge each day are the persons who ask me if I know the time then proceed to ask me for money,

- 128 and the overwhelming smell of urine and vomit in the parking deck.
- We get a lot of complaints about parking, so I don't want to give up so much. I would like to see outdoor dining as less of an afterthought, so I like not eating in a parking space.
- Maybe the city can help fill vacant spaces instead of writing reports about a missing ceiling tile.
- 130 Please make sure there is parking for those of us who can't parallel park
- 131 \$700,000 just to study one block in Downtown? Why are the improvements only planned in front of that one building? Downtown is large and needs updating throughout, not just in the middle. Do better.

How exciting is this?! As a new and young resident of Bloomington (24yo) it is important to me that this concept prioritizes walkability and green space. We live in such a manufactured world, that city squares that prioritize pedestrian and bike traffic are the places I want to be. Likewise, I challenge you to consider features that keep pavement and

132 urban heat to a minimum. No one wants to sit outside if the pavement is radiating heat and there is no shade; please use green spaces to mitigate this. I would much rather see the parklets and protected bike lanes than more parking spaces. Accessibility is important for drivers that need to park next to the storefront, however, how can you provide an option that respects accessibility and ENCOURAGES people to actually use the bike lanes? I'll tell you right now that I will never ride my bike to downtown if I am in fear of my life trying to dodge cars attempting to parallel park across my bike lane.

I absolutley love more walkability. I think the younger population will really value that and create a more "city like" feel helping retain younger professionals. I also think outdoor seating will bring huge potential to the downtown area. Additionally, I think the city needs to be more intentional on what shops and stores are approved and signage that is

- 133 required. There should be a consistent look and plastic signs make the area feel cheap and take away from the charm. We need more storefronts that cater to an experience, locally owened restaurants, and boutiques.
- 134 I believe that option B offers the best option to incorporate drivers, walkers, and bikers. If the Connect Transit hub that will replace the parking garage will not offer the public parking as being discussed, I would lean towards Option A.
- 135 Love the 1 driving lane idea, people drive too fast!

- 136 Beautification with no less parking is important. Street parking is already difficult in the area. There are garages, but people are less likely to use a garage when wanting to stop in a store or business
- 137 All 3 designs are terrible. They do not represent Bloomington's ideals or atmosphere in any sense.
- 138 I love the proposed improvements especially the projections and water features!
- 139 We want more plants and green. They give life to an area. Plants and green impress on people happiness and warmth, somewhere where people want to be .
- 140 We need this as soon as humanly possible! Yet this shouldn't also be confined to just the Downtown area, but in other areas of commerce around Bloomington, including along Veterans Parkway, such as the Eastland Commons, Empire Plaza, Oakland, El Dorado Rd. and the entire length of Market Street.
- 141 The water feature is absolutely the worst part of these plans. Not only will it cause problems for businesses with wet children walking in and out, but drives pedestrians away from the center in fear of playing children/getting wet.

I am favoring Plan A and favoring Classic design. I like the looks of the new garage on Market and Monroe with retail stores on lower level and a possible mix use of the upper deck of

- 142 the garage. I feel the Transportation Center (Connect Transit) should be at the Front Street Garage where there are more destinations and workers: New Public Library, Police Station, Jail, Law & Justice Center, McLean Co Health Dep., Grossinger Arena, Government Center and many eating locations.
- 143 So glad you are pursuing this!
- 144 No brick pavers, too high maintenance and not ADA accessible. Keep it simple, museum square only to start, and no splash pads. Parking can be a deal breaker for many people. Many do not like Uptown Normal for a quick meal or stop anymore because of all the parallel parking which also is dangerous when backing in in the small streetscape. Elderly shopper do not care to park far away in parking garages. The positives in Normal that would help Bloomington is the beautiful landscaping and flow of color and design of buildings. Water feature are aesthetically nice but can become a liability when children are encouraged to play in them unless meant
- for play. I am a native of Bloomington and grew up with downtown as our primary source for about everything. So glad it will be revitalized. The atmosphere of evening lighting in your photos is a great addition not just for safety but atmosphere to bring people in. Safety in design should be a priority as right now many homeless esp on the north end are a concern and not inviting.
- 146 Convenient and accessible public restrooms! Small designated spaces for street performers (jugglers, magicians, musicians, etc.). Plenty of benches.
- 147 I don't think bike lanes need to be a priority. I practically NEVER see anyone riding bikes in bike lanes around town. When we have out of town guests, they comment on how well preserved a lot of the old buildings in Bloomington are. I don't know why you'd want neon etc, when we have something much more timeless and interesting that just needs refurbished. Also, it mentions books, I don't know where to buy books in downtown b;ton. I also wish there was at
- 148 least a little place to buy beer nuts, people used to love that, and its an only in Bloomington sort of thing. I'm ok with the additional music space, don;t see a need for another splash park, plenty of those in town. Sometimes I don;t feel safe walking after dark, can that be addressed?
 - Hi, I have worked downtown for over 30 years. I am very happy to see progress on downtown renewal.

We have many clients who complain about the downtown parking situation. The complaint is about the over zealous parking ticketing- not about the lack of parking. Our clients are often here for hours at a time. This makes the downtown "unwelcoming". Please consider open-space (as opposed to deck) parking, like patrons would have at virtually every other

149 common area location in the city. This will likely entail tearing down some buildings for parking. Moreover, the parking deck on Monroe is an unsafe, unsightly structure. Tearing it down is a must for any revitalization. But in turn, the plan must include a different vision for parking.

Aside from that issue, all the proposed plans look fantastic.

Peter Brandt

150 How about none of these? I guess we haven't learned that downtown is a money pit. All the funds that have previously been funneled to downtown have just gone to waste. This will just be the same: millions of dollars for a whole lot of nothing. I know you won't scrap this project based on one person's feedback, but you should learn from previous history.

151 You MUST repair Washington street!!! I am disappointed to see that none of the plans include the cultural center or Art Center where parking is truly an issue. For a business to be successful it must be convenient for

- 152 people to get to and to use their services. Limiting parking should not be an option. If I understood the plan correctly, it would remove the parking level (Lincoln lot) and rebuild a bus station with another parking garage. Is that the best use of tax dollars? perhaps saving the present garage and adding a bus station would be more fiscally responsible. This is the way I interpret the plans you have laid out on this web site.
- 153 Can you tell me what is the estimated cost? And what will the increase in tax be per household?
- 154 Design 3 is my choice. The challenges don't seem too complex . I like the parking on both sides of the street. I like the classic/traditional design look because it is surrounding a very old building and would blend better with it make it shine.

- 155 The homeless population is a huge problem for the downtown. It needs to be addressed or national/local retailers will avoid it no matter how nice we make it. There is not enough green space or a large enough water feature to draw citizens downtown - therefore I would not reduce the parking.
- 156 If you're going to spend the money, do it right.
- 157 Consider adding speakers with soft music near sidewalks throughout downtown. Other downtowns do this and have the speakers mounted on light poles. It adds a lot of ambiance to the downtown area and draws in many more people, due to the added sensory of having music playing while being out in downtown.
- 158 Safety of the flex lane is IMPORTANT if people are going to use it! Thank you for making the curbs disappear :)
- 159 Definitely plan a and modern contemporary should be the point.

As a young person who moved here from California two years ago, I have to tell you that what makes this town special IS the classic, historic Main Street feel. I saw the contemporization process happen again and again in CA, and what you inevitably end up with is a look that feels VERY dated VERY quickly. There are a million towns with gimmicky water features, nonsense Adirondack chairs that seemingly exist solely to be vandalized, ugly light fixtures that try to capture some Carnaby Street magic but don't work out of context... but when you stumble upon a downtown that feels truly grounded in its history, it's something special. (There's a reason people flock to Galena every year!) Bloomington retains so much of this unique charm, and I hope it remembers to prioritize that. Because gimmicks like projection screens and water features are just that - gimmicks. (Come over to my Miller Park neighborhood and visit our tacky new Route 66 lawn ornament on any given day to see how that's working out.) What's more important are things like walkability and bikability - so I desperately hope you prioritize those parts of the project rather than flushing money down the toilet on flash. I applaud the attention paid to improving East and

160 Madison streets as well, and hope you consider expanding that part of the project lutter than item results in the "surrounding" neighborhoods would certainly encourage more foot traffic and ease parking worries - as of right now, there's no way for many Westsiders to walk downtown in a particularly safe or enjoyable way. I love going to the farmer's markets on Saturdays, shopping at Neighborhood Thrift, eating dinners at Rosie's and Anju, drinking cocktails at Mystic Kitchen - and I know my husband and I would do ALL of these things more frequently if there were a Constitution-Trail-style walking path or even a well-maintained street that could get us there more easily! We've noticed more and more young couples moving into our area near Miller Park, and I hope all of us will be able to walk and bike downtown more freely in the future.

Thanks for encouraging community comment, and kudos on the site design - this has been a very user-friendly experience, and I hope that attention to detail carries on through this process. Looking forward to the improvements!

161 FIX THE ROADS FIRST

There is no problem with "walkability" in downtown. Certainly to the extent the cost of this requires. There are more pressing problems in our community that giving downtown a

- 162 facelift. This is a luxury tax payers really can't afford at this time. Taxes are increasing almost monthly for one reason or another. Please consider people who would rather spend their money of food and housing. Some things we shouldn't do just because we can.
- 163 Gross waste of money. Fix the streets and the areas where 98% of the population LIVES, not spend money where 2% LIVES. This idea is to feed the ego of Gleason and company. His work in the Decatur downtown and it's failure proves he is wasting money.
- 164 Please don't spend our tax dollars like this.
- 165 Reduced lane usage is a mistake.
- 166 Walking downtown is a mistake. There was a murder down there.
- 167 Want to hear more about the blocks north of the square including the more dense bar district While the designs are lovely, we cannot afford to spend our hard earned money on items that are not necessary. Upgrades will not bring people to stay. Taxes are too high, roads

168 need fixing, we need to eliminate Smart cities, Agenda 2030, leftist government that does not care about the citizens, eliminate DEI, CRT and CSE in schools. Many students are not meeting education standards, many are not getting enough to eat.

Please re think your values. Put God into your life.

As a long time citizen of Bloomington (50+ years), I strenuously protest the spending being considered on downtown and the surrounding area. LISTEN to your citizens and STOP

- 169 SPENDING OUR MONEY ON THIS FOOLISHNESS! None of these choices are acceptable as they obviously will cost an extreme amount of money. People don't want to hang out downtown because of the difficulty with parking and the beggars. I am so sick of this city spending our money so recklessly. STOP!
- 170 Why do you think downtown needs to be reimaged? There isn't enough lipstick in the world for that pig! Stop spending our taxes on this. There are OTHER areas in Bloomington, in case you are not aware. Get your heads out of your asses and listen to the taxpayers. WE DON'T WANT THIS!
- 171 Please focus on the roads and sewers! Downtown will never be a safe or attractive place to shop as long as it is located near the west side, has an ongoing homeless and crime problem. I do not feel safe during the day time let alone the night time.
- 172 Waste of money nobody will go downtown for any of these ideas, aka arena

Don't spend the tax payer money. It sounds very utopian, but you must consider, the majority of people in this community will never visit downtown, no matter what. Decreasing parking and trying to increase walking and biking makes it even more inconvenient. Also, have you considered that out of 365 days in a year, less than 30% of those days are comfortable to spend any significant time outside? We are not Disney in a warm climate, and you can't change that. Nobody wants to be hanging out outside when it's under 50

- degrees, snowy, rainy or even humid and 90 degrees plus. All the ridiculous entertainment sounds real enticing, when you're trying to sell a bill of goods, but how much of our community really will regularly take advantage of that? Is it enough to justify the price tag? You know what would really benefit the community? Lower taxes. Everyone suffers under higher property taxes, and other taxes. I know you'll do what you want, but I'm like so many others that you want to pay for this, I'll be leaving this community and moving to a lower tax environment as soon as my child graduates high school (private school, because the schools we pay so much for are pathetic). By the way, how is the arena doing?
- 174 Before you get to far over your skis, fix the streets. Downtown streets are competing with IL State roads thru Bloomington as the very worst. No worries about speeding downtown with street conditions as they currently exist. And, if I have difficulty finding reasonable parking, I'll go elsewhere. So will everyone else
- I have mobility issues, but not to the extent that warrant a handicapped card. Every parking space eliminated limits my ability to access Downtown. I have virtually stopped going to
 Uptown Normal altogether because too much walking is required. I have never encountered pedestrian congestion Downtown with the current level of access. The impressive and interesting traditional architecture of Downtown does NOT need light shows and water parks: it just looks like mayhem to me.
- 176 All great options but also make sure that trash is taken care of in a timely manner, lighting is improved, and safety officers are patrolling to keep everyone safe. Great ideas and I'm looking forward to seeing the improvements! I'd suggest a curb type barrier for the bike lane. Offers protection to bikers. Another alternative to "design C" would
- 177 be to reverse the bike lane and parallel parking lane (Bike lane against the sidewalk, then park lane, then one-way street). Again, protects bikers and doesn't allow cross traffic between cars and bikes. I've seen this in many cities. Downtown St. Petersburg, FL is a good example, and it has one of the most vibrant downtowns I've seen. Thanks! I understand the need for parking, but perhaps part of the plan should be to use/purchase property for public metered parking lots nearby or small parking decks. I would really prefer almost zero cars and definitely no busses in areas where people want to eat/gather/walk. If not possible, then some sort of barrier (fencing, dense bushes, etc) to break it up. There's really nothing like having a nice dinner outside and having a bus or car belch exhaust just a few feet from the table. The buses really should be routed away from the streets within downtown anyway.

Maybe just a lot of speed bumps to discourage unsafe driving through the area.

178

I think these changes are exciting and I have long known that downtown has the potential to be a popular daytime and nighttime destination. I do really really like the vision you have put forth here.

However, making sure nearby areas and parking garages are well lit and surrounding areas are safe will also further promote interest. I think that is a challenge currently in today's environment. People not feeling safe or afraid to walk by themselves. Even in places I think should be relatively safe, there are times where I'm wary of some of the garage "residents" and some areas that just seem dark/isolated. Maybe it's optics but as someone who does frequent downtown socially I've heard similar sentiments, especially from It seems like option c only has parallel parking, I would visit downtown even less if there was only parallel parking because I do not enjoy trying to parallel park. Even with the amount of parking there is now, it is often difficult to find somewhere to park on busier weekends. The parking garage on Market is nice but I'm not sure what the plans are for

- 179 another of parking there is now, it is often unneutror intra somewhere to park on busief weekends. The parking garage of market is nice but if motivate what the parks are for parking in it with the new bus station going there. I think parking should be a priority. Even when downtown is busy, I have never thought "oh man, I need more walking space", but I have frequently thought "man I wish there were more places to park." Please don't get rid of the parking that is downtown, it will negatively influence the amount of visitors that I've always heard there are large caverns (not the right word) under the sidewalks downtown that would be required to be filled to make any substantial changes to the streetscape. If true, would that happen as a part of these potential streetscape improvements. This was always talked about as a barrier to improving downtown, so I am hopeful it's either not
- 180 an issue or the City is prepared to do what they need to do to move forward! I wish this plan would have addressed the major barrier that is the racetrack of a state route surrounding downtown, because until people start thinking about/feel safe parking across and/or walking from the other side, parking is going to continue to be a hot button topic in the core. I really like the online engagement platform built for this effort - very simple and easy to participate!

I prefer option C over B and A for the reason that it offers the most pedestrian space while keeping the street even. Option A is too lopsided and option B isn't pedestrian enough.

181 Also the parallel parking on option C is really good because it provides a cheap buffer for the sidewalk from the street. I would look into incorporating many plants into the design as well, as it is proven that established tree canopies above streets reduces driver's speeds which may reduce pedestrian deaths.

The historic nature of Bloomington's downtown is its greatest asset and needs to be preserved and maintained.

Downtown is a unique multi-use neighborhood where commercial, retail, and residential exist in tandem. Residents and visitors, especially pedestrians and cyclists should feel safe while using their mode of transportation. Pedestrians and cyclists are more likely to visit various retail shops as opposed to automobile users.

182 Since Bloomington's Park system has several splash pads in various locations throughout the City, a splash pad water feature would be duplicative. Perhaps permanent tables/benches for chess/checkers would be of more value for visitors of all ages to gather and linger while playing downtown. This is where Bloomington should think outside the box.

While I believe access to permanent electrical outlets in necessary to support entertainment activities throughout downtown, constructing a permanent bandstand on the History Museum stairs detracts from the architectural beauty of the former historic courthouse.

183 I'd be interested in the cost breakdown of the options as well as proposed implementation plan. Although this is a "nice to have" design concept, we have other priorities in the city that need attention, like road repairs, sewers, and efforts to reduce costs by improving efficiencies.

Don't listen to any of the NIMBY east siders who'd never step into Downtown anyway, including Motney, Becker, and Lee. They have their little suburban enclaves and subdivisions, let us who actually live in and around Downtown be the loudest voices in what we want to see. Our historic core needs far more attention than these far flung money drains and this plan should go beyond just a streetscape. This should come with a new 5-10 year plan for downtown including how the city is going to tackle delinquent landlords, providing resources and aid to bring buildings completely into compliance, tax land speculators and parking lot holders, encourage indirect rent control measures until the state ban is lifted like a rental profit tax, and actually curate a Downtown through public funding and control rather than allowing the private market to ineffectively, inefficiently, and unequally develop it.

I'd love to see these plans extended to areas outside of the immediate downtown, Main and Center. It should be extended east to at least Gridley, preferably McLean, west to Lee, and south to Oakland taking in the old industrial center. We can extend North later. Downtown really is more than just the 15 or so square blocks everyone thinks about - mostly because of the abominations that bind it in, the four lane sections of Madison and East.

The city should explore ideas of emanate domain and other tools to ensure compliance and better move building stock, even if just the threat of it for absentee, slum, and out of town landlords.

184

At a larger scale we should readjust our taxing bodies to lower the taxes on downtown properties, if possible, and increase it on the suburban sprawl developments along Veterans and other areas. Removing parking mins and adding a tax per parking spot I think would do wonders. We subsidize too much of their space as it is. If Best Buy really wants to keep their current square footage but not pay for parking, Front and Center building has been open for the taking for years. Hell, see if Jewel wants to move it. Urban two story grocery stores are nothing new and the whole area surrounding Downtown is a food desert.

The city should use and encourage completely native plantings in future landscaping, not just downtown but everywhere. Reduce the amount of turf grass the city needs to mow every other week, reduce ornamentals that dont add to our ecosystem, and reduce the risk of invasives getting into unwanted areas.

Work with Connect Transit and other agencies to improve public transportation, at least explore what would need to happen first to ensure more ridership.

Start rezoning areas immediately outside of Downtown to allow multi-family and mixed use development. R1 zoning will continue to bleed the city dry. No one's going to get hurt if all of a sudden there is a duplex next to their single family home, or a coffeeshop with a few apartments on top over on the corner. This will also bring more traffic downtown with increasing amounts of people living around it.

I hope this is not rude to say, but it was very difficult for me to actually see the differences between the options, even with the pictures and explanations. I had the same photos
 open in all three areas and I was really squinting to see what the actual differences are. Still, I think B probably makes the most sense? There is already a parking garage downtown that is free for quite a bit of time during the day, so losing some street parking would not bother me.

Where do the delivery trucks park? Will fewer car lanes in crease traffic jams and discourage people to go downtown? I prefer the seasonal traffic parklets because in the winter,

186 customers don't want to walk so far in the snow.

The graphics on the courthouse is excessive.

Definitely a much needed improvement. Lots of exciting things happening already....but one thing that you could do to make downtown more inviting on a daily basis is creating a greenway / walkway / trail space between Illinois Wesleyan through Franklin Park and into the North Corridor. This is blatantly obvious...restore the blighted area, increase viable

- 187 business, living space and help connect a tremendous asset of residents and students maybe even stretching to Constitution Trail around Empire or Emerson Streets. Most communities would love a chance to use this what is today a wasted space and opportunity. Please consider this :)
- 188 Love the redesign ideas!

Design. The parking that is lost with Plan B needs to be made up with additional parking in the parking garage or somewhere else. There is a shortage of parking downtown. Streetscape style. This is hard to tell because they do not look like Bloomington. I think it is important to keep the city connected to the two main attributes the city has--Abraham

189 Lincoln and Route 66. and perhaps the origins of State Farm downtown. what is happening with that building? Anyway, I guess that would mean a more traditional aesthetic with some modern bits.

I would like to see easier pedestrian access to the Library from down town.

- 190 I am excited for downtown Bloomington to be improved! This should make more people feel positive about the area and produce a desire to visit and spend time there.
- 191 Until you give people a reason to come downtown (not just a farmers market) they still will not come there. All you have now are bars a few restaraunts that are over priced and too niche.
- 192 Veterans memorial plaques MUST be preserved and prominently placed in whatever design is adopted. None of the above!!! Stop spending money on pet projects like this that bring little benefit to the average citizen but force us all to pay for it. Bloomington residents are being taxed to death and many are leaving the area. Businesses are leaving or selling property to keep costs down. The expensive pet projects (library upgrade, the new water park, many new
- 193 parks, etc) are getting a ridiculous amount of tax dollars allocated to them while necessities that we use every day are NOT being addressed. Our roads are crumbling horribly in Golden Eagle and many other neighborhoods. Our water infrastructure needs major updates. Do project to address that! Stop pouring money into the nice-to-haves while neglecting what you SHOULD be paying attention to!
- 194 Stop spending money on this type of stuff. My property taxes are really high, and you want to piss money away on this nonsense. Lower property taxes first, or fix the crappy roads. I have concerns with removing on street parking options considering we do not have a safe parking garage within a reasonable distance to most businesses in downtown. The Market Street garage not only needs a large number of repairs, it also falls into a "hang out" for many of the homeless people that frequent downtown. This makes patrons of
- 195 downtown feel very unsafe. With regards to the spaces for pedestrians there is a big concern to this being abused by the homeless population. This is a concern that also needs addressed. Currently the homeless population has increased and due our police not able to move them, adding more places for them to take up residence is concerning. I would like to see some sort of action taken with regards to the homeless prior to the streetscape updates happening. We live on the far east side of Bloomington and don't see us using the downtown facility. We attended two farmers markets last year and didn't see where you can really shop. We are not in the bar/grill scene and limited gluten free items available. Why do we need projections, water features? Will they be politically correct? Will it really draw a big crowd.
- 196 maybe at first, This is geared toward a specific audience. No real shopping specialty shops. How much did we pay (taxpayers/residents) for the consultants/plans for this project? No shoe stores, grocery stores, clothing stores. All of the buildings are owned by private entities so why don't they pick up the cost or provide products that people actually want/need to buy? Parking will be very limited. Will you have armed guards at the parking deck on Center street for the residents safety?
- 197 Option C and Hybrid Blend
- 198 Design alternate C is preferable & to offset the loss of parking we make downtown more accessible by re-establising the Streetcar & connect people from Hudson on the North to Funks Grove in the South
- How about we skip spending millions on this unnecessary project and put that money instead towards the water/sewer infrastructure, and other actually necessary projects.
- Seems stupid to double water rates over three years when we're also wasting money on a project like this one. Downtown will never be revived. Stop throwing money into a pit. Please coordinate with connect transit to increase the amount of buses and expand the bus schedule. Our city should work to eliminate dependence on cars and increase public transportation. A walkable city only becomes walkable when public transportation is actually available from people's homes.
- 200

We also need transit routes to and from Hudson, Heyworth, and many other surrounding cities.

I would love to see an option where Design A swaps the bikeway with the parallel parking spaces. It would be great to see a barrier between the motorway and the bikeway, emphasizing safety between the two zones. Additionally, as a young resident of Bloomington, I hope that you focus your designs on sustainability/greenery, walkability,

- 201 attractiveness, and safety for pedestrians. This is your opportunity to rebuild a downtown that grows with the next generation. We want a cool place to hang out where we can stretch our legs and support local business. The rest of Town is hard to walk through and full of concrete. Please let this space be different.
 Available parking is part of the current downside of downtown. The bulk of the citizens will have to drive to get to downtown. There will have to be lanes for delivery trucks without
- 202 causing traffic problems. Current venues/shops are not desirable to everyone. Bars are filled with drunks and the disorderly. We don't care to be around them. will there be any public restroom facilities? If so where. Who will maintain them? Will citizens be safe from drunks, drug addicts, homeless beggars, thugs, the mentally ill?
- 203 More trees down town will help tremendously. They provide cool shade and please our brains. PLEASE ADD TREES
- As a resident I believe in the value of this project. Thank you to the city employees prioritizing this and making it happen. Must, must consider the effect of delivery & ride-share drivers ignoring all parking regulations, posted or otherwise. Must not count on enforcement, either - just look to UT Normal circle for example of the avoidable mistake. Design should account for these needs to make it easy and natural for the delivery & ride share drivers to comply. None of these designs seem to take that into account. They're going to block the cycle lanes, park in the traffic lane, etc. It's also really odd the cycle lanes are not "protected" as is the current world-wide standard. In all of A, B, and C this can be improved by just swapping the parking and bike lane positions and adding structure (mindful of winter weather needs) that completely prevents cars from encroaching into the bike lane. Else, just forget the bike lane entirely... without doing that, it would just become a wasted space that cyclists will ignore. If you're doing a bike lane, please don't also forget bike parking and fixing infrastructure... but really, if you're not going to physically protect the lane, just don't add one.
- 205 Period.

Also, I live outside of town, but pass through often. I love to stop in at the bake and coffee shops, but if you get rid of convenient parking, that's going to come to an end because it would no longer be a quick stop for me... having instead to find parking outside the DT area or in a faraway parking structure. How to account for this? Nearly all parking could be short-term, 20 minute parking. Another -in-the-middle parking structure would also help. Really bizarre this hasn't happened with Connect Transit. I know it's expensive, but it's foundational and puts everyone (bus & car users) in quick-trip walking distances of their destinations. Else... a new parking structure just outside DT would need to somehow have the design of "quick-trip access" across 51.... Bridge? Ditto, even, for dining, but to a slightly lesser extent. Getting rid of parking near Brass Pig & Ale, etc, would mean I would be Two downtown projects that I really find attractive are Cincinnati, OH with their downtown playground, riverwalk, and color-changing waterfall fountain (https://www.cincinnati-

- 206 oh.gov/cincyparks/visit-a-park/find-a-parkfacility/smale-riverfront-park/) and Burlington, VT with their pedestrian-only (no vehicle traffic) downtown area with access to restaurants, shopping, and performances (https://www.vermontvacation.com/towns-and-regions/historic-downtowns/burlington).
- the proposal you choose will tell the community what you value cars, or more climate friendly, healthy choices of walking or biking...
- 208 I think an emphasis on the original architecture would preserve the unique quality of our historical heritage, with a reorganization of space and additional contemporary aesthetic would help us identify ourselves as a diverse, progressive community.
- 209 We absolutely need pedestrian friendly walking AND sitting space to attract our population and visitors Repurpose or remove abandoned buildings: Commerce Bank, Elks Lodge, Montgomery Ward (Front and Center). If removed, the vacant areas could, at the very least, provide needed greenspace or even a dog park. The best case scenario would be a sorely needed downtown hotel built on one of these sites. These buildings detract from any effort to
- 210 make downtown attractive.

Are there any provisions for public restrooms?

Thanks for reading

I love that the city is looking at reviving downtown. I spend a lot of time there and it would be great to have it more walkable and just fresh and vibrant in general. Love all the
 suggested enhancements around the courthouse building, especially the east side! As long as there is still plenty of parking and access to local businesses. I'd hate for them to suffer if parking was lost in front of their establishments.

Thank you for hearing us in the community.

212

May God Bless Blono

Frankly, I wish there was a pedestrian only street option. I don't care about parking, make the parking in the back of the business for those who really need it, as we have large

213 parking garages already. It's also so important to me that there is A LOT of plants. Style matters less to me than just avoiding bare concrete everywhere, which is what downtown often feels like.

214 Let's get this going!! Thumbs up!

Ideally I would prefer design C as it allows for the most pedestrian space, however having parallel parking on either side of the street makes the road feel very cramped. One example of this is uptown circle in Normal, granted the road is two ways. Maybe parallel parking on either side wouldn't feel as tight on a one-way.

215

For streetscape style, I think a blend of modern and classic would be best. The modern look is a bit too "flashy" and feels full of itself, but a lot of the smaller design elements such as the outdoor lighting and the use of greenery to break things apart is really nice. On the other end of this, classic and traditional can feel a little outdated at times, but you could argue that that's kind of the whole point.

There are many sidewalks in the downtown area that are currently out of ADA specifications, especially cross slope... I hope that these challenging issues are being addressed. There are many areas where a stores entrance is considerably higher than the current street level, creating a sidewalk cross slope that is dangerous. These areas would most likely need a split level sidewalk

need a split level sidewalk.

I think the designs that are being considered all look very nice. As for eliminating parking on one side of the street, i don't believe it will negatively effect businesses because it's not often that you can get a spot directly in front of the business you are going to, so you'll have to walk anyway. It may even help businesses because as you exit your vehicle you'll see all the store fronts, not just the one you're planning to shop at.

Our there are any plans to address the empty building downtown that could really be used for other things, or torn down entirely and then turned into park space.

217

All these ideas would be great for the city and to get people out to enjoy a part of town they maybe aren't going to. Would also be good to attract other businesses downtown. I love going to uptown and I wish downtown Bloomington was just as nice.

I have worked downtown for over 30 years. Any plan must include parking solutions. I see none here. Without a parking solution, any plan will fail. If the plan is to continue the

218 current parking enforcement policy (over zealous, aggressive ticketing), few will have the desire to head to the downtown area. I am not a city planner but it seems basic that this issue should be part of any plan and be addressed. Thanks.

I think maintaining the most amount of parking is vital to the downtown area - In designing this I believe we need to also be putting funds into maintenance. Cleaning up the streets (garbage, residue, cigarette butts, etc) gives the image of a safe environment. We can spend all this money to revamp the street scape but without maintaining it - it will become

219 (garbage, residue, organetic butts, etc) gives the image of a safe environment, we can spend at this money to revamp the street scape but without maintaining it "it with become just as we are right now - a decent but not great downtown area. Cleanliness makes one feel safer. Parking meters would be a great addition as well - every city now has online parking meters - easily maintained and convenient.

Downtown will never be a destination for chunks of time vs in and out stops if we don't create a culture of come, stay and play. The farmers market has even turned in to an in and

- out stop because you can't have breakfast or brunch nearby (though I love a coffee stop). We need a place and plan for all day all seasons visitors and residents. I went to the open house tonight at the BCPA and I have VERY mixed feelings. I feel like there is an over emphasis on bike lanes that will cut into parking for all of downtown bloomington businesses and residents. My main dislike is for the North Main Plaza that is in the 600 block of North main.. I have 3 commercial properties and 2 businesses on that
- block that will directly be impacted in a bad way. During events this lot will be barricaded. Also the entrance and exits will be removed from Main Street cutting off flow of traffic to that area. This will cut off traffic to my business and create a dead end landlocked section of Main Street. Patrons will have to backtrack 3-4 blocks in the opposite direction to get back on 51 or center street. This doesn't make any sense to be honest. Don't get me wrong, it looks pretty but pretty doesn't pay your bills. Commerce and tax dollars pay your bills. Whatever hurts business will also hurt the city. Please add an easement lane in North Main plaza to still create this green space but not kill business. Thank you
- 222 Please explain how cars in the 500-600 blocks of Main st get to Locust st or back to Main st to drive north. Please leave Main st open to East st/Main st

1'm on the Trustee board of Second Presbyterian Church and steward of their gardens. We would like to learn more about the plans along East St and Market St. We have a garden on each Street and need to understand the impacts.

As an owner of a business in the 600 block of Main St., the proposal for the amphitheater and removal of vehicle exits hinder the access. This would deter patrons from frequenting this area. This is not an acceptable plan for the area to promote visitors. This also restricts deliveries from being accomplished to keep the businesses open. Step back and view

this area. This is not an acceptable plan for the area to promote visitors. This also restricts delivered for being accomplianed to keep the businesses open. Step back and view this from the owner's point of view and how the decrease in access will affect our income, not to mention your Bev and Sales tax and first responder routes. Making it look pretty like uptown will not promote more traffic when there are no businesses open for them to visit.

Pat Fitzgerald and I attended the first open house on behalf of Crossroads Fair Trade but were unable to attend the open house held Feb. 11. Can you please advise as to where/how we can see the actual downtown renderings showing our location at 428 N. Main Street.

The graphics provided in the online "tour" are exceptional! I didn't see Crossroads depicted, maybe I missed it? As a part of downtown since 1988, I thought maybe you'd want to include it.

225 We are extremely grateful for the attention and resources put into the enhancements in Downtown Bloomington to make it a valuable asset to our community and a primary destination for families and all generations!

Donna Brouillet, Board Member

Crossroads Fair Trade

Cell: 309-824-5631

226

One of the problems of bicycles on sidewalks, has there been any accidents with bicycles and pedestrians? We come to down town Bloomington, to eat, and not be interrupted by a loud pickup or other noisy Camaro or Mustang. How do you have a parking space that also serves seating in amphitheater style seating at another time. Bicyclists currently do not follow the rules on riding on the sidewalks, stopping at stop signs or being careful and respectful on the trail, including uptown Normal. Now add in the skateboarders and scooters, find a way first prior to deal with these abuses.

The priority should be for pedestrian and bicycle safety and comfort. I care a lot less about the design style. This is a rare part of B/N that is somewhat walkable and bikable, so it

227 should be much easier for people of all ages and abilities to get around without needing to drive and park immediately in front of a location. If the streetscape is comfortable with reduced (and slower!) vehicular traffic, people will hang out longer and spend more money at the local businesses. If parking is made scarce or remote it will expedite businesses leaving downtown. a law firm or accounting practices' elderly clients cannot park in a garage and walk multiple blocks during typical winter weather. If they were to fall and hurt themselves they would be at the mercy of hoping someone saw them and came to their rescue

After parking, I care the most about pedestrians. I do not care at all about cyclists. Very few people who work downtown ride their bikes to work, and parts of the year it is not feasible. A bike lane seems like a massive waste of precious road space. I live near Washington and almost never see anyone use the bike lanes.

The current lack of parking, coupled with the parking time limits and aggressiveness with which the enforcement writes tickets, it is already hard to go shopping downtown. If I have to pay to park in a garage to then walk multiple blocks I will just go home instead of looking for parking or waiting for a spot. I know from talking to business owners the tickets have hurt business, and made it hard for employees. It deters from people from spending time downtown when they have to move their vehicle after every, or every other store they visit.

If we rely on garages for parking how aggressively are the sidewalks and crosswalks going to be attacked when bad winter weather arrives?

Can we get more lighting?

downtown's image.

I know it's not an easy problem, but any ideas for the homeless issue downtown?

I don't think the reduced parking or one side of the street parking are an issue (businesswise) if the area is walkable enough for people to actually safely walk downtown. As it stands no one gets to park on the side of the street they want to go to anyway, or usually even on the same block. You take what you can get or just use a parking garage and then you have to navigate car filled streets on foot. If parking is diverted to somewhere just outside of downtown, and the streets of actual downtown are mostly (or preferably to me, entirely)

229 people walking, it becomes a much more pleasant and safe place to spend time.

While you're at it, making it safer to walk into downtown from nearby areas would be nice as well, and beneficial to the businesses and culture downtown. We live under a mile away, an easy distance to walk for us, but our options are: 1) curb tight sidewalks on busy main street, 2) walking through a sparsely populated and run down area with abandoned looking warehouses and homes and having to cross a busy street at the end, or 3) going several blocks out of our way and still having to cross a busy street at the end anyway. What is being proposed for the Front & Center/Peoples Bank Building block? If nothing is being done, no amount of beautification and street enhancement, will improve

230

It would be great to not have cars parked along street where people are trying to enjoy their lunch. Maybe have 15 minute parking in some spots and make sure there is signage to

231 route them to a parking deck near by for longer parking.I like that some restaurants take up the parking spots during the summer.

I am Darrell Hartweg. I own the Illinois House Building and have been the owner/tenant of the Building since 1977. Currently, the Building has 27 tenants; all but one of these tenants (Mystic Kitchen) are counseling offices, insurance offices, social service agencies, and law firms. Every day, approximately 60 persons come to the Building as employees of the tenants. Every tenant provides services to its customers. I estimate that, on average, at least 30 to 40 clients/customers enter the Building every day to meet with one of

- these tenants. Each one of these people depends on their automobiles to deliver them to the Building; these automobiles require parking spaces in the Downtown area . Many of the clients/customers of IHB's tenants are elderly and need parking areas which are close to their ultimate destination whether that be Illinois House or other businesses. ANY reduction in the existing parking areas required to be made to accommodate a new streetscape design would cause great hardship to clients/customers doing business with IHB tenants with a resulting problem accruing to owners of office buildings like me. I STRONGLY oppose any reduction in the present parking spaces in the downtown area especially since there are not a sufficient number of parking spaces presently.
- My biggest problem with diagonal parking is that I have a Camry and when a van, suv, or truck park next to me I have to back out too far into traffic to see if anyone is coming. I like the look and ease of diagonal parking, however, looking at the plans if someone had to swerve to avoid me because I can't see them coming, they are going to swerve into the bike lane.
- 234 Please consider additional space and activities for children and families. Would love to see a greater focus on 3rd places to facilitate community engagement and interactions. Please consider adding additional space and/or businesses woth a focus on younger children - infant / toddlers. There is a heavy focus on adult spaces and would love for more
- 235 opportunities for families to utilize these spaces year round, especially during the winter months.
- 236 Decreased parking and/or close parking may result in fewer people being attracted to downtown despite the improvements. If parking is an issue, they won't come. Once again you are squandering valuable resources to correct a previously failed design which was done to correct a previously failed design. When will you learn that you aren't smart enough to fix the problem. Diagonal parking with today's vehicles is dangerous and just waiting for a lot of rear end accidents. Further, not a damned dime should be spent on
- 237 such ridiculous pipe dreams until you fix the damned rotting infrastructure of this City. Spend the money to fix the roads (without damned bicycle paths and other crap in the way) and stop your constant waste on a few bitching people. Want to satisfy the public? FIX THE DAMNED ROADS AND OTHER INFRASTRUCTURE. If downtown can't make it without this waste of money, maybe downtown is not worth saving, sadly the result of past foolishness by tone deaf politicians and city staff. Lipstick on a pig does not take away the smell.
- 238 Bloomington resident for 10 years and I love this. Very supportive! Maybe someone has already had this idea so forgive me if redundant. I've asked several of my customers if the city provided park and ride buses on Thursday Friday and Saturday
- nights, would you take advantage, and I got a lot of positive feedback. We could ask banks and other day time M-F businesses to be a park and ride bus stop strictly for shuttling downtown. This would really cut down on DUIs and be more reliable than Uber. \$5 rides that run back and forth from downtown from 5-1:30 Thursday Friday and Saturdays. This would fix parking concerns and drunk driving.

The addition of a much too small stage area and a water feature on the courthouse grounds compromises the historic integrity of the building and the surrounding lawns.

The arched stage structure blocks sightlines of the building and frankly, does not blend well with the courthouse itself. It looks forced. If you must do it, skip the arches and incorporate some other lower to the ground structure for plug and play.

The space you are proposing for the water feature is too small and again, does not blend well with the historic architecture surrounding it. Don't compromise that building's integrity for the sake of turning the square into an amusement park.

240

Why not buy the old bank building and parking lot on the corner of Washington and Center Streets and turn that into an area for concerts and gatherings with a larger water feature? (For that matter, find a developer who can turn that old Montgomery Ward's building into a chic boutique hotel.)

I have been visiting the square of Downtown Bloomington regularly since the 1960s. I have seen a lot of change, some good and some not so good. The past decade has seen a reawakening of the downtown area and I applaud it. But, let's leave the historical courthouse grounds alone and focus on reimagining and revitalizing some of the still blighted areas of downtown instead.

Thank you for your consideration. And thank you for wanting to improve the downtown space.

The parking situation in downtown is already bad. If the City really eliminates parking...unless the City is providing a free trolley (or something), I think your dream of "walkability" will negatively impact the businesses that are losing that parking.

In the last month, I tried twice going to Neighbor Thrift for a quick trip on a Saturday. Closest parking was by Abundant Life Thrift Shop. On the first attempt, I chose not to go because of the cold and I didn't want to walk. On the second attempt, I parked by Abundant Life Thrift and attempted to go, but sidewalks were icy so again chose not to go any further and left.

Parallel parking should not be an option. At all. For any of the designs.

Overall, I think the bike lanes are unnecessary for a small scope project like this. They can "park" their bikes and maneuver (the "walkability") like the rest of the people who arrive in cars. For the few summer months, I can see how bike lanes might seem like a good idea; however, this is Illinois, and I don't believe bike lanes are justifiable reasons to eliminate parking year round.

As far as the other design plans:

*definitely like the stage area

I would love to see more sustainability initiatives built into this design. I think Uptown Normal did a great job incorporating sustainability into the circle, and I would love to see

- 242 Bloomington take on the same type of leadership. This could include the addition of swales, EV charging spaces, permeable sidewalks, more trees and greenery, compacting trash receptacles, recycling bins, solar powered elements, etc.
- 243 Im. just extremely concerned about parking and how do we get deliveries ? Everyone does the double parking thing now which works. I have no back entrance options for deliveries & pick ups? 2 FruGALS Thrift 417 n main
- 244 I worry about reduced parking, especially with the new housing development proposed for Washington Street. It is assumed that those who live there will not drive but walk and use public transport but the streets are already jammed with cars now and I fear that reduced parking will have a negative effect on the downtown area.
- Parking in that area is already problematic. I live and work and shop in Bloomington and there have been times I wanted to shop but couldn't find parking that was close enough (I'm in my 70s), so I went home instead. Been a problem for a long time. Anything that reduces parking is counterproductive.

The parallel parking next to the bike lane does not make sense. Parallel parking shouldn't even be in downtown, especially since they are getting rid of the parking garage. I don't

think a bike lane is necessary for downtown. None of these are good options and seem like a waste of time and space and the renovations will take a long time and cramp the space more.

Since our roads are in such terrible conditions, perhaps we should take care of the important items (like infrastructure). Put this on hold till the necessary is taken care of. We are

- retirees on a fixed income and don't need to have taxes increased for this unnecessary extravagance. We are still paying for the coliseum and the cultural center. Maybe we should try to stay within our budget.
- 248 Costs are important as our taxes are a usual drawing card, and with the state increasing our taxes, they need to do the most contributing to a lovely addition. Parking is so important if you want the entire community to enjoy downtown, so with pedestrian areas very important, parking is as well.

Is the next community open house - Wednesday, October 8 or Friday, October 11/

The website shows Wednesday, October 11.

249 What is the correct date?

Please let me know, thanks

Ann Sullivan

- 250 I think as a homeowner in Bloomington, Illinois that until our streets get repaved and are safe to drive on that no more money needs spent on down town. I also think my property taxes are already too high, and dont need the added increases as I near retirement.
- leave downtown alone. Spend the money on infrastructure. Rescind the additional charges on the upcoming water bills and use the money planned for down improvement on infr
- 252 With more and more architecture being modernized, it'd be nice to see the downtown area stay people friendly and continue to have a home-like feel.
- 253 Stop the parking police!!
- Has delivery access from vendors been considered on the street design? I chose Design C based on this, it looks like delivery trucks could park to unload. Thinking of Amazon, Food suppliers, moving trucks, etc.
- 255 Make parking accommodations for those that work downtown (ex: apply for free garage parking pass with proof of employment)

- 256 Parking accommodation for people who work downtown. If you work downtown, you either have to move your car every couple of hours and risk parking tickets, or buy a parking pass, which is frankly overpriced and not worth it if you don't live downtown.
- 257 Give Downtown Business Owners Parking Passes for their employees. Increase restaurant varieties. Do public scooter/bike transport (diiv). Add seating for bus stops that lack one.
- 258 I'm Glad It's About Time They Do Something Down Town I Might Start Coming Back
- 259 Most city streets in bad shape. Use money to fix them first. Most people will not go downtown unless an event like farmers market happens. Reason: parking hard to find. Don't have less parking!

Downtown visitors already find existing amount of parking to be an obstacle. Removing street parking and forcing people to the parking garages (that usually have homeless lingering outside) is not a good idea if the goal is to bring more people downtown. If we're going to create more walking space and lose parking, can something be done about the

- 260 multiple private lots that aren't used by downtown businesses and tow cars? Why isn't the city buying those back from private owners or cutting a deal with owners to allow downtown visitors and residents to use those lots during certain hours?
- 261 Please make actual safe and well lit parking garages to accommodate the loss of parking. As a female, I don't feel safe using the parking garage but I would love to be able to spend more time in Downtown Bloomington but parking is always an issue. More pedestrian pathways would be amazing but we need better parking.
- 262 These renderings are missing all the homeless people and tents.
- 263 There so much more we could do with the tax dollars in our community, over the last 15 years my property taxes have increased dramatically. Let's take care of our Locust St from 150 to Bloomington Country Club and Empire from Country Club to Main St.

As an EV driver for over 10 years I would suggest that charging stations not be placed on the streets. The one in Miller Park is very hard to use, I witnessed a EV facing the wrong way

on the one-way street to charge. The chargers work best in a dedicated area with access from both sides, more like a gas pump. Also, I believe it is best to have the electricity paid for by the user, not the taxpayers. I would suggest Charge point or Electrify America

Public Voting Results

Public Voting Results (as of 3/23/24)

Open House	Votes
Alternate A	11
Alternate B	72
Alternate C	31
Classic/Tradition	38
Modern/Contem	10
Hybrid Blend	73

Website	Votes
Alternate A	64
Alternate B	117
Alternate C	77
Classic/Tradition	90
Modern/Contem	43
Hybrid Blend	126

Total	Votes	%
Alternate A	75	20%
Alternate B	189	51%
Alternate C	108	29%
Classic/Tradition	128	34%
Modern/Contem	53	14%
Hybrid Blend	199	52%

City Utility Information



Downtown Streetscape - Watermain Evaluation

3.28.2023	
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		-		r							
No. Street	From	To	Size	Install Date	Location Repair / Break History	Useful life (5-10 yrs.)	Useful life (10-20 yrs.)	Useful life (>20 yrs.	.) Notes	Recommendation	Hydrants
1 Olive	Lee	Roosevelt	12"	2005	Street None	Yes	Yes	Yes			Hyd 423 Waterous
2 Olive	Roosevelt	Madison	12"	2005	Street 2013	Yes	Yes	Yes			Hyd 424 Waterous, Hyd 359 Iowa, Hyd 425 Iowa
3 Olive	Madison	Center	6"	1895	Street 2013	Yes	Yes	potential	128 year pipe	Potential replacement after 20 years	Hyd 426 Corey
4 Olive	Center	Main	401	2017	No Water Main None	N.					11 1 400 1
5 Olive	Fact	EdSL	12	2017	Parkway/Sidewalk None	Yes	Tes	Yes			Hyd 427 Iowa
7 Grove	Albert	Prairie	10	1934	South side sidewalk None	Vec	Vec	Ves			Hyd 773 Eddy
8 Front	lee	Roosevelt	10	2005	north edge of WB travel lane None	Yes	Yes	Yes			Hyd 354 Waterous
9 Front	Roosevelt	Madison	12"	2005	north edge of WB travel lane None	Yes	Yes	Yes			Hyd 3304
40 5 1		c	41	4075					no real issues. Not in 20 year plan. Very aged -		u larruu
10 Front	Madison	Center	4	18/5	north edge of WB travel lane not much recent, 2009	yes	yes	potential	potential for breaks but history ok.	Main remains, Potential replacement after 20 years	Hyd 355 Waterous
11 Front	Center	Main	6"	1890	north edge of WB travel lane 2016	Yes	Yes	potential	133 year pipe	Potential replacement after 20 years	Hyd 356 Clow 2500
12 Front	Main	Fast	6"	1916	north edge of WB travel lane 2012 2012 2019 one without date	No	No	No	Would recommend replacing watermain with		Hvd 357 Clow 2500
12 110110	i i i i i i i i i i i i i i i i i i i	LUST	ő	1510					the number of breaks.		194 557 6101 2500
13 Front	East	Prairie	6"	1916	north edge of WB travel lane 2018, 2012, 2009, 2012, 2018, 2017	No	No	No	Would recommend replacing watermain with		Hyd 358 Iowa
		a	401	4054	5 // / D000	N	×.		the number of breaks.		
14 Washington	Lee	Rooseveit	12	1901	South parkway 2009	res	tes	Potential		Potential replacement after 20 years	Hyd 320 Iowa
15 Washington	Roosevelt	Madison	12"	1961	EB travel lane	Yes	Yes	Potential		Potential replacement after 20 years	Hyd 321 Clow Medallion
16 Washington	Madison	Center	16"	1913	north edge of WB travel lane 2003	Yes	Potential	Potential	110 year pipe	Potential replacement after 10 years	Hvd 322 Iowa
17 Washington	Center	Main	16"	1913	north edge of WB travel lane None	Yes	Yes	Potential	110 year pipe	Potential replacement after 20 years	Hyd 323 Iowa
18 Washington	Main	East	16"	1913	north edge of WB travel lane None	Yes	Yes	Potential	110 year pipe	Potential replacement after 20 years	Hyd 324 Clow 2500
19 Washington	East	Prairie	12"	1914	north edge of WB travel lane None	Yes	Yes	Potential	109 year pipe	Potential replacement after 20 years	Hyd 325 Waterous
20 Jefferson	Roosevelt	Madison	6"	1893	north edge of WB travel lane None	Yes	Potential	Potential	130 year pipe	Potential replacement after 10 years	Hyd 289 Waterous, Hyd 290 Iowa
21 Jefferson	Madison	Center	6"	1893	north edge of WB travel lane None	Yes	Potential	Potential	130 year pipe	Potential replacement after 10 years	
22 Jefferson	Center	Main			No Water Main						Hyd 291 Waterous
23 Jefferson	Main	East		2617	No Water Main	N.	N.				Hyd 292 Waterous
24 Jetferson	East	Prairie	8"	2015	north edge of WB travel lane 2012,2013, two more without dates	Yes	Yes	Yes	Breaks occurred prior to the new water main	Detection and a first 20	Hya 293 Waterous
25 Monroe	Koosevelt	Contor	10"	1900	north edge of WB travel lane None	Yes	res	Potential	123 year pipe	Potential replacement after 20 years	nyu zo1 Waterous
20 IVIOIITO8	Center	Main	10	1900	No Water Main	Tes	yes	Potential	123 year pipe	Add main for unserved properties	Hvd 263 Jowa
27 1010100	Center		+		Main only extends half the				-	rea man for unserved properties	
28 Monroe	Main	East	6"	1996	block, south EB travel lane	Yes	Yes	Yes			Hyd 264 Waterous
			c 11	4070	Between WB driving lane and	м					u 1999 di 9599
29 Market	Rooseveit	Madison	6	1879	turn lane.	Yes	potential	potential	144 year pipe	Potential replacement after 10 years	Hyd 239 Clow 2500
30 Market	Madison	Center	6"	1879	Between WB driving lane and None	Yes	notential	notential	144 year nine	Potential replacement after 10 years	Hvd 240 Iowa
			-		turn lane.						
31 Market	Center	Main	6"	1875	Between WB driving lane and None	Yes	potential	potential	148 year pipe	Potential replacement after 10 years	Hyd 241 Iowa
32 Market	Main	Fact			turn lane. No Water Main					Add main for unserved properties	Hvd 242 Clow 2500
52 WINKEL	IVIGIII	Last			North edge along parking					Add main for diserved properties	11yu 242 Clow 2500
33 Market	East	Prairie	6"	1897	spots	Yes	potential	potential	126 year pipe	Potential replacement after 10 years	Hyd 243 Waterous
34 Douglas	East	Prairie	8"	1995	Sidewalk None	Yes	Yes	Yes			Hyd 220 Waterous
35 Mulberry	Roosevelt	Madison	6"	1899	North edge of WB travel lane	Yes	notential	notential	124 year nine	Potential replacement after 10 years	Hvd 210 Corev 22 Hvd 211 Jowa
55 Maiberry	nooseven	maaison	ő	1055	along curb	103	potential	potential	ie year pipe	rotential replacement after 10 years	nya zio corcy, nya zizi toma
36 Mulberry	Center	Main			No Water Main						Hyd 212 Iowa
37 Madison	Olive	Front	10"	1900	Olive Street None	Yes	potential	potential	123 year pipe	Potential replacement after 10 years	Hyd 425 Iowa, Hyd 388 Waterous
					From east side of curb						
38 Madison	Front	Washington	10"	1900	between lane one and two	Yes	potential	potential	123 year pipe	Potential replacement after 10 years	Hyd 355 Iowa, Hyd 3304 Waterous
20 Madiana	March in sta		10"	1000	From east side of curb	¥	and and int	a stantial	133		Und 222 James
39 Iviadison	wasningto	n Jerrerson	10	1900	between lane one and two	res	potential	potential	123 year pipe	Potential replacement after 10 years	Hýd 322 Iowa
40 Madison	Jefferson	Monroe	10"	1900	From east side of curb	Yes	potential	potential	123 year pipe	Potential replacement after 10 years	Hvd 290 Iowa
					between lane one and two						,
41 Madison	Monroe	Market	6"	1985	None None	Yes	Yes	Yes			Hyd 262 Iowa
					From east side of curb						
42 Madison	Market	Mulberry	6"	1896	2003 2003	Yes	potential	potential	127 year pipe	Potential replacement after 10 years	Hyd 240 Iowa
42. 14. 15			C 1	4000	Runs through the parking lot				107		
43 IVIADISON	www.erry	LOCUSE	0	1990	of 202 Locust St.	res	potential	potential	127 year pipe	Fotential replacement after 10 years	nyu 211 IOWa
44 Center	Kentucky	Olive	6"	1875	Along bridge wall east side of None	Yes	notential	notential	148 year nine	Potential replacement after 10 years	
			-	-575	bridge		Persinta	p - territor	· / p-p-	10 / 2013	
	- ·		C 11	4075	under parking spots on east	X					U 1955 0 5 9599
45 Center	Front	wasnington	0	18/5	under sidewalk hump out	res	potential	potential	148 year pipe	Potential replacement after 10 years	Hyd 356 Corey F-2500
					under parking spots on east						
46 Center	Washingto	n Jefferson	20"	1913	parking/water main runs 2018, 2019	potential	potential	potential	110 year pipe		Hvd 323 Waterous, Hvd 291 Waterous
					under sidewalk bump out			• • • • •			,
					under parking spots on east						
47 Center	Jefferson	Monroe	20"	1913	parking/ water main runs None	Yes	Yes	potential	110 year pipe	Potential replacement after 20 years	
					under sidewalk bump out						
48 Center	Monroe	Market	20"	1913	Between parallel parking and None	Yes	Yes	potential	110 year pipe	Potential replacement after 20 years	Hyd 263 Waterous
					NB traffic lane						
49 Center	Market	Mulberry	20"	1913	None NB traffic lane	Yes	Yes	potential	110 year pipe	Potential replacement after 20 years	Hyd 241 Iowa
50 Center	Mulberry	Locust	20"	1913	Along parallel parking None	Yes	Yes	potential	110 year pipe	Potential replacement after 20 years	Hyd 212 Iowa
	1				Runs through all 4 lanes of						
51 Main	Kentucky	Olive	8"	1875	NB traffic lanes and through None	Yes	potential	potential	148 year pipe	Potential replacement after 5 years	Hyd 756 Corey
		1	1		grass island at Olive St.						
			1		Main runs through the						
53 M	Olive	Front	10"	1000	parking lot along side of the	v	v				Hud 2624 Clow E 2500 Hud 257 Weterson
52 IVIdITI	Olive	FIONL	12	1990	the Fact Street Parking	Tes	res	res			nyu 5054 Clow F-2500, nyu 557 Waterous
			1		Garage.						

3.2	8.2	02	3	

										1			
No. Stre	et From		<u>To</u>	Size	Install Date	e Location	Repair / Break History	Useful life (5-10 yrs.)	Useful life (10-20 yrs.)	Useful life (>20 yrs.)	Notes	tecommendation	Hydrants
						At Front Street, main runs							
						under the sidewalk bump							
						north under the narking							
52 Mair	Front		Washington	12"	1996	spots on the east side of	None	Yes	Yes	Yes			Hyd 3634 Clow F-2500, Hyd 357 Waterous
						Main St. Water main runs							
						under the bump out at							
						Washington St.							
						At Washington St, main runs							
						under the sidewalk bump							
						out and main continues							
53 Mair	washin	ington	Jefferson	12"	1996	north under the parking	None	Yes	Yes	Yes			Hyd 324 Clow F-2500, Hyd 292 Waterous
						Main St. Water main runs							
						under the bump out at							
						Jefferson St.							
						At Jefferson Street, main							
						runs under the sidewalk							
						bump out and main							
54 Maii	Jefferso	son	Monroe	12"	1996	continues north under the	None	Yes	Yes	Yes			
						side of Main St. Water main							
						runs under the bump out at							
						Monroe St.							
						Water main runs in the							
						middle of the east NB lane,							
55 Mair	Monro	oe	Market	12"	1996	before Market St. the water	None	Yes	Yes	Yes			Hyd 264 Waterous
						main transitions into the							
						east side walk Main runs under the east							
56 Mair	Market	et	Mulberry	12"	1994	sidewalk at the curb line	None	Yes	Yes	Yes			Hyd 242 Clow F-2500, Hyd 3145 Clow F-2500
						Main runs under the							
						sidewalk bump out and							
						continues north under the							
						parking spots between							
						Mulberry and Locust and							
						heads North under all four							
57 Mair	Mulber	erry	Locust	12"	1994	ianes of the NB traffic. At	None	Yes	Yes	Yes			
						and runs on the west side of							
						Main Street under the							
						parking spots on the west							
						side of Main. and ties back							
						into the 12" WM just North							
						of the west parking spots.							
58 East	Olive		Grove	16"	1973	water main runs under the	None	Yes	Yes	Yes			
						Water main runs under the							
59 East	Grove	•	Front	16"	1973	east side sidewalk	None	Yes	Yes	Yes			
						Water main runs under the							
	1					east side sidewalk. At							
60 East	Front		Washington	16"	1973	Washington St. the main	None	Yes	Yes	Yes			Hyd 358 Iowa
	1					transitions under the east							
\vdash						Water main is in the cast NP							
	1			1		traffic lane before transiting							
61 East	Washin	ington	Jefferson	8"	1991	back under the east	None	Yes	Yes	Yes			Hyd 325 Waterous
	1					sidewalk.							
62 Ear+	lafform	son	Monroe	8"	100/	WM runs under the east	None	Yes	Yes	Yes			HVD 293 Waterous
OZ EdSL	Jerlerso	3011	wonoe	0	1554	sidewalk	None	ies	res	162			1110 200 Walerous
63 East	Monro	oe	Market	8"	1994	WM runs under the east	None	Yes	Yes	Yes			Hyd 265 Waterous
\vdash						runs along the east curb line		+					+
64 East	Market	et	Douglas	8"	1995	and the NB traffic lane.	None	Yes	Yes	Yes			Hyd 243 Waterous
						Runs along the east							
	1			1		sidewalk into the green							
	1			1		space in front of the							
65 East	Douela	as	Mulberry	8"	2004	Bloomington Center for the	None	Yes	Yes	Yes			Hyd 220 Waterous
		-	,			Performing Arts. A 12" WM							
	1					runs between Main St. and							
				1		in front of the BCPA							

 Notes

 1
 All fire hydrams older than 10 years impacted by Streetscape features should be replaced.

 2
 Values will need to be evaluated and determined if replacement is needed based on proximity and impacts from roadway and streetscape features.

 3
 Service line material will need to be reviewed. Service line replacement will be needed when lead is present. Further review to be performed as detailed design development occurs.

 4
 Proposed roadway reconstruction, curb line placement, and streetscape features may impact the recommendations for water main replacement. Further coordination during design development.

Considerations 1 Size - flow, capacity

2 location within ROW

3 repair history 4 expected "short term" life expectancy

5 unserved properties 6 master plan identified fire flows

Traffic Calculations
Downtown Bloomington Streetscaping Project HCS Traffic Model Results 5/3/2023



				2	024 - 2 Lane	s, No Parkir	g			2	044 - 2 Lane	s, No Parkin	g	
				AM			PM			AM			PM	
Intersection	App.	Max Q	Delay (s)	LOS	95%ile Q	Delay (s)	LOS	95%ile Q	Delay (s)	LOS	95%ile Q	Delay (s)	LOS	95%ile Q
	EB	454	29.8	С	139	29.6	С	129	29.5	С	154	29.3	С	142
Locust & Madison	SB	326	9.3	Α	220	10.0	Α	243	11.6	В	270	12.8	В	307
	Int.		15.4	В		15.2	В		16.9	В		17.2	В	
	EB	194	28.3	С	167	28.3	С	181	27.8	С	185	27.0	С	194
Locust & East	NB	790	8.2	Α	181	9.1	А	203	9.8	Α	217	10.7	В	231
	Int.		15.6	В		16.1	В		16.4	В		16.6	В	
	EB	192	33.0	С	158	31.9	С	192	32.9	С	174	34.2	С	216
	WB	182	26.7	C	68	25.2	C	72	26.2	C	75	24.8	C	78
Market & Madison	SB	787	8.2	A	182	11.1	В	240	10.1	В	223	13.9	В	291
	Int.		12.8	B		15.3	B		14.2	B		17.7	B	
	EB	427	34.1	С	65	33.8	С	134	34.7	С	73	34.6	C	148
Market & Fast N	NB	197	3.0	A	51	4 5	A	71	3.6	A	61	5.7	A	88
indirice di Editerri	Int.	107	4.9	A	51	7.7	A	/-	5.4	A	01	8.8	A	
	FB	193	34.8	C	4	35.0	C	50	34.8	C	4	34.6	C	56
Monroe & Fast	NB	237	1.6	Δ	31	2.0	Δ	38	17	Δ	35	22	Δ	44
Wollie d'East	Int	237	1.0	Δ	51	3.4	Δ	50	1.7	Δ	55	3.6	Δ	
	ED.	101	26.7	C C	20	25.4	<u> </u>	25	20.0	<u> </u>	21	28.0	<u> </u>	20
	LD W/B	103	20.7	C	15	25.4	<u> </u>	23	29.0	C	18	28.0	<u> </u>	23
Jefferson & Madison	SB	252	5.6	<u>د</u>	120	7.6	<u>د</u>	168	23.0	Δ	10	66	^	152
	Int	252	5.0	^	120	9.0 8.7		100	8.2		155	8.0		155
	ED.	202	20.4	A	20	20.2	A	42	20.0	A (21	20.6	A	40
		205	29.4	C	29	29.5	<u> </u>	45	29.9	<u> </u>	17	29.0		49
Jefferson & East	ND	424	20.7	ر ۱	14	20.5	<u>ر</u>	34	50.1	<u>ر</u>	162	20.7	<u>ر</u>	30
	IND	257	0.5	A	155	0.7	A	102	0.2	A	102	7.2	A	1/9
	int.	402	7.5	A	112	0.5	A	442	7.5	A		9.0	A	422
	EB	192	31.5	C	112	30.7	<u> </u>	113	21.0	ι 	114	25.5	<u> </u>	122
Washington & Madison	VV B	193	34.7	L A	192	33.4	L A	182	23.4	L D	193	27.0		192
	SB	240	5.8	A	142	0.3	A	164	13.5	B	212	11.1	B	230
	int.	402	13.6	В	4.60	13.1	В	474	16.1	В	400	15.2	В	400
	EB	193	28.7	C	168	27.6	<u> </u>	1/4	29.5	0	188	28.7	0	193
Washington & East	WB	417	33.2	C	253	29.0	<u> </u>	238	33.4	<u> </u>	2/8	34.4	<u> </u>	286
	NB	239	8.3	A	181	9.0	A	180	10.1	В	217	9.7	A	201
	int.		16.7	В		16.5	В		18.0	В		18.4	В	
	EB	191	34.1	C	8/	33.1	<u> </u>	41	33.2	0	101	33.1	0	46
Front & Madison	WB	191	32.9	C	33	34.4	<u> </u>	56	29.6	<u> </u>	35	34.4	<u> </u>	61
	SB	230	5.7	A	140	5.5	A	154	8.1	A	196	6.1	A	175
	int.		9.5	A		9.0	A		11.4	В		9.5	A	
	EB	200	30.7	C	27	31.5	0	104	28.0	0	30	28.8	0	110
Front & East	WB	411	32.5	C	126	28.7	<u> </u>	41	28.2	0	130	26.7	<u> </u>	48
	NB	564	6.5	A	161	6.8	A	160	9.6	A	225	8.9	A	203
	Int.		11.1	В		12.3	В		13.0	В		13.4	В	
	EB	181	26.9	C	46	33.8	C	83	28.4	C	53	33.3	C	87
Olive & Madison	WB	282	25.8	C	25	32.1	C	28	27.0	C	29	30.5	C	29
	SB	570	7.7	A	163	5.6	A	132	7.6	A	173	7.6	A	209
	Int.	ļ	10.0	Α		9.0	Α		10.0	В	ļ	10.7	В	
	EB	271	30.7	C	88	32.7	С	134	34.9	С	112	28.6	С	138
Olive & East	WB	287	28.9	C	21	26.7	С	25	29.1	С	22	23.5	С	26
	NB	224	7.3	A	166	7.9	A	161	7.8	Α	193	11.3	В	223
1	Int.	1	10.2	В	1	12.3	В		11.0	в		14.3	В	

Downtown Bloomington Streetscape - Signal Vs All-Way Stop Analysis 11/14/2023



				20)24					20)44		
Interception	A nn		AM			PM			AM			PM	
intersection	App.			95th %									
		Delay (s)	LOS	Queue									
	EB	25.9	С	48	26.0	С	118	25.6	С	53	25.5	С	123
Market & Center	WB	27.3	С	98	25.8	С	103	27.2	С	108	25.8	С	120
(SIGNAL)	SB	3.7	А	18	4.6	Α	20	3.9	Α	20	5.0	А	25
	Int.	16.4	В		18.3	В		16.4	В		18.2	В	
	EB	27.3	С	38	27.1	С	130	26.9	С	43	26.9	С	145
Market & Main	WB	30.1	С	85	26.1	С	95	30.0	С	95	25.4	С	103
(SIGNAL)	NB	2.9	А	8	4.2	А	13	3.1	А	8	4.7	А	18
(SIGNAL)	Int.	20.0	С		19.8	В		19.9	В		19.6	В	
	EB	7.9	А	8	8.9	А	23	8.0	А	8	9.2	А	25
Market & Center	WB	8.6	А	18	9.0	А	20	8.8	А	20	9.3	А	23
(ALL WAY)	SB	8.6	А	13	9.0	А	15	8.7	А	13	9.2	А	18
	Int.	8.4	Α		9.0	Α		8.6	Α		9.3	Α	
	EB	8.6	А	8	9.2	А	10	8.7	А	8	9.4	А	13
Market & Main (ALL	WB	7.6	А	5	8.2	А	8	7.9	А	5	8.4	А	8
WAY)	NB	7.9	А	13	8.8	А	23	8.1	А	15	9.1	А	28
	Int.	8.0	Α		8.8	Α		8.2	Α		9.1	Α	

HCM 6 Delays and LOS

			Ű							ý					
General Inform	nation								Interse	ction Inf	ormati	on	4	***	s l <u>a</u>
Agency									Duratio	ո, h	0.250)		Ļ	
Analvst				Analys	is Date	4/12/2	023		Area Tv	pe	CBD		 		۲. ۲.
Jurisdiction				Time F	eriod				PHF	<u>.</u>	0.85			W A E	~_2
Urban Street		East		Analys	is Year	2023			Analysi	s Period	1> 8:	00			+- *7
Intersection		East & Front		File Na	me	2 lane	- East 2	2024	AM S.xu	s				ৰ দ	
Project Descrip	tion	2024 AM											1	치 † ጥ ነ ነ	* (*
Demand Inform	nation				EB			N	/B		NB			SB	
Approach Move	ement			L	Т	R	L		r r	L	Т	R	L	Т	R
Demand (v), v	eh/h			13	30			12	27 36	40	904	16		0	
Signal Informa	tion					5				_					
	80.0	Reference Phase	2	e	+	la ≥	=						KT Z		~
Offset s	64	Reference Point	2 Begin									1	2	3	4
Uncoordinated	No	Simult Can E/M	On	Green	57.0	12.2	0.0	0.0	0.0	0.0	_				A
Force Mode	Fixed	Simult, Gap N/S	On	Red	3.2	3.2	0.0	0.0		0.0	_	5	6	7	. 8
T OFCE MODE	TIXCU	onnuit. Cap 14/0	Oll	Red	2.7	2.0	0.0	0.	0 10.0	0.0		Ũ	, in the second		
Timer Results	_			EBL		EBT	WB	L	WBT	NBI	_	NBT	SBL		SBT
Assigned Phase	e					4			8			2			6
Case Number				<u> </u>		6.0			7.0			8.0			8.0
Phase Duration	I, S					17.4			17.4			62.6			62.6
Change Period	, (Y+R (c), S				5.2			5.2			5.6			5.6
Max Allow Head	eadway (<i>MAH</i>), s					4.2			4.2			0.0			0.0
Queue Clearan	Clearance Time (g_s), s					9.7			8.7						
Green Extensio	n Time	(g _e), s				0.7			0.7			0.0			0.0
Phase Call Pro	bability					1.00			1.00						
Max Out Proba	bility					0.00			0.00						
Movement Gro		ulte			EB			\٨/٢	3	_	NB			SB	
Approach Move	ment	Suits		1	 	R		Т	, R		T	R		т	R
Assigned Move	ment			7	4			8	18	5	2	12		6	
Adjusted Flow F	Rate (v), veh/h		15	35			149	9 42	570	_	522		0	
Adjusted Satura	ation Flo	w Rate (s), veh/h/	In	1078	1759			165	7 1362	1665		1529		1635	
Queue Service	Time (d	as). S		1.1	1.4			6.7	2.2	0.0		12.3		0.0	
Cvcle Queue C	learance	e Time (<i>q</i> c). s		7.7	1.4			6.7	2.2	12.1		12.3		0.0	
Green Ratio (g	/C)			0.15	0.15			0.1	5 0.15	0.71		0.71		0.71	
Capacity (c), v	/eh/h			164	268			252	2 208	1235		1090		1166	
Volume-to-Cap	acity Ra	tio(X)		0.093	0.132			0.59	0.204	0.461		0.479		0.000	
Back of Queue	(Q), ft	/In (95 th percentile	e)	13.1	26.9			126	.1 33.7	161.3		151		0	
Back of Queue	(Q), ve	eh/In (95 th percent	ile)	0.5	1.0			4.9) 1.3	6.5		6.0		0.0	
Queue Storage	Ratio (RQ) (95 th percen	tile)	0.15	0.00			0.0	0 0.34	0.00		0.00		0.00	
Uniform Delay	(d 1), s/	/veh		34.5	28.7			30.	9 29.7	5.3		5.3		0.0	
Incremental De	elay (d 1), s/veh al Delay (d 2), s/veh			0.2	0.2			2.2	2 0.5	1.1		1.3		0.0	
Initial Queue De	ue Delay (d 3), s/veh			0.0	0.0			0.0	0.0	0.0		0.0		0.0	
Control Delay (ontrol Delay (d), s/veh			34.7	28.9			33.	1 30.1	6.4		6.6		0.0	
Level of Service	I of Service (LOS)			С	С			С	С	Α		A			
Approach Delay	pproach Delay, s/veh / LOS			30.7		С	32.5	5	С	6.5		А	0.0		
Intersection De	ntersection Delay, s/veh / LOS					11	.1						В		
Multime et al D	ultimodal Results							10/7						00	
Redestrian L CC	trian LOS Score / LOS			1.00	EB	P	1.04			4.05	NR	P	1.05	SB	P
Biovole LOS So				1.93		B	1.94	† 2	B	1.00		B	1.00		B
	JIG / LU			1.04		U	1.00	,	В	2.48		U	1.00		U

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			- 5							,					
General Inform	nation								Intersec	tion Info	ormatio	on		****	× L
Agency									Duration	. h	0.250)		Ļ	
Analyst				Analys	is Date	4/12/2	023		Area Typ	e.	CBD		 		۲. ۲.
Jurisdiction				Time F	Period				PHF		0.83		→ -{	wļe	
Urban Street		Fast		Analys	is Year	2023			Analysis	Period	1> 8:0	00	→		+ ∵
Intersection		East & Jefferson		File Na	ame	2 lane	- East 2	2024	AM S.xus					.	
Project Descrip	tion	2024 AM											1	1 1 1 1 1 1 1	* (*
													1		
Demand Inform	nation				EB			W	В		NB			SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Demand (v), v	eh/h			6	23			1'	1 4	21	947	26		0	
				li.											
Signal Informa	ition			4	↓		-								_
Cycle, s	80.0	Reference Phase	2		51	,						1		3	4
Offset, s	56	Reference Point	Begin	Green	56.8	12.8	0.0	0.0	0.0	0.0					5
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0	0.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.0	2.0	0.0	0.0	0.0	0.0		5	6	7	8
			_										0.51		
Timer Results				EBL	· -	EBT	WBI		WBT	NBL		NBT	SBL	-	SBT
Assigned Phase	e					4		_	8		_	2			6
Case Number						8.0		_	8.0		_	8.0			8.0
Phase Duration	i, S	```				18.0		_	18.0		_	62.0			62.0
Change Period	, (Y+R a	c), S			_	5.2		_	5.2	<u> </u>	_	5.2			5.2
Max Allow Head	ax Allow Headway (<i>MAH</i>), s				_	4.1		-	4.1		_	0.0		_	0.0
Queue Clearan		(gs), s				3.3		-	2.8		_	0.0		_	0.0
Green Extensio	n lime	(ge), s				0.1			0.1			0.0			0.0
Phase Call Pro						1.00			0.69					_	
Max Out Proba	DIIILY					0.00			0.00						
Movement Gro	oup Res	ults			EB			WB	;		NB			SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Assigned Move	ment			7	4			8	18	5	2	12		6	
Adjusted Flow I	Rate (v), veh/h			35			18		582		527		0	
Adjusted Satura	ation Flo	w Rate (s), veh/h/l	In		1706			1594	1	1660		1504		1622	
Queue Service	Time (g	gs), s			0.0			0.8		0.0		12.5		0.0	
Cycle Queue C	learance	e Time (g c), s			1.3			0.8		12.4		12.5		0.0	
Green Ratio (g	/C)				0.16			0.16	;	0.71		0.71		0.71	
Capacity (c), v	/eh/h				327			255		1226		1068		1152	
Volume-to-Cap	acity Ra	tio(X)			0.107			0.07	1	0.475		0.493		0.000	
Back of Queue	(Q), ft	/In (95 th percentile	e)		28.7			13.7	′	154.9		145.7		0	
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		1.1			0.5		6.2		5.8		0.0	
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00			0.00		0.00		0.00		0.00	
Uniform Delay	(d 1), s/	/veh			28.8			28.5	;	5.2		5.2		0.0	
Incremental De	lay (<i>d</i> 2), s/veh			0.7			0.1		1.0		1.2		0.0	
Initial Queue De	nitial Queue Delay (d 3), s/veh				0.0			0.0		0.0		0.0		0.0	
Control Delay (Control Delay (<i>d</i>), s/veh				29.4			28.7	<u> </u>	6.2		6.4		0.0	
_evel of Service (LOS)					С			С		A		A			
Approach Delay, s/veh / LOS				29.4		С	28.7	7	С	6.3		А	0.0		
ntersection Delay, s/veh / LOS						7.	3						A		
Multimodal Re	Iultimodal Results				EB			WB			NB			SB	
Pedestrian LOS	Score	/LOS		1.93		В	1.94	F	В	1.62		В	1.63	;	В
Bicycle LOS Sc	edestrian LOS Score / LOS cycle LOS Score / LOS					В	1.59		В	2.55		С	1.55	5	В

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		<i>.</i>															
General Information		_							In	torsect	ion Infr	ormatic		2	444	t bi lu	
										uration	h	0 250	,,,,				
Apolyet			Analys	ie Dat	o 1/12	120	123			rea Type		CBD					₹
Analysi			Time		C 4/12	/20	123			пеа туре ше	5	0.07		→ - 4 - →	w↓e		<u>≻</u>
	Foot				r 2022	>					Dariad	1 0.07	0				1.
			Analys	is rea	1 2023) 	Fact	0004			Peniod	12 0.0	0				
Intersection			File Na	ame	Ziar	ie -	- East 2	2024		/i in.xus				- 5	¶ ¶ ••• • •••	a to a	
Project Description	2024 AM																
Demand Information				EB				V	VB			NB			SB		
Approach Movement			L	Т	R		L	Τ	Т	R	L	Т	R	L	Т		R
Demand (v), veh/h			83	478	-			\square				842	93				
			0														
Signal Information	v	2			7		1										
Cycle, s 70.0	Reference Phase	2		1	, ⊨≍								1	P			→
Offset, s 50	Reference Point	Begin	Green	45.9	13 1	1 1	0.0	0	0	0.0	0.0	_	1	2			4
Uncoordinated No	Simult. Gap E/W	On	Yellow	3.2	3.2	•	0.0	0.	.0	0.0	0.0						
Force Mode Fixed	Simult. Gap N/S	On	Red	2.3	2.3		0.0	0.	.0	0.0	0.0		5	6	7		8
Timer Results			EBL	-	EBT	4	WBI	-	V	NBT	NBL	-	NBT	SBL	_	SB	т
Assigned Phase					4								2				
Case Number					12.0								8.0				
Phase Duration, s					18.6								51.4				
Change Period, (Y+R	c), S				5.5								5.5				
Max Allow Headway (I	(MAH), s				4.1								0.0				
Queue Clearance Time	ime (g_s) , s				11.2												
Green Extension Time	(ge), s				1.9								0.0				
Phase Call Probability					1.00												
Max Out Probability					0.17												
				ED		-		14/				NID			0.0		
Movement Group Res	Suits			EB	Б	╉		VV T	B	D			D		5B 	_	_
Approach Novement				1	R	╉		1	\rightarrow	R		1	R 10	L	- 1	+-	ĸ
Assigned Movement	·		/	4		╉			-			Z				+-	
Adjusted Flow Rate (V	'), ven/n		227	418		╉			\rightarrow			5/1	551			+-	_
Adjusted Saturation Fig	ow Rate (s), ven/n/l	In	1622	1657		╉			_			1657	1598			+	
Queue Service Time (g s), s		9.2	8.2		+			\rightarrow			15.5	13.1			+-	
Cycle Queue Clearanc	e Time (<i>g</i> c), s		9.2	8.2		+			_			15.5	13.1			+	
Green Ratio (g/C)			0.19	0.19		+			_			0.66	0.66			+-	
Capacity (c), veh/h			303	619		+			_			1087	1048			+-	
Volume-to-Capacity Ra	atio (X)	<u>, </u>	0.747	0.675		+			_			0.525	0.526			+-	
Back of Queue (Q), f	Vin (95 th percentile	e)	167	146.4	-	+			_			180.5	1/0.7			+	
Back of Queue (Q), Ve	en/In (95 th percent	lle) tile)	6.6	5.7		╉			\rightarrow			7.0	6.8			+-	_
Queue Storage Ralio (RQ) (95 in percen	uie)	0.00	0.00		÷			\rightarrow			0.00	0.00			+-	
Uniform Delay (<i>a</i> 1), s			20.4	20.0		╉			\rightarrow			0.7	0.7			+-	_
Incremental Delay (a 2	Delay (<i>d</i> ₂), s/veh			1.3		╉			-			1.5	1.0			+	
Control Delay (d	al Queue Delay (d_3), s/veh			0.0		╉						0.0	0.0				
Lovel of Service (LCC)	vel of Service (LOS)			21.3	-	+						0.2	0.3			-	
Level of Service (LOS)	l of Service (LOS) oach Delay, s/yeb / LOS					╉						A	A				
Approach Delay, s/Ven		28.3		U	15	0.0				ö.2		A	0.0				
mersection Delay, s/ve	tion Delay, s/veh / LOS					15.	0						1	D			
Multimodal Results	lts			FB				W	B			NB			SB		
Pedestrian I OS Score	/LOS	1 71		В	╉	1.72		_	В	1.85		В	1 95		R		
Bicycle LOS Score / LO	DS		1.91		В						2.45		В	0.00		A	

			s eigi					004			mary					
General Inform	nation								Inters	ecti	ion Info	ormatio	on		l ad _a t _a eta ↓	be l <u>e</u>
Agency	lation								Duratio	on l	h	0 250)		Ļ	
Analyst				Analys	is Dat	e 4/12/2	023		Area T	vne	ر بر	CBD		 		₹. 4
				Time F	Period	0 1/ 12/2	020		PHE	Jpo	,	0.82		→ ∻ - ≯	w∔e	<u>}-</u> ◆
Urban Street		Fast			is Yea	r 2023			Analys	sis F	Period	1> 8.	00	-4		+ ₹
Intersection		East & Market		File Na		2 Jane	- Fast '	2024			chidu	12 0.	00			
Project Descrip	tion	2024 AM						2024		us				-	- \ ব ↑ ক \7	ተተ
Troject Descrip		2024711														
Demand Inform	nation				EB			N	/B			NB			SB	
Approach Move	ement			L	Т	R	L	-	T F	२	L	Т	R	L	Т	R
Demand (v), v	eh/h			59	0						80	861			0	
Signal Informa	tion	v-		-		7										_
Cycle, s	70.0	Reference Phase	2		l Rt	, ⊨≍							1	N	2	-4 [
Offset, s	29	Reference Point	Begin	Green	48.5	10.6	0.0	0.0	0 0.	0	0.0				5	~*
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.	0 0.	0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.3	2.2	0.0	0.	0 0.	0	0.0		5	6	7	8
Timer Results				EBL	-	EBT	WB		WBT	_	NBL	-	NBT	SB	-	SBT
Assigned Phase	е					4							2			6
Case Number						12.0				_			8.0			8.0
Phase Duration	i, S					16.0				_			54.0			54.0
Change Period	, (Y+R)	c), S				5.4		\rightarrow		_			5.5			5.5
Max Allow Head	llow Headway (<i>MAH</i>), s					4.2		\rightarrow		_			0.0			0.0
Queue Clearan	ie Clearance Time (g_s), s					5.1				_						
Green Extensio	on Time	(ge),s				0.1				_			0.0			0.0
Phase Call Pro	bability					1.00				_						
Max Out Proba	bility					0.24										
Movement Gro	un Ros	aulte			FB			١٨/٢	3	1		NB			SB	
Approach Move	mont	Suits			Т	R	1	Т	, B	-	1	Т	R		т	R
Assigned Move	ment				4					+	5	2		<u> </u>	6	
Adjusted Flow F	Rate (v) veh/h		- ·	72					-	592	555		<u> </u>	0	
Adjusted Satura	ation Flo	w Rate (s) veh/h/	In		1633				-	+	1522	1443		<u> </u>	1825	
Queue Service	Time (a	γ_{s}) s			3 1						0.0	12.7			0.0	
Cvcle Queue C	learance	e Time (a c), s			3.1					+	4.2	12.7			0.0	
Green Ratio (g	/C)	, s			0.15					T	0.69	0.69			0.69	
Capacity (c), y	/eh/h				247						1115	999			1264	
Volume-to-Cap	acitv Ra	tio(X)			0.291					T	0.531	0.556			0.000	
Back of Queue	(Q), ft	/In (95 th percentile	e)		65.1					1	48.8	51.3			0	
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		2.5					T	2.0	2.0			0.0	
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00						0.00	0.00			0.00	
Uniform Delay	(d1), s/	/veh			31.2						1.0	1.0			0.0	
Incremental De	lay (d 2), s/veh			2.9						1.8	2.2			0.0	
Initial Queue De	remental Delay ($d z$), s/ven ial Queue Delay ($d z$), s/veh				0.0						0.0	0.0			0.0	
Control Delay (control Delay (<i>d</i>), s/veh				34.1						2.8	3.3			0.0	
Level of Service	evel of Service (LOS)				С						А	А				
Approach Dela	Approach Delay, s/veh / LOS					С	0.0		E.		3.0		A	0.0	<u> </u>	
Intersection De	Intersection Delay, s/veh / LOS					4	.9							A		
Multimodal Re	Itimodal Results				EB			W	3			NB			SB	
Pedestrian LOS	S Score	/LOS		1.94		В	1.95	5	В		1.33	;	Α	1.33	3	A
Bicycle LOS So	lestrian LOS Score / LOS γcle LOS Score / LOS					В					2.51		С	1.58	5	В

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General Inform	nation								Inte	ersect	ion Infe	ormatio	on		∫ ≈l °Y*h⊅ ↑	te la
Agency	lation								Dur	ration	h	0 250)		Ļ	
Analyst				Analys	is Dat	te 4/12/2	2023		Δre	a Tvn	<u></u>	CBD	·	 		۲. 4
Jurisdiction				Time	Poriod		.023			Г		0.82		→ ->>>>>>>>	w‡e	}- 4
Lirban Street		Fast		Analys		ar 2023			And	alveie	Doriod	1 2 8.	00			+ *
Intersection		East & Monroe				2 Jane	- East (2024		S VUS	renou	12 0.	00			<u> </u>
Project Descrip	tion				ame			2024		0.xu5				-	শ্] মিকাকাপ	12 (
T Toject Descrip	uon	2024 AM														
Demand Inform	nation				EB	;		V	/B			NB			SB	
Approach Move	ement			L	Т	R	L		Т	R	L	Т	R	L	Т	R
Demand (v), v	eh/h			3	0						28	924			0	
				1			1									
Signal Informa	tion				 	2								-+		
Cycle, s	80.0	Reference Phase	2		<u>_ 5</u>	F ₽							1	2	3	→ 4
Offset, s	60	Reference Point	Begin	Green	58.8	10.8	0.0	0.	0	0.0	0.0					
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.	0	0.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.0	2.0	0.0	0.	0	0.0	0.0		5	6	7	8
			_					_				1			. 1	
Timer Results				EBL	-	EBI	WBI		VV	ві	NBL	-	NBI	SB	L	SBI
Assigned Phase	e				_	4	<u> </u>	_		_			2	<u> </u>		6
Case Number						12.0	<u> </u>	\rightarrow					8.0	<u> </u>		8.0
Phase Duration	i, s	`		<u> </u>	_	16.0	<u> </u>	\rightarrow		_			64.0	<u> </u>		64.0
Change Period	Allow Headway (<i>MAH</i>), s				_	5.2	<u> </u>	\rightarrow		_		_	5.2			5.2
Max Allow Head	Allow Headway (<i>MAH</i>), s ue Clearance Time (<i>g</i> s), s			<u> </u>	+	4.2	<u> </u>	\rightarrow		_		_	0.0	<u> </u>	_	0.0
Queue Clearan	eue Clearance Time (g_s), s			<u> </u>	+	2.2	<u> </u>	-+		_			0.0	<u> </u>		0.0
Green Extensio	h n lime	(ge), s			+	0.0		\rightarrow		_			0.0	<u> </u>		0.0
Phase Call Pro					\rightarrow	1.00	<u> </u>	-		_				<u> </u>		
Max Out Proba	DIIILY					0.00										
Movement Gro	oup Res	ults			EB	_		W	В			NB			SB	
Approach Move	ement			L	Т	R	L	Т		R	L	Т	R	L	Т	R
Assigned Move	ment			7	4						5	2			6	
Adjusted Flow I	Rate (v), veh/h			4						558	511			0	
Adjusted Satura	ation Flo	w Rate (<i>s</i>), veh/h/l	In		1657	7					1759	1616			1599	
Queue Service	Time (g	g s), S			0.2						0.0	10.4			0.0	
Cycle Queue C	learanc	e Time (<i>g c</i>), s			0.2						2.5	10.4			0.0	
Green Ratio (g	/C)				0.13						0.74	0.74			0.74	
Capacity (c), v	/eh/h				224						1340	1188			1175	
Volume-to-Capa	acity Ra	tio(X)			0.016	3					0.416	0.431			0.000	
Back of Queue	(Q), f	/In (95 th percentile	e)		3.6						31.3	31.2			0	
Back of Queue	(Q), ve	eh/In (95 th percenti	ile)		0.1						1.3	1.2			0.0	
Queue Storage	Ratio (RQ) (95 th percent	tile)		0.00						0.00	0.00			0.00	
Uniform Delay	(d 1), s	/veh			34.7						0.7	0.7			0.0	
Incremental De	nental Delay (<i>d</i> ²), s/veh				0.1						0.8	0.9			0.0	
Initial Queue De	tial Queue Delay (d 3), s/veh				0.0						0.0	0.0			0.0	
Control Delay (Control Delay (<i>d</i>), s/veh				34.8						1.5	1.6			0.0	
Level of Service	el of Service (LOS)				C						Α	A				
Approach Delay	Approach Delay, s/veh / LOS			34.8	3	С	0.0				1.6		А	0.0)	
Intersection De	tersection Delay, s/veh / LOS					1	.7							A		
Maria I I -	nodal Resulte							1.4.1				ND			0.5	
Nultimodal Re	odal Results			4.05	EB	P	4.00	VVI	5		4.00	NB	Δ	4.04	SB	^
Pedestrian LOS	o ocore	1 LUS		1.95)	D	1.96	,	E		1.32	-	A	1.3	3 F	A
BICYCIE LOS SC	ore / LC	15		1.56		в					2.52	-	U	1.5	0	В

General Informa	ation								Intersec	tion Info	ormatio	on	2	4244	s l <u>a</u>
Agency									Duration	, h	0.250)		. ↓	
Analyst				Analys	is Date	e 4/12/2	023		Area Tvr)e	CBD		 		₹. \$
Jurisdiction				Time P	eriod				PHF		0.92		→ ∻ ≯	W TE	↓ ↓
Urban Street		East		Analvs	is Year	2023		_	Analysis	Period	1> 8:0	00			++ *
Intersection		East & Olive		File Na	me	2 lane	- East 2	2024	AM S.xus					st ta	<u>_</u>
Project Description	on	2024 AM		1				-					1	▲↑↓∀ 1	* (*
, ,															
Demand Informa	ation				EB			W	/B		NB			SB	
Approach Moven	nent			L	Т	R	L	٦	Г R	L	Т	R	L	T	R
Demand (v), ve	h/h			53	47			1	5 23	28	928	10		0	
				l)			ы	1							
Signal Informati	ion	:	-	-	+	. 3 8	=						-		
Cycle, s	80.0	Reference Phase	2		51	≈ —						1		3	→ 4
Offset, s	51	Reference Point	Begin	Green	55.8	13.1	0.0	0.0	0.0	0.0					5
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0	0.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.4	2.3	0.0	0.0) 0.0	0.0		5	6	7	8
T ' D K			_	EDI		EDT				NIDI		NDT	0.01	_	ODT
Assisted Dhose				EBL	·	EBI	VVB		WBI	NBL	· -	NBI	SBL	-	SBI
Assigned Phase						4		-	8			2		_	0
Case Number					_	8.U		-	8.0	<u> </u>	_	8.0			8.0
Change Duration,	S (V+D .			<u> </u>		5.5		+	5.5	<u> </u>	-	5.6			5.6
Max Allow Heady	eadway (MAH), s					4.2		+	4.2	-	-	0.0			0.0
Queue Clearance	x Allow Headway (<i>MAH</i>), s eue Clearance Time (<i>g</i> s), s					7.2			3.1						
Green Extension	Time ((ge).s	_			0.2			0.3			0.0			0.0
Phase Call Proba	ability					0.96		-	1.00						
Max Out Probabi	ility					0.25			0.01						
	, ,														
Movement Grou	ıp Res	ults			EB			WE	3		NB			SB	
Approach Moven	nent			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Assigned Movem	nent			7	4			8	18	5	2	12		6	
Adjusted Flow Ra	ate (v), veh/h			109			16	25	549		501		0	
Adjusted Saturat	ion Flo	w Rate (<i>s</i>), veh/h/l	In		1448			178	4 1607	1544		1413		1825	
Queue Service T	īme (g	1 s), S			3.5			0.6	1.1	0.0		13.2		0.0	
Cycle Queue Cle	earance	e Time (<i>g c</i>), s			5.2			0.6	i 1.1	13.1		13.2		0.0	
Green Ratio (g/0	C)				0.16			0.16	6 0.16	0.70		0.70		0.70	
Capacity (c), ve	eh/h				306			292	2 263	1125		985		1273	
Volume-to-Capac	city Ra	tio (X)			0.355			0.05	6 0.095	0.488		0.508		0.000	
Back of Queue (Q), ft	/In (95 th percentile	e)		88.4			13.	5 20.6	165.6		154.7		0	
Back of Queue (Q), ve	h/ln (95 th percent	ile)		3.4			0.5	0.8	6.6		6.2		0.0	
Queue Storage F	Ratio (RQ) (95 th percent	tile)		0.00			0.00	0.00	0.00		0.00		0.00	
Uniform Delay (a	d 1), s/	veh			30.1			28.2	2 28.4	5.6		5.6		0.0	
Incremental Dela	emental Delay (<i>d</i> ₂), s/veh				0.7			0.4	0.7	1.5		1.9		0.0	
Initial Queue Del	itial Queue Delay (d ₃), s/veh				0.0			0.0	0.0	0.0		0.0		0.0	
Control Delay (d	Control Delay (d), s/veh				30.7			28.6	o 29.1	7.1		7.5		0.0	
Level of Service	(LOS)	(1.00			C		00.0		C	A		A			
Approach Delay,	Approach Delay, s/ven / LOS					C	28.9	1	C	7.3		A	0.0		
Intersection Dela	ay, s/ve	n / LUS				10	0.2						Б		
Multimodal Res	ultimodal Results				EB			WE	3		NB			SB	
Pedestrian LOS	timodal Results					В	1.94	L I	В	1.85		В	1.86	;	В
Bicycle LOS Sco	trian LOS Score / LOS 9 LOS Score / LOS					В	1.59)	В	2.43		В	1.55	,	В

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HCS[™] Streets Version 2023

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General Inform	nation								Inter	sect	ion Info	ormatic	on	2	***	× l <u>x</u>
Agency									Dura	tion.	h	0.250			Ļ	
Analyst				Analys	is Dat	e 4/12/2	2023		Area		3	CBD		 		₹
Jurisdiction				Time F	Period				PHF		-	0.80		→^ *	W A E	 ↓ ↓ ↓
Urban Street		Fast		Analys	is Yea	r 2023			Analy	vsis F	Period	1> 8:(00	→		+ *
Intersection		East & Washington		File Na	me	2 lane	- Fast	2024	AMS	xus					5 4 4 7	<u> </u>
Project Descrip	tion	2024 AM												1	ব † ক 🕎 1	* (*
, , , , , , , , , , , , , , , , , , ,																
Demand Inform	nation				EB			Ν	/B			NB			SB	
Approach Move	ement			L	Т	R	L		г	R	L	Т	R	L	Т	R
Demand (v), v	eh/h			10	171			23	36	77	23	899	53		0	
							ш	_	_		-					
		Deference Dhase	2		+	1.2 ×	1							stz.		~
Cycle, s	60.0	Reference Priase	Z		<u>≞</u> †								1	2	3	4
Unset, s	04		Беуіп	Green	50.1	18.5	0.0	0.	0 (0.0	0.0					5
	INO Fixed	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0		0.0	0.0	-			_	•
Force Mode	Fixed	Simult. Gap N/S	On	Rea	2.8	Z.Z	0.0	0.	0 [[0.0	0.0		5	6	1	8
Timor Results			_	EBI		FBT	W/B	1	W/B	т	NRI		NBT	SBI		SBT
Assigned Phase	۵				-	4	VUD		8	<u> </u>			2			6
Case Number	<u> </u>			<u> </u>		6.0		-	7.0				5.0			8.0
Phase Duration	S				-	23.9			23.9	9			56.1			56.1
Change Period	.(Y+R	c). S	_		+	5.4		-	5.4				6.0		+	6.0
Max Allow Head	Allow Headway (<i>MAH</i>), s					4.2			4.2				0.0			0.0
Queue Clearan	ax Allow Headway (<i>MAH</i>), s ieue Clearance Time (<i>g</i> s), s					16.6			15.6	6						
Green Extensio	n Time	(ge),s				1.4			1.6	;			0.0			0.0
Phase Call Pro	bability					1.00			1.00	D						
Max Out Proba	bility					0.40			0.29	9						
	_								_			NIE			0.0	
Movement Gro	oup Res	sults			EB				3			NB			SB	
Approach Move	ement				1	R	L		F	R		1	R	L	I	R
Assigned Move	ment)		/	4			8	- 0		00	Z	12		0	
Adjusted Flow I	Rate (V), ven/n		13	214		<u> </u>	29:	5 9 5 40	10 77	20	1001	59		0	
			in	959	0.6			103		6	0.5	10/0	1007		1070	
Queue Service		f(s), S		14.6	0.0		<u> </u>	13.	0 4. 6 1	.0	0.5	12.0	1.2		0.0	
Cycle Queue C		e fille (<i>g c</i>), s		0.23	0.0			0.2	0 4. 3 0 4	23	0.5	12.0	0.63	_	0.0	
Green Kato (g	/0) /eh/h			1/18	407			378	$\frac{3}{3}$ $\frac{3}{3}$	18	1070	2103	0.00		1052	
Volume-to-Can	acity Ra	tio (X)		0.084	0 525			0.78	3 0 3	303	0.024	0 476	0.061		0.000	
Back of Queue	(Q), ft	/In (195 th percentile	e)	11.1	167.8			253	.4 70	0.5	7	181	16		0.000	
Back of Queue	(Q), ve	eh/In (95 th percent	ile)	0.4	6.5	<u> </u>		9.8	3 2	.7	0.3	7.0	0.6		0.0	
Queue Storage	Ratio (RQ) (95 th percen	tile)	0.17	0.00	<u> </u>		0.0	0 0.	.88	0.09	0.00	0.00		0.00	
Uniform Delay	(d1), s/	/veh		36.2	27.3			29.	3 25	5.4	5.6	7.9	5.7		0.0	
Incremental De	lay (d 2), s/veh		0.2	0.9			6.3	3 0.	.5	0.0	0.7	0.1		0.0	
Initial Queue De	cremental Delay (d ₂), s/ven itial Queue Delay (d ҙ), s/veh			0.0	0.0	1		0.0) 0.	0.0	0.0	0.0	0.0		0.0	
Control Delay (ontrol Delay (d), s/veh			36.4	28.3			35.	6 26	6.0	5.7	8.5	5.8		0.0	
_evel of Service (LOS)				D	С			D	0	С	А	А	Α			
Approach Delay	Approach Delay, s/veh / LOS					С	33.2	2	С		8.3		А	0.0		
Intersection De	lay, s/ve	h / LOS				1	6.7							В		
	ultimodel Reculto															
Multimodal Re	ultimodal Results				EB			WE	3			NB			SB	
Pedestrian LOS	Score	/LOS		2.28		В	1.93	3	B		1.87		В	1.87		В
Bicycle LOS Sc	edestrian LOS Score / LOS cycle LOS Score / LOS					В	2.2	1	В		2.57		С	1.55		В

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HCS[™] Streets Version 2023

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General Information								Interse	ction I	nforr	natio	n	J.	╡Ҳ╬	₽ L
Agency								Duratio	n. h	0	.250			4	
Analyst			Analys	is Date	- 4/12/2	023		Area Ty	/ne	- C	CBD		 		た 本
			Time F	Period		.020		PHE	P 0	0) 75		→ \$	w‡e	↓ ↓
Urban Street	Madison		Analys	is Yea	r 2023		_	Analysi	s Perio	d 1).7 0 > 8∙0	0			부 ¥
Intersection	Madison & Front		File Na	me	2 Jane	- Madie	son 2()24 AM	S YUS		0.0				<u> </u>
Project Description	2024 AM						5011 20		5.743				5	1 1	te d
Troject Beschption	20247110														
Demand Information				EB			W	В			NB			SB	
Approach Movement			L	Т	R	L	Т	R	1	-	Т	R	L	Т	R
Demand (v), veh/h				60	15	21	3	1			0		58	704	33
			lu-	- II.				j.							
Signal Information		-		<mark>∠↓</mark>	÷										
Cycle, s 80.0	Reference Phase	2		ľ ↑	R							1	2	3	
Offset, s 4	Reference Point	Begin	Green	58.6	10.2	0.0	0.0) 0.0) 0	.0					
Uncoordinated No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0) 0.0) 0	.0			1		
Force Mode Fixed	Simult. Gap N/S	On	Red	2.5	2.3	0.0	0.0) 0.0	0 0	.0		5	6	7	8
		_		_							_				
Timer Results			EBL	-	EBT	WB	L	WBT		IBL		NBT	SBL		SBT
Assigned Phase					4		\rightarrow	8	-		-	6			2
Case Number					8.0		\rightarrow	6.0	-		-	8.0			8.0
Phase Duration, s					15.7		\rightarrow	15.7	_		6	54.3			64.3
Change Period, (Y+R	c), S				5.5		\rightarrow	5.5	-		+	5.7			5.7
Max Allow Headway (A	v Headway (<i>MAH</i>), s earance Time (<i>g</i> _s), s		<u> </u>		4.2		\rightarrow	4.2				0.0	<u> </u>		0.0
Queue Clearance Time	Clearance Time (g_s) , s			_	6.8	<u> </u>	\rightarrow	8.7	-		+	0.0			0.0
Green Extension Time	(ge), s				0.5		\rightarrow	0.5	-		+	0.0			0.0
Phase Call Probability				_	0.98	<u> </u>	\rightarrow	0.98	-		+			_	
Max Out Probability					0.00			0.00							
Movement Group Res	ults			EB			WE	}			NB			SB	
Approach Movement			L	Т	R	L	Т	R	L		Т	R	L	Т	R
Assigned Movement				4	14	3	8				6		5	2	12
Adjusted Flow Rate (v), veh/h			100		28	41				0		553		510
Adjusted Saturation Flo	ow Rate (<i>s</i>), veh/h/l	n		1531		1163	1749	9		1	705		1621		1503
Queue Service Time (g	g s), S			4.8		1.8	1.7			(0.0		0.0		11.4
Cycle Queue Clearance	e Time (<i>g c</i>), s			4.8		6.7	1.7			(0.0		10.9		11.4
Green Ratio (g/C)				0.13		0.13	0.13	3		0).73		0.73		0.73
Capacity (c), veh/h				196		168	224			1:	248		1238		1100
Volume-to-Capacity Ra	tio(X)			0.511		0.166	0.18	5		0.	.000		0.447		0.463
Back of Queue (Q), ft	/In (95 th percentile	e)		86.5		24.6	33.4	ł			0		140.1		130.7
Back of Queue (Q), ve	eh/In (95 th percenti	ile)		3.3		0.9	1.3		_	(0.0		5.5		5.2
Queue Storage Ratio (RQ) (95 th percent	tile)		0.00		0.41	0.00)	_	0	0.00		0.00		0.00
Uniform Delay (d 1), s/	/veh			32.1		35.2	30.7	7	-	(0.0		4.5		4.6
Incremental Delay (d 2	elay (<i>d</i> ₂), s/veh			2.1		0.5	0.4			(0.0		1.0		1.2
Initial Queue Delay (d	tial Queue Delay (d 3), s/veh			0.0		0.0	0.0		-	(0.0		0.0		0.0
Control Delay (d), s/ve	ntrol Delay (d), s/veh			34.1		35.6	31.1			(0.0		5.5		5.8
Level of Service (LOS)	ce (LOS)			C		D	C						A		A
Approach Delay, s/veh	proach Delay, s/veh / LOS				C	32.9	1	C		0.0			5.7		A
Intersection Delay, s/ve	n Delay, s/veh / LOS				9	.5							A		
Multimodal Results	ts			FR			\//E	3			NB			SB	
Pedestrian LOS Score	Its				В	1 93	3	B	1	84		В	1 61		В
Bicycle LOS Score / LC)S		1.72		B	1.67	7	 B	1	.56		B	2.43		B

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General Inform	nation								Intersed	tion In	formati	on	4	4 <u>1</u> 44 ↓	la la
Agency									Duration	. h	0.25	0		4	
Analvst				Analys	is Dat	e 4/12/	2023		Area Tv	,)e	CBD	-	 		بر ج
Jurisdiction				Time F	Period				PHF		0.76		⇒ ∲_ _	w ∔ e	<u>≁</u> ‡
Urban Street		Madison		Analys	is Yea	r 2023			Analysis	Period	1> 8	:00			 २ २
Intersection		Madison & Jefferso	n	File Na	ame	2 Ian	e - Madi	son 20	, 24 AM S	.xus				+	
Project Descrip	tion	2024 AM											1	414Y	11
, , ,		-													
Demand Inform	nation				EB			W	В		NE	;		SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				15	4	4	12	2		0		12	817	11
				1											
Signal Informa	tion				24 ¹ 2		_								
Cycle, s	80.0	Reference Phase	2		ľ 1	• 🖹 '						1	2	3	
Offset, s	10	Reference Point	Begin	Green	53.6	15.9	0.0	0.0	0.0	0.0					
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0	0.0	0.0			1		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.2	1.9	0.0	0.0	0.0	0.0	_	5	6	7	8
				501	_				MOT			NET	0.01		0.0.7
Timer Results				EBL	-	EBI	WB	iL	WBI	NE	SL	NBI	SBL		SBI
Assigned Phase	e				_	4	<u> </u>		8	<u> </u>		6	<u> </u>	+	2
Case Number					-	8.0			8.0	<u> </u>		8.0	<u> </u>	+	8.0
Phase Duration	I, S	\ -		<u> </u>	+	21.0	<u> </u>		21.0	<u> </u>	-+	59.0	<u> </u>	+	59.0
Change Period	eriod, (Y+R c), s Headway (<i>MAH</i>), s				-	5.4			5.4	<u> </u>		5.4		+	5.4
	ullow Headway (<i>MAH</i>), s e Clearance Time (<i>g</i> s), s				+	4.1	<u> </u>	-	4.1			0.0	<u> </u>	+	0.0
Green Extensio	e Clearance Time (g_s) , s					0.1	-		0.1			0.0			0.0
Phase Call Pro	hability	(ge), s			+	1.00	<u> </u>		0.64	<u> </u>	-+	0.0	<u> </u>	+	0.0
Max Out Proba	hility			<u> </u>	-	0.00			0.00				<u> </u>		
Max Out 1 105a	onity					0.00			0.00					a de la com	
Movement Gro	oup Res	ults			EB			WB			NB			SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Assigned Move	ment				4	14	3	8			6		5	2	12
Adjusted Flow I	Rate (<i>v</i>), veh/h			25			21			0		579		526
Adjusted Satura	ation Flo	ow Rate (<i>s</i>), veh/h/	In		1587			1748	3		1689		1643		1493
Queue Service	Time (g	g s), s			1.0			0.0			0.0		0.0		10.1
Cycle Queue C	learance	e Time (<i>g c</i>), s			1.0			0.7			0.0		10.1		10.1
Green Ratio (g	/C)				0.19			0.19			0.67		0.67		0.67
Capacity (c), v	/eh/h				309			397			1131		1147		1000
Volume-to-Capa	acity Ra	tio(X)			0.081			0.05	3		0.000	·	0.505		0.526
Back of Queue	(Q), ft	/In (95 th percentile	e)		19.5			14.9			0		120.1		111.3
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		0.8		<u> </u>	0.6			0.0	<u> </u>	4.7		4.5
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00		<u> </u>	0.00			0.00	<u> </u>	0.00		0.00
Uniform Delay ((d 1), s/	/veh			26.2		<u> </u>	26.1			0.0		3.8		3.8
Incremental De	emental Delay (<i>d</i> ₂), s/veh				0.5		<u> </u>	0.1	<u> </u>		0.0	<u> </u>	1.6		2.0
Initial Queue De	hitial Queue Delay (<i>d</i> ₃), s/veh				0.0			0.0			0.0		0.0		0.0
Control Delay (Control Delay (d), s/veh				26.7			26.1			0.0		5.4		5.8
Level of Service	el of Service (LOS)				C				<u> </u>				A		
Approach Delay, s/veh / LOS				26.7		С	26.	1	С	0.0)		5.6		A
Intersection De	ersection Delay, s/veh / LOS					6	j.4				_		A		
Multimodal Ba	al Results				EP						ND			C D	
Pedestrian I OS	Score	/1.05		1 02		B	1 0'	3	R	16	3	R	1 63		B
Bicycle I OS Sc	core / I C)S		1.00		B	1.5	9	B	1.0	6	B	2 47		B
_10,010 200 00		-		1.00		2	1.0	~		1.0	-	5	2.71		-

			3								,					
General Inform	ation								Interse	ction	Info	rmatio	n	2	**	x l _a
Agency									Duratio	n. h		0.250			7††	
Analyst				Analys	is Date	- 4/12/2	023		Area Ty	, /ne		CBD		 		たみ
Jurisdiction				Time P	Period	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	020		PHE	P 0		0.82		→ -+	w‡e	-√ - -
Urban Street		Madison		Analys	is Yea	r 2023			Analysi	s Peri	nd	1> 8·0	0	7		+ ⊊
Intersection		Madison & Locust		File Na	me	2 lane	- Madis	son 20)24 AM	Nixus		1 0.0			^	<u></u>
Project Descript	tion	2024 AM		1 110 110			maare		21740	T.N.GO				1	★↑┿₩1	× (*
i rojoot booonp		202 17 40														
Demand Inform	nation				EB			W	В			NB			SB	
Approach Move	ment			L	Т	R	L	Т	R		L	Т	R	L	Т	R
Demand (v), v	eh/h				415	52	4	46	6			0		148	942	134
				lu-					j.					· · · · · ·		
Signal Informa	tion		-		k tin											—
Cycle, s	70.0	Reference Phase	2		ľ ↑	B							1	2	3	4
Offset, s	47	Reference Point	Begin	Green	47.0	15.0	0.0	0.0) 0.0) (0.0					
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.0	3.0	0.0	0.0) 0.0) (0.0			1	_	→
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	0.0	0.0) 0.0) (0.0		5	6	7	Y 8
Timer Deculto			_			ГРТ	\//DI							<u>CDI</u>		ODT
Assigned Phase	<u></u>			EDL			VVDI			+	NDL		6	SDL		2
Assigned Phase	=			<u> </u>		0	<u> </u>	+	4	-		_	0	<u> </u>		2
Case Number	<u> </u>			<u> </u>		0.0		-	0.0	+			0.U	<u> </u>		7.0 51.0
Change Duration	, S (V+D)			<u> </u>		19.0		+	19.0	-		-	4.0	<u> </u>		4.0
Max Allow Hear	Allow Headway (<i>MAH</i>), s					4.0		-	4.0	+		-	4.0			4.0
	ax Allow Headway (<i>MAH</i>), s ieue Clearance Time (<i>g</i> s), s					4.0 13.6		+	13.7	-		-	0.0			0.0
Green Extensio	ueue Clearance Time (g_s), s					13.0		-	13	-		-	0.0			0.0
Phase Call Prot	nability	(90),0			-	1.00		-	1.00	+			0.0			0.0
Max Out Proba	bility					0.46			0.46				_			
	• .					0110			0110							
Movement Gro	oup Res	ults			EB			WB	}			NB			SB	
Approach Move	ment			L	Т	R	L	Т	R			Т	R	L	Т	R
Assigned Move	ment				8	18	7	4				6		5	2	12
Adjusted Flow F	Rate (v), veh/h			288	282		61			$ \downarrow$	0		683	646	155
Adjusted Satura	ation Flo	w Rate (<i>s</i>), veh/h/l	In		1657	1616		1664	4	-	\rightarrow	1710		1537	1508	1449
Queue Service	Time (g	g s), S			10.5	11.6		0.0		-	$ \rightarrow$	0.0		14.8	17.3	2.8
Cycle Queue C	learance	e Time (<i>g ₀</i>), s			10.5	11.6		11.7	′	-	\rightarrow	0.0		18.1	17.3	2.8
Green Ratio (g	/C)				0.21	0.21		0.21		-	\rightarrow	0.67		0.67	0.67	0.67
Capacity (c), v	eh/h				355	346		412		-	\rightarrow	1148		1097	1012	973
Volume-to-Capa	acity Ra	tio (X)	<u>, </u>		0.810	0.814		0.14	8	-	\rightarrow	0.000		0.623	0.638	0.159
Back of Queue	(Q), ft	/In (195 th percentile	e)		225.8	216.8		37.8	3	+	\rightarrow	0		220.3	219	31.9
Back of Queue	(Q), Ve	eh/In (95 th percent	ile)		8.8	8.7		1.5		+	\rightarrow	0.0		8.7	8.5	1.3
Queue Storage	Ratio (RQ) (95 th percent	tile)		0.00	0.00		0.00		+		0.00		0.00	0.00	0.00
Uniform Delay ((a1), S/				20.1	20.2		23.5	>	+	\rightarrow	0.0		0.7	0.0	4.3
Incremental De	ncremental Delay (d 2), s/veh				1.1	8.1		0.0		-	\rightarrow	0.0		2.7	3.1	0.3
nitial Queue Delay (d $_3$), s/veh					0.0	24.0		0.0	2	-		0.0		0.0	0.0	0.0
Lontrol Delay (d), s/veh					33.8 C	34.Z		23.6	,	-	-	0.0		9.4	9.7	4.0 A
	Approach Delay, s/veh / LOS						22.6			-					A	
Intersection Dol	ntersection Delay, s/veh / LOS					16	23.0	,	U	-	0.0			9.0 B		~
	tersection Delay, siven / LOS													J		
Multimodal Re	lultimodal Results				EB			WB	3			NB			SB	
Pedestrian LOS	Score	/LOS		1.92		В	2.12	2	В	1	.85		В	1.85		В
Bicycle LOS Sc	ore / LC	S		2.03		В	0.59)	А	1	.56		В	2.78		С

		5													
General Inform	nation				_				Intersec	tion Inf	ormatio	on	لير	4 사 수 수	₽ L
Agency									Duration	. h	0.250			4	
Analyst				Analys	is Dat	e 4/12/2	023		Area Tvr	, e	CBD		 		<u>≮_</u> &
Jurisdiction				Time F	Period	,, _			PHE	-	0.74		→ \$ - \	w ‡ e	↓ ↓
Urban Street		Madison		Analys	is Yea	r 2023			Analysis	Period	1> 8:0	00	→		* ← *
Intersection		Madison & Market		File Na	me	2 lane	- Madis	son 20	24 AM N	XUS	1 0			•	<u>r</u>
Project Descrip	tion	2024 AM					maan	2011/20					ň	4 1 4 Y	۲ (⁴
r rojoot b ooonp		2021744													
Demand Inform	nation				EB			W	В		NB			SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				65	81	10	75	5		0		6	753	84
Signal Informa	tion		1	-	₽ ₩3		-								
Cycle, s	70.0	Reference Phase	2		ľ 🕇	• 🗟 "	···]						2	3	
Offset, s	64	Reference Point	Begin	Green	47.1	12.3	0.0	0.0	0.0	0.0					_
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0	0.0	0.0			1		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.0	2.2	0.0	0.0	0.0	0.0	_	5	6	7	8
			_												
Timer Results				EBL	-	EBT	WB		WBT	NB	-	NBT	SBL		SBT
Assigned Phase	9				+	4		_	8		_	6		_	2
Case Number						8.0			6.0			8.0		_	8.0
Phase Duration	, S	```			\rightarrow	17.7		_	17.7			52.3			52.3
Change Period,	(Y+R)	c), S		_	5.4		_	5.4		_	5.2		_	5.2	
Max Allow Head	dway(A	vay(<i>MAH</i>), s Time(g s), s			_	4.3	<u> </u>	_	4.3		_	0.0	<u> </u>	\rightarrow	0.0
Queue Clearan	Clearance Time (g_s) , s				_	10.9		_	11.7			0.0		+	0.0
Green Extensio	n Time	(ge), s			+	0.6		+	0.6			0.0		+	0.0
Phase Call Pro					+	1.00	<u> </u>	+	1.00	<u> </u>				+	
Max Out Proba	bility					0.28			0.43						
Movement Gro	oup Res	ults			EB			WB	,		NB			SB	
Approach Move	ment			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Assigned Move	ment				4	14	3	8			6		5	2	12
Adjusted Flow F	Rate (v), veh/h			197		14	101	<u> </u>		0		648		569
Adjusted Satura	ation Flo	w Rate (s), veh/h/	In		1485		1049	1727	7		1922		1570		1378
Queue Service	Time (g	g s), S			8.9	1	0.9	3.6			0.0		0.0		15.9
Cycle Queue C	learance	e Time (g c), s			8.9		9.7	3.6			0.0		15.8		15.9
Green Ratio (g	/C)				0.18		0.18	0.18	;		0.67		0.67		0.67
Capacity (c), v	eh/h				261		154	303			1294		1109		927
Volume-to-Capa	acity Ra	tio(X)			0.757	·	0.088	0.334	4		0.000		0.584		0.614
Back of Queue	(Q), ft	/In (95 th percentile	e)		158.1		10.4	68.2	2		0		182.2		166.5
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		6.1		0.4	2.6			0.0		7.2		6.7
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00		0.00	0.00			0.00		0.00		0.00
Uniform Delay ((d 1), s/	/veh			27.6		32.3	25.5	;		0.0		6.2		6.2
Incremental De	remental Delay (<i>d</i> ²), s/veh				5.3		0.2	0.5			0.0		1.7		2.3
Initial Queue De	itial Queue Delay (d ₃), s/veh				0.0		0.0	0.0			0.0		0.0		0.0
Control Delay (control Delay (<i>d</i>), s/veh				33.0		32.5	26.0			0.0		7.9		8.6
Level of Service	vel of Service (LOS)				С		С	С					A		A
Approach Delay	Approach Delay, s/veh / LOS					С	26.7	7	С	0.0			8.2		А
ntersection Delay, s/veh / LOS						12	2.8						В		
Multimodal Re	sults			EB			WB			NB			SB		
Pedestrian LOS	Score	/LOS		1.92		B	1.92	2	В	1.8	5	В	1.63	\perp	В
Bicycle LOS Sc	ore / LC	DS		1.89		В	1.75	5	В	1.56	6	В	2.50		В

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HCS[™] Streets Version 2023

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General Inform	nation								Inte	ersect	ion Inf	ormatio	on	4	ad ada ↓ ↓	≜ L <u>a</u>
Agency									Dur	ration.	h	0.250			7††	
Analyst				Analys	is Dat	<u>a</u> 4/12/2	023		Area	a Type	<u>،</u>	CBD		 		r. 4
Jurisdiction				Time F	Period	5 17 12/2	020		PHE	F	,	0.92		→ ∻ →	w‡e	← <mark>⊱</mark>
Urban Street		Madison		Analys	is Yea	r 2023			Ana	alvsis I	Period	1> 8.0	0			+ ↓
Intersection		Madison & Olive		File Na	me	2 Jane	- Madie	son 21	024 4			1. 0.		- <u>-</u>		<u> </u>
Project Descrip	tion	2024 AM		1 110 110			maan	5011 2.	0217	// 0./				1	* 1 * * * 1	- r
	aon	2021744														
Demand Inform	nation				EB			W	/B			NB			SB	
Approach Move	ement			L	Т	R	L	٦	Г	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				55	33	3	3	3			0		39	657	52
				11	- 11:		-	1			-					
Signal Informa	ition			-	11	5										
Cycle, s	80.0	Reference Phase	2		ľ ↑	″							1	2	3	
Offset, s	21	Reference Point	Begin	Green	52.1	16.5	0.0	0.0	0	0.0	0.0					_
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0)	0.0	0.0			1		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.7	2.3	0.0	0.0)	0.0	0.0		5	6	7	8
					_	EDT	14/5			D.T.			NET	0.01		0.D.T.
Timer Results				EBL	-	EBI	WB		W	BI	NBI	-	NBI	SBL	-	SBI
Assigned Phase	e			<u> </u>	\rightarrow	4		\rightarrow	8	5			6	<u> </u>	_	2
Case Number				<u> </u>	_	7.0	<u> </u>	_	6.	.0		_	8.0	<u> </u>		7.0
Phase Duration	i, S	`		<u> </u>		22.0	<u> </u>	_	-22	2.0		_	58.0	<u> </u>		58.0
Change Period	, (Y+R a	c), S		-	5.5		\rightarrow	5.	.5			5.9			5.9	
Max Allow Head	away (A	ИАН), S		<u> </u>	+	4.2		-	4.	.2			0.0	<u> </u>		0.0
Queue Clearan					4.3	<u> </u>	-	4.	.5			0.0		_	0.0	
Green Extensio	n Time	(ge), s		<u> </u>	\rightarrow	0.3		\rightarrow	0.	.3			0.0			0.0
Max Out Broke						0.00	<u> </u>		0.8	95		_			_	
Max Out Proba	DIIILY					0.00			0.0	00						
Movement Gro	oup Res	ults			EB			WE	3			NB			SB	
Approach Move	ement			L	Т	R	L	Т		R	L	Т	R	L	Т	R
Assigned Move	ment				4	14	3	8				6		5	2	12
Adjusted Flow I	Rate (v), veh/h			60	36	3	36				0		478	443	69
Adjusted Satura	ation Flo	w Rate (<i>s</i>), veh/h/l	In		1678	1422	1209	178	9			1662		1678	1564	1456
Queue Service	Time (g	y s), S			2.3	1.6	0.2	1.3	;			0.0		0.0	11.1	1.4
Cycle Queue C	learance	e Time (<i>g c</i>), s			2.3	1.6	2.5	1.3	;			0.0		10.8	11.1	1.4
Green Ratio (g	/C)				0.21	0.21	0.21	0.2	1			0.65		0.65	0.65	0.65
Capacity (c), v	/eh/h				346	293	304	369)			1083		1143	1018	948
Volume-to-Cap	acity Ra	tio(X)			0.173	0.122	0.011	0.09)7			0.000		0.418	0.435	0.073
Back of Queue	(Q), ft	/In (95 th percentile	e)		46.3	27.8	2.3	25.1	1			0		163.3	154	17.7
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		1.8	1.1	0.1	1.0)			0.0		6.4	6.1	0.7
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00	0.35	0.00	0.00	0			0.00		0.00	0.00	0.35
Uniform Delay	(d 1), s/	/veh			26.0	25.7	27.0	25.6	6			0.0		6.8	6.8	5.1
Incremental De	remental Delay (<i>d</i> ₂), s/veh				1.1	0.9	0.0	0.1				0.0		1.0	1.2	0.1
Initial Queue De			0.0	0.0	0.0	0.0)			0.0		0.0	0.0	0.0		
Control Delay (d), s/veh					27.1	26.6	27.0	25.	7			0.0		7.8	8.0	5.3
Level of Service (LOS)					С	С	С	С						Α	Α	А
Approach Delay, s/veh / LOS				26.9		С	25.8	3	С	C	0.0			7.7		А
Intersection Delay, s/veh / LOS						1(0.0							A		
Multimodal Re	timodal Results				EB			WE	3			NB	_		SB	
Pedestrian LOS	Score	/LOS	1.93		В	2.12	2	B	3	1.86	5	В	1.86		В	
Bicycle LOS Sc	ore / LC	DS		1.72		В	1.62	2	B	3	1.56	6	В	2.23		В

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General Inform	nation								Int	tersect	ion Inf	ormatio	on	لير	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	≜ L <u>a</u>
Agency									Du	iration.	h	0.250)		7 † † ľ	
Analyst				Analys	is Dat	e 4/12/2	023		Are	ea Type	<u>،</u>	CBD		 		r. 4
Jurisdiction				Time F	Period	0 1/ 12/2			PH	HF		0.78		→ \$ - 	w‡e	↓ ↓ ↓
Urban Street		Madison		Analys	is Yea	r 2023			An	" alvsis I	Period	1> 8.	0	- 1		+ + +
Intersection		Madison & Washing	notr	File Na	me	2 Jane	- Madie	son 2	024			1. 0.		- <u>-</u>		<u> </u>
Project Descrip	tion	2024 AM	gion	1 110 110			maan		021	7 101 0.2	100			1	* 1 4 * * 1	- (*
r rojoor Beeerip	lion	2021710														
Demand Inform	nation				EB			V	٧B			NB			SB	
Approach Move	ement			L	Т	R	L	-	Т	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				97	9	34	18	88			0		37	765	22
				i	- 11			_			-					
Signal Informa	ition		-		₽ ₽₽₽	6										
Cycle, s	80.0	Reference Phase	2		ľ ↑	Ř							1	2	3	
Offset, s	79	Reference Point	Begin	Green	55.6	13.4	0.0	0.	0	0.0	0.0					_
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.	0	0.0	0.0			1		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.4	2.2	0.0	0.	0	0.0	0.0		5	6	7	8
					_	EDT	14/5			(D.T.			NET	0.01		0.D.T.
Timer Results				EBL	-	EBI	WB		N	VBI	NBI	-	NBI	SBL	-	SBI
Assigned Phase	e			<u> </u>	\rightarrow	4		\rightarrow	-	8			6	<u> </u>	_	2
Case Number				<u> </u>	_	8.0	<u> </u>	-	6	5.0		_	8.0	<u> </u>		5.0
Phase Duration	i, S	`		<u> </u>		18.8	<u> </u>	-	18	8.8			61.2	<u> </u>		51.2
Change Period	, (Y+R a	c), S		+	5.4	<u> </u>	\rightarrow	5	o.4			5.6		_	5.6	
Max Allow Head	dway(/	ИАН), S		\rightarrow	4.2		\rightarrow	4	1.2		_	0.0			0.0	
Queue Clearan		(gs), s			8.0	<u> </u>	\rightarrow	1	2.1			0.0		_	0.0	
Green Extensio	n lime	(ge), s			+	1.5		-	1	00			0.0			0.0
Phase Call Pro						1.00		-	1.	.00					_	
Max Out Proba	biiity					0.00			0.	.01						
Movement Gro	oup Res	ults			EB			W	В		_	NB			SB	
Approach Move	ement			L	Т	R	L	Т	Т	R	L	Т	R	L	Т	R
Assigned Move	ment				4	14	3	8				6		5	2	12
Adjusted Flow I	Rate (v), veh/h			136		44	24	1			0		49	1008	29
Adjusted Satura	ation Flo	w Rate (<i>s</i>), veh/h/l	In		1648	1	1110	183	33			1689		1578	1644	1509
Queue Service	Time (g	y s), S			6.0		3.0	10.	1			0.0		0.8	10.9	0.5
Cycle Queue C	learance	e Time (<i>g c</i>), s			6.0		8.9	10.	1			0.0		0.8	10.9	0.5
Green Ratio (g	/C)				0.17		0.17	0.1	7			0.69		0.69	0.69	0.69
Capacity (c), v	/eh/h				277		194	308	8			1173		1186	2284	1048
Volume-to-Cap	acity Ra	tio(X)			0.491		0.225	0.78	33			0.000		0.041	0.441	0.028
Back of Queue	(Q), ft	/In (95 th percentile	e)		111.8		37.3	191	.9			0		10.1	142.2	5.8
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		4.3		1.4	7.4	1			0.0		0.4	5.5	0.2
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00		0.57	0.0	0			0.00		0.07	0.00	0.00
Uniform Delay	(d 1), s/	/veh			30.1		34.2	31.	8			0.0		3.9	5.5	3.9
Incremental De	lay (<i>d</i> 2	(d 2), s/veh			1.3		0.4	3.0)			0.0		0.1	0.5	0.0
Initial Queue De	nitial Queue Delay (d 3), s/veh				0.0		0.0	0.0)			0.0		0.0	0.0	0.0
Control Delay (Control Delay (<i>d</i>), s/veh				31.5		34.5	34.	8			0.0		4.0	6.0	3.9
Level of Service	evel of Service (LOS)				С		С	С						А	Α	А
Approach Delay, s/veh / LOS				31.5		С	34.7	7		С	0.0			5.8		А
Intersection Delay, s/veh / LOS						13	3.6							В		
Multimodal Re	sults	lts			EB	_		W	B	_		NB	_		SB	_
Pedestrian LOS	Score	/ LOS	DS			В	2.29)		В	1.85		В	1.62	2	В
Bicycle LOS Sc	ore / LC)S		1.78		В	2.03	3		В	1.56	j	В	2.43	5	В

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General Inform	nation								Intersec	tion Info	ormatio	on	2	4244	× (<u>,</u>
Agency									Duration	. h	0.250	 		÷	
Analyst				Analys	is Date	e 4/12/2	2023	_	Area Tvr)e	CBD		 		₹. \$
Jurisdiction				Time F	Period				PHF		0.87		→ _^	w ^N ∈	 <!--</td-->
Urban Street		Fast		Analys	is Yea	r 2023			Analysis	Period	1> 8:0	00			 *7
Intersection		East & Front		File Na	ame	2 lane	- East 2	2024	PM S.xus		1			st tr	<u>_</u>
Project Descrip	tion	2024 PM						-					1	4 1 4 17 1	- (*
, ,															
Demand Inform	nation				EB			W	В		NB			SB	
Approach Move	ment			L	Т	R	L	Г	- R	L	Т	R	L	Т	R
Demand (v), v	eh/h			82	115			4	8 22	43	910	29		0	
				li	1										
Signal Informa	tion			-	↓		4						-		
Cycle, s	80.0	Reference Phase	2		51							1		3	→ 4
Offset, s	58	Reference Point	Begin	Green	56.1	13.1	0.0	0.0	0.0	0.0			•		5
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0	0.0	0.0			-		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.4	2.0	0.0	0.0) 0.0	0.0		5	6	7	8
T D			_	EDI		EDT			MOT	NIDI		NDT	0.01		ODT
Timer Results				EBL		EBI	VVB		WBI	NBL		NBI	SBL	-	SBI
Assigned Phase	5				_	4		-	8	<u> </u>	_	2		_	6
Case Number						0.0			7.0			8.0			8.0
Change Duration	, S					10.3			10.3	<u> </u>		61.7 E.C		<u> </u>	51.7
Max Allow Hear	way (A	c), S MAH) S		-	5.Z		-	5.2 4.3			5.0 0.0			5.0 0.0	
Queue Clearan	Headway (<i>MAH</i>), s earance Time (<i>g</i> s), s				-	10.2			4.2			0.0			0.0
Green Extensio	n Time	(ge),s				0.9			1.1			0.0			0.0
Phase Call Pro	oability	(3,), -			-	1.00		-	1.00						
Max Out Probal	bility					0.01			0.00						
	,														
Movement Gro	up Res	ults			EB			WE	3		NB			SB	
Approach Move	ment			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Assigned Move	ment			7	4			8	18	5	2	12		6	
Adjusted Flow F	Rate (v), veh/h		94	132			55	25	532		485		0	
Adjusted Satura	ation Flo	w Rate (<i>s</i>), veh/h/	In	1181	1787			168	3 1380	1690		1546		1662	
Queue Service	Time (g	g s), S		5.9	5.3			2.2	1.2	0.0		11.6		0.0	
Cycle Queue C	learance	e Time (g ₀), s		8.2	5.3	<u> </u>		2.2	1.2	11.3		11.6		0.0	
Green Ratio (g	/C)			0.16	0.16	<u> </u>		0.16	6 0.16	0.70		0.70		0.70	
Capacity (<i>c</i>), v	eh/h			251	293	<u> </u>		276	5 226	1233		1084		1165	
Volume-to-Capa	acity Ra	tio (X)		0.376	0.451	<u> </u>		0.20	0 0.112	0.431		0.448		0.000	
Back of Queue	(Q), ft	/In () 95 th percentile	e)	17.8	104.3			41.3	3 19.1	160		149		0	
Back of Queue	(Q), Ve	eh/In (95 th percent	ile)	3.1	4.1			1.6	0.8	6.4		6.0		0.0	
Queue Storage	Ratio (RQ) (95 th percent	tile)	0.86	0.00			0.00	0.19	0.00		0.00		0.00	
Uniform Delay (d 1), si	veh		31.8	29.6			28.3	3 28.5	5.6		5.7		0.0	
Incremental De	remental Delay (d_2), s/veh			0.9	1.1			0.4	0.2	1.0		1.2		0.0	
nitial Queue Delay (d ȝ), s/veh				0.0	0.0	<u> </u>		0.0	0.0	0.0		0.0		0.0	
Control Delay (evel of Service (LOS)				30.7			28.1	28.7	0.0		6.9		0.0	
Level of Service	Approach Delay, s/veh / LOS				U					A		A			
Approach Delay		31.5		0	28.7		U	6.8		A	U.U				
	intersection Delay, s/ven/ LOS					12	2.3								
Multimodal Re	modal Results							WE	3		NB			SB	
Pedestrian LOS	an LOS Score / LOS					В	1.94	ŀ	В	1.85		В	1.85		В
Bicycle LOS Sc	ore / LC)S		1.93		В	1.69)	В	2.49		В	1.55		В

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General Information								Intersec	tion Info	ormatio	on	2	4241	× (<u>,</u>
Agency								Duration	. h	0.250	- <u> </u>		Ļ	
Analyst			Analys	is Date	4/12/2	023		Area Tvr	e	CBD		 		<i>د</i>
Jurisdiction			Time P	eriod				PHF	-	0.96		→ 	w ∔ E	↓ ↓
Urban Street	Fast		Analys	is Year	2023		-	Analysis	Period	1> 8:0	00			+ ∵
Intersection	East & Jefferson		File Na	ime	2 Jane	- Fast 2	2024		i onou			- <u>-</u>	-+ +-	<u> </u>
Project Description	2024 PM					Luot	20211	W O.Xuo				1	1 / 1 1 4 1 1 1	- (*
r roject Becomption	20211111													
Demand Information				EB			W	В		NB			SB	
Approach Movement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Demand (v), veh/h			26	25			22	2 21	21	1032	2 6		0	
			1						_					
Signal Information				+	. a È	=						rta		
Cycle, s 80.0	Reference Phase	2		517	\rightarrow						1		3	→ 4
Offset, s 62	Reference Point	Begin	Green	55.8	13.8	0.0	0.0	0.0	0.0					5
Uncoordinated No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0	0.0	0.0					
Force Mode Fixed	Simult. Gap N/S	On	Red	2.0	2.0	0.0	0.0	0.0	0.0		5	6	7	8
Timer Desults		_	EDI	_	FDT				NDI		NDT	0.01	_	ODT
			EBL		EBI	VVBI	-	VVB1	NBL	·	NBI	SBL	· -	SBI
Assigned Phase					4		_	8			2			0
Case Number					8.0			8.0			8.0			8.0
Change Duration, s					19.0			19.0			50			51.0
Max Allow Headway (nange Period,(Y+R c), s ax Allow Headway(MAH), s				5.2 4.2			5.Z		-	5.Z			5.Z
Queue Clearance Time	(q_s)				4.1			4.0			0.0			0.0
Green Extension Time	(ge),s				0.2			0.2			0.0			0.0
Phase Call Probability	(9 °), °				1.00		-	0.89			0.0			
Max Out Probability					0.01			0.01						
,														
Movement Group Res	sults			EB			WB			NB			SB	
Approach Movement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Assigned Movement			7	4			8	18	5	2	12		6	
Adjusted Flow Rate (v), veh/h			53			45		569		518		0	
Adjusted Saturation Flo	ow Rate (<i>s</i>), veh/h/	In		1506			1527	7	1674		1528		1636	
Queue Service Time (g s), s			0.1			2.0		0.0		12.3		0.0	
Cycle Queue Clearanc	e Time (<i>g c</i>), s			2.1			2.0		12.3		12.3		0.0	
Green Ratio (g/C)				0.17			0.17	·	0.70		0.70		0.70	
Capacity (c), veh/h				328			263		1215		1066		1141	
Volume-to-Capacity Ra	itio (X)			0.162			0.17	2	0.468		0.486		0.000	
Back of Queue (Q), f	In (95 th percentile	e)		43.3			33.7	·	161.7		150.8		0	
Back of Queue (Q), ve	eh/In (95 th percent	ile)		1.7			1.3		6.5		6.0		0.0	
Queue Storage Ratio (RQ) (95 th percen	tile)		0.00			0.00		0.00		0.00		0.00	
Uniform Delay (<i>d</i> 1), s	/veh			28.2			28.2	<u> </u>	5.5		5.5		0.0	
Incremental Delay (d 2			1.1			0.3		1.1		1.3		0.0		
Initial Queue Delay (d			0.0			0.0		0.0		0.0		0.0		
Control Delay (d), s/v	Control Delay (d), s/veh						28.5		6.6		6.8		0.0	
Level of Service (LOS)	// 00			С			C		A		A			
Approach Delay, s/veh	Approach Delay, s/veh / LOS					28.5)	С	6.7		A	0.0		
Intersection Delay, s/ve	en / LOS				8	5						A		
Multimodal Results	Multimodal Results						WB			NB			SB	
Pedestrian LOS Score		1.93		В	1.94		В	1.62		В	1.63		В	
Bicycle LOS Score / LO	DS		1.64		В	1.63	3	В	2.47		В	1.55		В

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General Inform	nation			_				_	In	tersect	ion Infr	ormatic	n	4	at Late 1	, 4× l <u>u</u>	
Agency	lation										uration	h	0 250				
Analyst				Analys	ie Da	ato I	1/12/2	023		Δr	rea Type	<u>، انا</u>	CBD		 		۲. 4
Jurisdiction				Time E		4 1	4 /1Z/Z	025			неа туре ше	,	0.03		 -⇒	w‡e	<u>≯</u> ♦
Jurisaiction		Fast				u vor	2023				nolveje [Poriod	1 2.95	0			1 - 1
Intersection		East & Looust		File No			2023	Fact	0024			enou	1~ 0.0		- E		<u> </u>
Breiget Deserin	tion				ame		z lane	- East 2	2024	PIV	I IN.XUS				- 5	* 1 * * * * *	140
Project Descrip	lion	2024 Pivi															
Demand Inform	nation				E	B			V	٧B			NB			SB	
Approach Move	ement			L	Т	-	R	L	—	Т	R	L	Т	R	L	Т	R
Demand (v), v	eh/h			102	54	2					1		1004	209			
					1												
Signal Informa	tion						7								†		
Cycle, s	70.0	Reference Phase	2			<u>†7</u>	=>							1	2	3	4
Offset, s	30	Reference Point	Begin	Green	45.4	4	13.6	0.0	0.	0	0.0	0.0					
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2		3.2	0.0	0.	0	0.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.3		2.3	0.0	0.	0	0.0	0.0		5	6	7	8
Timer Description			_		_		DT						_			_	ODT
Assigned Dhee				EBL	-	E	BI	VVBI	-+	V	VBI	NBL	-		SBL	·	SBI
Assigned Phase	e				\rightarrow	4	4		\rightarrow				_	2			
Case Number					+	12	2.0		-				_	8.0			
Change Duration	I, S				\rightarrow	5	9. I		\rightarrow					50.9			
Max Allow Hear	, (7 + K (dway (M	с), S ИЛН) с		+	5	1		+					0.0		-		
	ce Time	$\alpha(\alpha_s)$ s	<u> </u>	+	11	17		+					0.0		+		
Green Extensio	on Time	e (g s), s (g e), s				1	.9		-					0.0			
Phase Call Pro	bability	(3-),-			+	1.	00		+								
Max Out Proba	bility					0.	31										
	, ,								1								
Movement Gro	oup Res	ults			EE	3			W	В			NB			SB	
Approach Move	ement			L	Т	\perp	R	L	Т		R	L	Т	R	L	Т	R
Assigned Move	ment			7	4	\downarrow							2	12			
Adjusted Flow I	Rate (v), veh/h		243	449	9							628	590			
Adjusted Satura	ation Flo	w Rate (s), veh/h/l	In	1646	168	3							1683	1573			
Queue Service	Time (g	g s), S		9.7	8.6	5				_			20.0	15.3			
Cycle Queue C	learance	e lime (<i>g c</i>), s		9.7	8.6	j A				_			20.0	15.3			
Green Ratio (g	<i>VC</i>)			0.19	0.19	9				_			0.65	0.65			+
Capacity (c), v	/en/n	·· /) /)		319	653	3				_			1092	1021			
Volume-to-Capa		$\frac{1}{\sqrt{2}} \int \frac{1}{\sqrt{2}} \frac{1}{\sqrt{2}} \int \frac{1}{\sqrt{2}} \frac{1}{$		0.761	0.68	39	_			\rightarrow			0.575	0.578			+
Back of Queue	$\frac{(Q)}{(Q)}$	vin (195 in percentile) ile)	101.1	153.	./				-			202.7	190.9			+
Dueue Storage	(Q), Ve	PO(95 th percent)	tilo)	7.1	0.0	,	_			\rightarrow			0.0	7.0			+
Uniform Delay	(d_1)	/veh		26.1	25	7				-	_		7.4	7.4			+
Incremental De	(<i>d</i> ₁), s/veh			4.5	1 3	2	_			\rightarrow			1.7	1.4			+
Initial Queue De	ntal Delay (<i>d</i> ₂), s/ven eue Delay (<i>d</i> ₃), s/veh			4.0	0.0	, ,	_				_		0.0	0.0			
Control Delay (Control Delay (d), s/veh			30.7	27 (0				+			9.0	9.0			+
Level of Service	Level of Service (LOS)				<u>_</u> г.	-				+			A	A			+-+
Approach Delay, s/veh / LOS				28.3		(С	0.0				9.1		A	0.0		
Intersection Delay, s/ven / LOS				20.0			- 16	5.1				0.1			B		
Multimodal Re	sults			EE	3			W	В			NB			SB		
Pedestrian LOS	Score	/LOS	1.71		E	В	1.72			В	1.86		В	1.95		В	
Bicycle LOS Sc	ore / LC	DS		1.94	-	E	В					2.64		С	0.00		А

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General Inform	nation								Intere	sect	ion Inf	ormatio	n		at _1_ata + ,	يد ايد
Agency	lation								Durat	tion	h	0 250			Ļ	
Apolyet				Analys	ic Dot	0 1/12/2	023		Aroa	Type	<u>.</u>	CRD		1		<u>د</u> 4
Analyst				Time	os Dau	.e 4/12/2	.023			туре	5	0.04		→ 	w1 ⊨	≿ -
Jurisaiction		Foot				r 2022				ioio [Doriod	1 0.94	20			4
		Edst Fast & Market				2023	East (0004			enou	12 0.	50			, in the second se
Intersection	tian			File Na	ame	2 lane	- East 2	2024	PIVI IN.	xus					¶ †	2 C
Project Descrip	lion	2024 PM														2.1.
Demand Inform	nation				EB			W	/B			NB			SB	
Approach Move	ement			L	Т	R	L	T -	г	R	L	Т	R	L	Т	R
Demand (v), v	eh/h			137	0			+			126	1008	3		0	
Signal Informa	tion				↓											
Cycle, s	70.0	Reference Phase	2		.	E								N	_	4
Offset, s	14	Reference Point	Begin	Green	44 5	14.6	0.0			0	0.0		1	2	3	4
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.).0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.3	2.2	0.0	0.	0 0	0.0	0.0		5	6	7	8
Timer Results				EBL		EBT	WB	-	WBT	Г	NBL	-	NBT	SBL		SBT
Assigned Phase	e					4							2			6
Case Number						12.0							8.0			8.0
Phase Duration	i, S					20.0							50.0			50.0
Change Period	, (Y+ R (c), S				5.4							5.5			5.5
Max Allow Head	leadway (MAH), s					4.2							0.0			0.0
Queue Clearan	ow Headway (<i>MAH</i>), s Clearance Time (<i>g</i> ₅), s					8.3										
Green Extensio	n Time	(ge),s				0.2							0.0			0.0
Phase Call Pro	bability					1.00										
Max Out Proba	bility					0.19										
Movement Gre		ulte	_		ER			\٨/١	2			NR			SB	
Approach Move	mont	Suits			ED	P				,	-		D		т	D
Assigned Move	ment			7	1	IX.		1			5	2	IX.		6	
Adjusted Flow F	Rate (v) veh/h		<u> </u>	146						619	587			0	
Adjusted Satura	ation Flo	w Rate (s) veh/h/	In		1607	,			+-		1471	1418			1825	
Queue Service	Time (a	a s). S			6.3	_			-		0.0	16.9			0.0	
Cvcle Queue C	learance	e Time (6.3				+		5.2	16.9			0.0	
Green Ratio (g	/C)	- ····· (3 ·), -			0.21						0.64	0.64			0.64	
Capacity (c), v	, /eh/h				335						997	902			1160	
Volume-to-Cap	acity Ra	itio(X)			0.435	5					0.621	0.651			0.000	
Back of Queue	(Q), ft	/In (95 th percentile	e)		134.4	1					64.3	70.5			0	
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		5.2					T	2.6	2.7			0.0	
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00						0.00	0.00			0.00	
Uniform Delay	(d1), s/	/veh			30.9						1.2	1.3			0.0	
Incremental De	niform Delay (<i>a</i> 1), s/ven icremental Delay (<i>d</i> 2), s/veh				2.9						2.9	3.6			0.0	
Initial Queue De	nitial Queue Delay ($d = 3$), s/veh				0.0						0.0	0.0			0.0	
Control Delay (d), s/veh					33.8						4.1	4.9			0.0	
Level of Service	Level of Service (LOS)				С						А	Α				
Approach Delay, s/veh / LOS				33.8		С	0.0				4.5		А	0.0		
Intersection Delay, s/veh / LOS						7	.7							A		
	Aultimodal Posulta								_						05	
Multimodal Re	ion LOS Score / LOS				EB	_	4.07	W	5		4.0.1	NB	•	4.07	SB	_
Pedestrian LOS	Score	/ LUS	1.94		В	1.95)	В	_	1.34		A	1.35		A	
BICYCIE LOS SC	ore / LC	15		1.79		В					2.55		C	1.55		В

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General Inform	nation								Interse	ction	Info	ormatic	n	K	4 .L	þa l <u>a</u>
Agency	lation								Duratic	n h		0 250			Ļ	
Analyst				Analys	is Date	- 4/12/2	023		Area Ty	/ne		CBD		 24		<u>ار</u> الح
Jurisdiction				Time F	Period	, , , , , , , , , , , , , , , , , , , ,	.020		PHE	/pc		0.97		→ 	w∔e	2- 4-
Urban Street		Fast		Analys	is Yea	2023			Analys	s Peri	bd	1> 8.0	0	-4		+ ₹
Intersection		East & Monroe		File Na	ame	2 Jane	- Fast 2	2024	PM S xi	IS IS	Ju	1. 0.0		- <u>-</u>	.4.4	<u> </u>
Project Descrip	tion	2024 PM					- Lust z	-02-	1 10 0.20	10						÷ ۲
Troject Descrip		202411														
Demand Inform	nation				EB			N	/B			NB			SB	
Approach Move	ement			L	Т	R	L	<u> </u>	T R		L	Т	R	L	Т	R
Demand (v), v	eh/h			48	0						19	1054			0	
				1												
Signal Informa	tion	r	1		†	2								-+		-
Cycle, s	80.0	Reference Phase	2		51	F							1	2	3	→ 4
Offset, s	58	Reference Point	Begin	Green	54.8	14.8	0.0	0.0	0 0.0) (0.0					
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.	0 0.0) (0.0			L _	╱	
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.0	2.0	0.0	0.	0 0.0) (0.0		5	6	7	8
			_													
Timer Results				EBL		EBT	WBI		WBT		NBL		NBT	SBI		SBT
Assigned Phase	e				\rightarrow	4		\rightarrow		_		_	2			6
Case Number						12.0		\rightarrow		_			8.0			8.0
Phase Duration	i, s	\ \		<u> </u>	\rightarrow	20.0	<u> </u>	\rightarrow		-			50.0	<u> </u>		60.0
Change Period	, (Y+R a	c), S	<u> </u>		5.2	<u> </u>	\rightarrow		-		_	5.2	<u> </u>	_	5.2	
Max Allow Head	dway(A	ИАН), s	<u> </u>	+	4.2	<u> </u>	\rightarrow		-		_	0.0	<u> </u>	_	0.0	
Queue Clearan	ance Time (g_s) , s					4.4	<u> </u>	\rightarrow		-			0.0			0.0
Green Extensio	n lime	(ge), s			\rightarrow	0.1		\rightarrow		-		_	0.0			0.0
Phase Call Pro	bability				_	1.00	<u> </u>			-					_	
Max Out Proba	DIIITY					0.00										
Movement Gro	oup Res	ults			EB			W	3	T		NB			SB	
Approach Move	ement			L	Т	R	L	Т	R			Т	R	L	Т	R
Assigned Move	ment			7	4					5		2			6	
Adjusted Flow I	Rate (v), veh/h			49					57	9	529			0	
Adjusted Satura	ation Flo	w Rate (<i>s</i>), veh/h/l	n		1632					178	31	1628			1599	
Queue Service	Time (g	g s), S			2.4					0.	0	11.4			0.0	
Cycle Queue C	learance	e Time (<i>g c</i>), s			2.4					2.	8	11.4			0.0	
Green Ratio (g	/C)				0.18					0.6	69	0.69			0.69	
Capacity (c), v	/eh/h				302					12	67	1115			1095	
Volume-to-Cap	acity Ra	tio(X)			0.164					0.4	57	0.474			0.000	
Back of Queue	(Q), ft	/In (95 th percentile	e)		49.6					37	.7	37.5			0	
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		1.9					1.	5	1.5			0.0	
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00					0.0	00	0.00			0.00	
Uniform Delay	(d 1), s/	/veh			33.8					0.	8	0.8			0.0	
Incremental De	ntal Delay (<i>d</i> ₂), s/veh				1.2					1.	0	1.2			0.0	
Initial Queue De	itial Queue Delay (d ȝ), s/veh				0.0					0.	0	0.0			0.0	
Control Delay (ontrol Delay (<i>d</i>), s/veh				35.0					1.	9	2.1			0.0	
Level of Service	ervice (LOS)				С					Α	\	А				
Approach Delay	proach Delay, s/veh / LOS					С	0.0				2.0		А	0.0		
Intersection De	ntersection Delay, s/veh / LOS					3	.4							A		
Multimodal Re	sults				EB	_		W	3			NB			SB	
Pedestrian LOS	Score	/ LOS	1.95	,	В	1.96		В		.34		A	1.34	•	A	
Bicycle LOS Sc	ore / LC	05		1.63	5	В				2	2.47		В	1.55		В

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General Inform	nation								Intersec	tion Info	ormatio	on	4	***	s l <u>a</u>
Agency									Duration	, h	0.250			Ļ	
Analvst				Analvs	is Date	4/12/2	023	_	Area Tvo	e	CBD		 		₹
Jurisdiction				Time P	eriod				PHF		0.92		→ ∻ →	W + E	↓ ↓ ↓
Urban Street		East		Analys	is Year	2023		_	Analysis	Period	1> 8:0	00			+- *
Intersection		East & Olive		File Na	me	2 lane	- East 2	2024	PM S.xus					ৰ দি	
Project Descrip	tion	2024 PM											1	1 1 4 M 1	* /*
													- I		
Demand Inform	nation				EB			W	'B		NB			SB	
Approach Move	ement			L	Т	R	L		r R	L	Т	R	L	Т	R
Demand (v), v	eh/h			79	60			3	5 27	16	830	14		0	
Signal Informa	tion					5			_						
	80.0	Peference Phase	2		+	12 8	=						KTZ -		~
Offset s	52	Reference Point	2 Begin									1	2	3	4
Uncoordinated	No	Simult Can E/M	On	Green	53.4	15.5	0.0	0.0	0.0	0.0	_				A
Force Mode	Fixed	Simult, Gap N/S	On	Red	3.2	3.2	0.0	0.0		0.0	-	5	6	7	8
T OFCE MODE	TIXCU	onnuit. Cap 14/0	Oll	Ticu	2.7	2.5	0.0	10.0	0.0	0.0			, in the second		
Timer Results	_			EBL		EBT	WB	L	WBT	NBL		NBT	SBL	_	SBT
Assigned Phase	e					4			8			2			6
Case Number				<u> </u>		8.0		-	8.0			8.0			8.0
Phase Duration	I, S					21.0			21.0			59.0			59.0
Change Period	ge Period, (Y+R c), s					5.5		_	5.5			5.6			5.6
Max Allow Head	(Allow Headway (<i>MAH</i>), s					4.2			4.2			0.0			0.0
Queue Clearan	ax Allow Headway (<i>MAH</i>), s leue Clearance Time (<i>g</i> s), s					9.6			3.4						
Green Extensio	on Time	(ge),s				0.4			0.6			0.0			0.0
Phase Call Pro	bability					1.00			0.99						
Max Out Proba	bility					0.32			0.01						
Mayamant Cra	un Dee	ulto			ГР			۱۸/۲	>		ND			CD.	
Approach Move	mont	Suits			 	D						D		оd т	D
	ment				1			8	18	L 5	2	12		6	TX
Adjusted Flow F	Rate (v) veh/h			151			3/	33	190	2	12	_	0	
Adjusted Satura	ation Flo	w Rate (s) veh/h/	In		1404			178	4 1627	1552		1410		1825	
Queue Service	Time (a	π_s) s			6 1			1.3	1 4	0.0		12.2		0.0	
Cycle Queue C	learance	e Time (a_c) s			7.6			1.3	1.1	12.1		12.2		0.0	
Green Ratio (a	/C)	o milo (g o), o			0.19			0.19	9 0.19	0.67		0.67		0.67	
Capacity (c), y	/eh/h				343			346	315	1082		941		1218	
Volume-to-Cap	acitv Ra	tio (X)			0.441			0.09	8 0.106	0.453		0.473		0.000	
Back of Queue	(Q), ft	/In (95 th percentile	e)		134.2			24.8	3 24.1	161.4		149.6		0	
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		5.2			1.0	0.9	6.5		6.0		0.0	
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00			0.0	0.00	0.00		0.00		0.00	
Uniform Delay ((d1), s	/veh			28.9			26.	5 26.5	6.4		6.4		0.0	
Incremental De	ncremental Delay (<i>d</i> 1), s/ven				3.7			0.1	0.1	1.4		1.7		0.0	
Initial Queue Delay (d 3), s/veh					0.0			0.0	0.0	0.0		0.0		0.0	
Control Delay (d), s/veh					32.7			26.0	6 26.7	7.8		8.1		0.0	
Level of Service	Level of Service (LOS)				С			С	С	Α		Α			
Approach Delay, s/veh / LOS				32.7		С	26.7	7	С	7.9		А	0.0		
Intersection Delay, s/veh / LOS						12	.3						В		
Multimodal Results														~-	
Multimodal Re	lultimodal Results				EB		4.04	WE	5	4.00	NB		4.00	SB	
Pedestrian LOS	Score	/ LUS		1.93		В	1.94		В	1.86		В	1.86		В
BICYCIE LOS SC	ore / LC	15		1.80		В	1.62	<u> </u>	В	2.33		В	1.55		В

			Ű								,					
General Inform	nation								Inte	ersect	ion Info	ormatic	n	4	4244	s l <u>a</u>
Agency									Dur	ration,	h	0.250			. ↓	
Analyst				Analys	is Date	e 4/12/2	2023		Are	a Type	e	CBD		 		₹
Jurisdiction				Time F	eriod				PHF	F		0.93			W TE	~_4 +
Urban Street		East		Analys	is Yea	2023			Ana	alvsis I	Period	1> 8:0	00			+ ¥
Intersection		East & Washington		File Na	me	2 lane	- East 2	2024	PM S	S.xus			-		5 4 4 7	
Project Descrip	tion	2024 PM												1	ব↑ ⇔ ₩1	* /*
							Y				1r			r		
Demand Inform	nation				EB			N	/B			NB			SB	
Approach Move	ement			L	Т	R	<u> </u>		Т	R	L	Т	R		Т	R
Demand (v), v	eh/h			34	222			28	85	87	31	906	69		0	
Signal Informa	tion					5										
	80.0	Reference Phase	2	c .	I [▲] .	La P	7							N 2		
Offset s	60	Reference Point	Begin		1								1	2	3	4
	No	Simult Gap E/W	On	Green	48.2	20.4	0.0	0.0	0	0.0	0.0	_				A
Force Mode	Fixed	Simult Gap N/S	On	Red	3.Z 2.8	3.Z	0.0	0.0	0	0.0	0.0	-	5	6	7	8
	T INCU	onnan: oup n/o	en	Ttou	2.0		0.0	0.	0	0.0	0.0					
Timer Results				EBL		EBT	WB	L	WE	BT	NBL	-	NBT	SBL		SBT
Assigned Phase	e					4			8	3			2			6
Case Number						6.0			7.	.0			5.0			8.0
Phase Duration	, S					25.8			25	5.8			54.2			54.2
Change Period	od, (Y+ <i>R c</i>), s eadway (<i>MAH</i>), s					5.4			5.	.4			6.0			6.0
Max Allow Head	w Headway (<i>MAH</i>), s					4.2			4.	.2			0.0			0.0
Queue Clearan	Allow Headway (<i>MAH</i>), s ue Clearance Time (<i>g</i> s), s					18.4			15	5.5						
Green Extensio	n Time	(ge),s				1.8			2.	.1			0.0			0.0
Phase Call Pro	bability					1.00			1.0	00						
Max Out Proba	bility					0.31			0.1	13						
Movement Gro		ulte			EB			\٨/٢	B			NB			SB	
Approach Move	mont	Juits			T	R	1			R	1	T	R		Т	R
Assigned Move	ment			7	4			8	+	18	5	2	12		6	
Adjusted Flow F	Rate (v) veh/h	_	37	239			306	6	94	33	957	73		0	
Adjusted Satura	ation Flo	w Rate (s) veh/h/	In	965	1789			166	5 2 1	1384	1629	1703	1528		1678	
Queue Service	Time (a	T_{s}) s		2.9	9.2	<u> </u>		13	5	4.3	0.6	12.2	1.6		0.0	
Cvcle Queue C	learance	e Time (16.4	9.2			13.	5	4.3	0.6	12.2	1.6		0.0	
Green Ratio (g	/C)	· ····· (3 ·), ·		0.25	0.25	<u> </u>		0.2	5 0	0.25	0.60	0.60	0.60		0.60	
Capacity (c), v	/ veh/h			173	456			424	4 :	353	1071	2053	921		1011	
Volume-to-Cap	acity Ra	tio(X)		0.211	0.523	1		0.72	23 0	.265	0.031	0.466	0.079		0.000	
Back of Queue	(Q), ft	/In (95 th percentile	e)	31.3	173.7	1		238	.3 6	64.5	9.4	179.5	21.7		0	
Back of Queue	(Q), ve	eh/In (95 th percent	ile)	1.2	6.8	1		9.4	1	2.5	0.4	7.1	0.9		0.0	
Queue Storage	Ratio (RQ) (95 th percen	tile)	0.48	0.00	1		0.0	0 0	0.81	0.12	0.00	0.00		0.00	
Uniform Delay	(d1), s/	/veh		34.7	25.6			27.	2 2	23.8	6.3	8.6	6.5		0.0	
Incremental De	ncremental Delay (<i>d</i> 2), s/veh			0.5	0.8			3.3	3	0.4	0.0	0.6	0.1		0.0	
Initial Queue De	initial Queue Delay (d_3), s/veh			0.0	0.0			0.0)	0.0	0.0	0.0	0.0		0.0	
Control Delay (Control Delay (<i>d</i>), s/veh				26.4			30.	5 2	24.2	6.4	9.2	6.6		0.0	
Level of Service	evel of Service (LOS)				С			С		С	А	A	A			
Approach Dela	Approach Delay, s/veh / LOS					С	29.0)	С)	9.0		А	0.0		
Intersection De	lay, s/ve	h / LOS				16	6.5							В		
Mall	Aultimodal Results							1.4.7				NIE			05	
Multimodal Re	ultimodal Results					B	4.00	VVE	5		4.07	NB	В	4.00	SB	B
Pedestrian LOS	o Score	100		2.28		D	1.93		B		1.8/		B	1.88		D
Dicycle LUS SC	ore / LC	13		2.01		D	Z.22	<u> </u>	В	0	2.45		D	1.55		D

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HCS[™] Streets Version 2023

	Il Information									-	,					
General Inform	ation								Interse	ction I	nforma	tion		لير	4144	ta la
Agency									Duratio	1. h	0.2	50			4 ↓	
Analyst				Analys	is Date	4/12/2	023	_	Area Tv	ne	CB	D		 		۲. 4
Jurisdiction				Time F	Period	, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	.020		PHE		0.9	7		→ +	w∔e	+ ↓
Urban Street		Madison		Analys	is Year	2023			Analysi	: Perio	1 1>	, 8∙00		4		± ₽
Intersection		Madison & Front		File Na	ame	2 Jane	- Madie	son 20)24 PM 9		· []'	0.00		·	•	
Project Descript	tion	2024 PM		1 110 110			maan	5011 20		5.740			_	٦ ۲	414Y	1 4
r rojoot Dooonp		2021111														
Demand Inform	nation				EB			W	В		N	IB			SB	
Approach Move	ment			L	Т	R	L	Т	R	L		T	R	L	Т	R
Demand (v), v	eh/h				35	13	53	6	5			0		22	1026	27
				i						-				_		
Signal Informa	tion				<mark>⊿∔</mark> a	÷							$\mathbf{\lambda}$			
Cycle, s	80.0	Reference Phase	2		ľ ↑	Ř						1	-+	2	3	
Offset, s	8	Reference Point	Begin	Green	59.8	9.0	0.0	0.0	0.0	0.	0					_
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0	0.0	0.	0			1		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.5	2.3	0.0	0.0	0.0	0.	0	5		6	7	8
Timer Deculto				ГРІ		грт				N		NDT	- 1	CDI		CDT
Assigned Phase	<u></u>			EDL			VVD						+	SDL		2
Assigned Flase	=			<u> </u>		4		-	6.0			0	+			2
Case Number				<u> </u>	-	0.0	<u> </u>	-	14.5	-		0.0 65 5	+			0.0 65.5
Change Period	, s (V+P,					5.5		+	5.5	-		5.7	+			57
Max Allow Hear	renod, (Y+R c), s r Headway (MAH), s					4.2	<u> </u>	-	4.2	-		0.0	-		-	0.0
Queue Clearan	Allow Headway (<i>MAH</i>), s ie Clearance Time (<i>g</i> s), s					4.3		-	7.7			0.0	+			0.0
Green Extensio	eue Clearance Time (g_s), s					0.5		-	0.5	-	-	0.0	-			0.0
Phase Call Prot	pability	(90),0				0.98		+	0.98	-		0.0	+			0.0
Max Out Probat	bility					0.00			0.00				-			
	. ,					0.00			0.00							
Movement Gro	oup Res	sults			EB			WB	5		NE	3			SB	
Approach Move	ment			L	Т	R	L	Т	R	L	Т	F	2	L	Т	R
Assigned Move	ment				4	14	3	8			6			5	2	12
Adjusted Flow F	Rate (v), veh/h			49		55	67			0			645		584
Adjusted Satura	ation Flo	ow Rate (<i>s</i>), veh/h/	In		1549		1247	1789	9		170)5		1708		1549
Queue Service	Time (g	g s), S			2.3		3.3	2.7			0.0)	_	0.0		12.5
Cycle Queue Cl	learance	e Time (<i>g c</i>), s			2.3		5.7	2.7			0.0)	_	12.4		12.5
Green Ratio (g	/C)				0.11		0.11	0.11		_	0.7	5	_	0.75		0.75
Capacity (c), v	eh/h				175		194	202		-	127	'4	_	1323		1157
Volume-to-Capa	acity Ra	itio (X)	<u>, </u>		0.283		0.281	0.33	2	-	0.00	00	4	0.487		0.505
Back of Queue	(Q), ft	/In (95 th percentile	e)		40.9		47.2	55.5)		0		-	154		140.3
Back of Queue	(Q), Ve	eh/In (95 th percent	ile)		1.6		1.9	2.2		-	0.0)	-	6.1		5.6
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00		0.79	0.00			0.0	0	-	0.00		0.00
Uniform Delay (Iniform Delay (d_1), s/veh				32.3		34.8	32.4	•	-	0.0)	-	4.2		4.3
ncremental Delay (d ₂), s/veh					0.9		0.8	1.0		-	0.0)	-	1.1		1.3
nitial Queue Delay (<i>d</i> ₃), s/ven					22.4		0.0	0.0		-	0.0	, ,	-	0.0		0.0
Level of Sonvice	Level of Service (LOS)				- 33.1 C		35.0 D	33.4	•	-	0.0	,	-	0.5		0.0
Approach Dolo		22.1		C	34 /				0			5.5				
Intersection Dol		55.1		0	0	•	U	0			^	5.5		~		
	intersection Delay, siven / 200					9							7	`		
Multimodal Re	ultimodal Results				EB			WB	;		NE	3			SB	
Pedestrian LOS	odal Results an LOS Score / LOS					В	1.94	1	В	1.	84	В		1.61		В
Bicycle LOS Sc	ore / LC)S		1.64		В	1.76	3	В	1.	56	В		2.47		В

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	al Information										•••	J					
General Inform	nation									Inte	ersecti	on Inf	ormatio	on	J.	┥┵┿↓	be la
Agency	lation									Du	ration	h	0 250			4 4	
Analyst				Analys	is Da	te 4	4/12/20)23		Are	ea Type		CBD		 		۲. 4
Jurisdiction				Time F	Period		1, 12,20			PH	IF	·	0.85		→ \$- ,	w‡e	<u>≁</u>
Urban Street		Madison		Analys	is Yea	ar í	2023			An	" alvsis F	Period	1> 8.0	20	- 1		+ ₹
Intersection		Madison & Jefferso	n	File Na	ame		2 lane -	- Madis	on 20)24	PMS	us	1 0			•	
Project Descrip	tion	2024 PM		1 110 110				maare			1 11 0.				ň	ৰ ↑ ক প	* /*
r reject becomp		2021111															
Demand Inform	nation				EB	3			W	Β			NB			SB	
Approach Move	ement			L	Т		R	L	Т	-	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				19)	9	25	2	1			0		18	1046	17
				b.													
Signal Informa	tion	Y		4	14	•											
Cycle, s	80.0	Reference Phase	2		1	t I	≓ ‴	1							2	3	
Offset, s	6	Reference Point	Begin	Green	51.6	3	17.9	0.0	0.0)	0.0	0.0			-		
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2		3.2	0.0	0.0)	0.0	0.0			1		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.2		1.9	0.0	0.0)	0.0	0.0	_	5	6	7	8
				1			_								1		
Timer Results				EBL	-	E	BT	WBI	- -	W	/BT	NBI	-	NBT	SBL		SBT
Assigned Phase	9				_		4		\rightarrow	8	8			6			2
Case Number						8	.0		\rightarrow	8	3.0			8.0			8.0
Phase Duration	, S					23	3.0		\rightarrow	23	3.0			57.0			57.0
Change Period,	nge Period, (Y+R c), s Allow Headway (<i>MAH</i>), s					5	.4		\rightarrow	5	5.4			5.4			5.4
Max Allow Head	Allow Headway (<i>MAH</i>), s eue Clearance Time (<i>q</i> s), s				_	4	.2		\rightarrow	4	.2			0.0			0.0
Queue Clearan	eue Clearance Time (g_s), s				\rightarrow	3	.3		\rightarrow	3	3.9						
Green Extensio	n Time	(ge),s			_	0	.2		\rightarrow	0).2			0.0			0.0
Phase Call Prol	bability				\rightarrow	1.	00		\rightarrow	0.	.86		\rightarrow				
Max Out Proba	bility					0.	00			0.	.00						
Movement Gro	un Res	aults	_		FB		_		WF	3			NB			SB	
Approach Move	ment				Т		R	1	Т	, 	R	1	Т	R		Т	R
Assigned Move	ment				4	+	14	3	. 8	+		_	6		5	2	12
Adjusted Flow F	Rate (v), veh/h			33				54				0		667		605
Adjusted Satura	ation Flo	w Rate (s), veh/h/	In		1568	8			158	2	-		1689		1655		1504
Queue Service	Time (d	a s). S			1.3	+			0.2			_	0.0		0.0		14.2
Cvcle Queue C	learance	e Time (<i>q</i> c). s			1.3	+			1.9	+			0.0		14.2		14.2
Green Ratio (g	/C)				0.22	2			0.22	2	_		0.64		0.64		0.64
Capacity (c), v	/ /eh/h				345	; †			417	· †			1089		1114		970
Volume-to-Cap	acity Ra	itio(X)			0.09	5			0.13	0			0.000		0.599		0.624
Back of Queue	(Q), ft	/In (95 th percentile	e)		24.6	3			37.5	5			0		167.5		156.2
Back of Queue	(Q), Ve	eh/In (95 th percent	ile)		1.0	╈			1.5				0.0		6.6		6.2
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00	5			0.00	5			0.00		0.00		0.00
Uniform Delay ((d1), s	/veh			24.9	7			25.1	1			0.0		4.9		4.9
Incremental De	Jniform Delay (d 1), s/veh ncremental Delay (d 2), s/veh				0.5	╈			0.1	╈			0.0		2.4		3.0
nitial Queue Delay (<i>d</i> 3), s/veh					0.0	╈			0.0				0.0		0.0		0.0
Control Delay (d), s/veh					25.4	1			25.2	2			0.0		7.3		7.9
Level of Service (LOS)					С				С						A		A
Approach Delay, s/veh / LOS				25.4		(С	25.2	2	(С	0.0			7.6		A
Intersection Delay, s/veh / LOS							8.	7							A		
											أعيروا						
Multimodal Re	Iultimodal Results				EB				WE	3			NB			SB	
Pedestrian LOS	Score	/LOS		1.93	5	E	В	1.93	;	E	В	1.64	-	В	1.64		В
Bicycle LOS Sc	ore / LC	DS		1.61		E	В	1.65	;	E	В	1.56	;	В	2.61		С

			5								J					
General Inform	nation								Inter	rsecti	on Infe	ormatic	n	4	ad _1_abs \$ \$	ι Ļ _k
Agency									Dura	ation,	h	0.250			5 † †	
Analvst				Analvs	is Dat	e 4/12/2	023		Area	a Type	;	CBD		- - 1 - 4		r. 4.
Jurisdiction				Time P	eriod				PHF	:		0.94			w∔e	
Urban Street		Madison		Analvs	is Yea	r 2023			Anal	Ivsis F	Period	1> 8:0)0	4		 ∵
Intersection		Madison & Locust		File Na	ime	2 lane	- Madis	son 20)24 P	, M N.)	kus				- †	
Project Descrip	tion	2024 PM		И											41491	۲ (^۳
							14				14					
Demand Inform	nation				EB			W	В			NB			SB	
Approach Move	ement			L	Т	R	L	Т	-	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				444	63	4	5	7			0		196	1182	165
	41a.m			1	6 I I -	-	_	_			-					
Signal Informa	tion	Deference Dhees	2	-	₽ ∱,¤	. 5										
Cycle, s	70.0	Reference Phase	Z		[↑								1	2	3	4
Ulisel, s	33 No	Simult Con 5/M	Begin	Green	47.8	14.2	0.0	0.0)	0.0	0.0					
Uncoordinated	INO Fixed	Simult. Gap E/W	On	Yellow	3.0	3.0	0.0	0.0		0.0	0.0	-	-	T	_	\mathbf{x}
Force Mode	Fixeu	Simult. Gap N/S	OII	Reu	1.0	1.0	0.0	0.0	, ,	0.0	0.0		5	0	I	M °
Timer Results				EBI		EBT	WB		WB	RT 🛛	NBI		NBT	SBI		SBT
Assigned Phase	<u> </u>				-	8		-	4			-	6			2
Case Number					-	8.0		+	. 8.0	2			8.0			7.0
Phase Duration	S					18.2		-	18.	2			51.8			51.8
Change Period	, c (Y+R)	c). S			+	4.0		-	4.0	2			4.0			4.0
Max Allow Head	ange Fendu, (7+K c), s ax Allow Headway (<i>MAH</i>), s					4.0			4.0	- C			0.0			0.0
Queue Clearan	ax Allow Headway (<i>MAH</i>), s ueue Clearance Time (<i>g</i> s), s					12.9			12.	9						
Green Extensio	n Time	(ge),s				1.2			1.2	2			0.0			0.0
Phase Call Prol	bability					1.00			1.0	0						
Max Out Proba	bility					0.48			0.4	8						
	_	14			ED				`			ND			0.0	
Movement Gro	oup Res	sults			EB				3	_		NB	D		SB	
Approach Move	ement			L	 	R 10		1	_	R	L	I C	ĸ	L	1	R 10
Assigned Move	meni Dete (v) yeb/b			0	18	1	4	_	-		0		5 757	Z	169
Adjusted Flow r), ven/n	In		1602	1625		154	1	-		1710		151	1522	1440
					1003	1035		0.0		-		0.0		19.7	10.2	2.0
	learance	$g(a_{\alpha})$			9.9 Q Q	10.9		10.0		-		0.0		21.0	10.2	2.9
Green Ratio (a	\sqrt{C}	c finic (<i>g t</i>), 3			0.20	0.20		0.20	י ר	-		0.0		0.68	0.68	0.68
Capacity (c) y	/eh/h				342	333		368	3			1167		1126	1045	989
Volume-to-Cap	acity Ra	itio (X)			0.797	0.802		0.17	6			0.000		0.673	0.678	0.170
Back of Queue	(Q), ft	/In (95 th percentile	e)		212.2	206.7		40.7	7			0		246.1	235	32.8
Back of Queue	(Q), ve	eh/In (95 th percent	, ile)		8.4	8.3		1.6				0.0		9.7	9.3	1.3
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00	0.00		0.00)			0.00		0.00	0.00	0.00
Uniform Delay ((d1), s	/veh			26.5	26.5		24.0)			0.0		6.9	6.6	4.0
Incremental De	lay (<i>d</i> 2), s/veh			7.1	7.5		0.1				0.0		3.2	3.5	0.4
Initial Queue Delay (d 3), s/veh					0.0	0.0		0.0				0.0		0.0	0.0	0.0
Control Delay (<i>d</i>), s/veh					33.6	34.1		24.1	1			0.0		10.1	10.2	4.4
Level of Service (LOS)					С	С		С						В	В	А
Approach Delay, s/veh / LOS				33.8		С	24.1		С		0.0			9.5		А
Intersection Delay, s/veh / LOS						15	5.8							В		
Multimedal D	Multimodal Results								,			ND			00	
Aultimodal Results				1 00	EB	P	2 10	VVE		-	1 05	INB	B	1 05	3B	B
Ricycle I OS So	OS Score / LOS					R	0.50	-	Δ		1.00		B	2 01		C
	SIC/LC			2.00		5	0.08	·	Л		1.50		5	2.31		9

			s eigi								J					
General Inform	nation						_		Inter	rsecti	on Infe	ormatio	on	J.	╡╎╬╪╸↓	la la
Agency									Dura	ation.	h	0.250				
Analyst				Analys	is Dat	e 4/12/2	2023	_	Area	a Type	<u>.</u>	CBD		 		۲. ۸
Jurisdiction				Time F	Period				PHF	:	·	0.94		→ \$- ,	w↓e	↓ ↓
Urban Street		Madison		Analys	is Yea	r 2023			Anal	lvsis F	Period	1> 8.0	00	- 1		→ ¥ ⊊
Intersection		Madison & Market		File Na	ame	2 Jane	- Madie	son 2(124 P	M N y		1. 0.			A	<u> </u>
Project Descrip	tion	2024 PM					- Madi	5011 20	5271	101 1 1.7	105			-	* 1 * *	17 1
T TOJECT Descrip		2024110														
Demand Inform	nation				EB			W	B			NB			SB	
Approach Move	ement			L	Т	R	L	Г	-	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				124	108	24	10)8			0		12	931	146
Signal Informa	ation	1	1		144		-									
Cycle, s	70.0	Reference Phase	2		1	· 🗟 "							1	2	3	
Offset, s	52	Reference Point	Begin	Green	44.6	14.8	0.0	0.0)	0.0	0.0	_		-		
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0)	0.0	0.0			1		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.0	2.2	0.0	0.0)	0.0	0.0		5	6	7	8
Timer Results				EBL	-	EBT	WB	L	WB	BT	NBL	-	NBT	SBL		SBT
Assigned Phase	е					4			8				6			2
Case Number						8.0			6.0)			8.0			8.0
Phase Duration	i, s					20.2			20.2	2			49.8			49.8
Change Period	, (Y+R (c), S				5.4			5.4	1			5.2			5.2
Max Allow Head	Allow Headway (<i>MAH</i>), s					4.3			4.3	3			0.0			0.0
Queue Clearan	eue Clearance Time (g_s), s					12.5			14.2	2						
Green Extensio	n Time	(ge),s				0.8			0.6	3			0.0			0.0
Phase Call Pro	bability					1.00			1.00	0						
Max Out Proba	bility					0.41			0.88	8						
Movement Gro	un Res	ults			FB			W/F	2			NB			SB	
Approach Move	ment				Т	R			,	R	1	Т	R		т	R
Assigned Move	ment				4	14	3	8			-	6		5	2	12
Adjusted Flow Flow	Rate (v), veh/h			247		26	115	;			0		710		619
Adjusted Satura	ation Flo	w Rate (s), veh/h/	In		1547		1019	175	4	-		1922		1595		1382
Queue Service	Time (d	(s), S			10.5	1	1.7	3.9	-			0.0		0.0		20.5
Cycle Queue C	learance	e Time (<i>q</i> c), s			10.5		12.2	3.9				0.0		20.2		20.5
Green Ratio (g	/C)				0.21		0.21	0.2	1			0.64		0.64		0.64
Capacity (c), v	, /eh/h				327		166	371				1225		1069		880
Volume-to-Cap	acity Ra	itio(X)			0.754	I I	0.154	0.31	0			0.000		0.665		0.703
Back of Queue	(Q), ft	/In (95 th percentile	e)		191.9)	19.1	71.7	7			0		239.6		218.5
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		7.6		0.8	2.8				0.0		9.4		8.7
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00		0.00	0.00)			0.00		0.00		0.00
Uniform Delay	(d 1), si	/veh			25.9		31.7	23.3	3			0.0		8.2		8.3
Incremental De	lay (<i>d</i> 2), s/veh			5.9		0.3	0.4				0.0		2.4		3.4
Initial Queue Delay (d_3), s/veh					0.0		0.0	0.0				0.0		0.0		0.0
Control Delay (d), s/veh					31.9		32.0	23.7	7			0.0		10.6		11.7
Level of Service (LOS)					С		С	С						В		В
Approach Delay, s/veh / LOS				31.9		С	25.2	2	С		0.0			11.1		В
Intersection Delay, s/veh / LOS						15	5.3							В		
Multime del D	Multimodal Results				EB			10/5	,						00	
Redestrian L CC	Aultimodal Results				EB	P	1.00	VVE			1.00	INB	P	1.60	58	B
Rievelo LOS So	LOS Score / LOS				-	B	1.92		D	-	1.00	·	B	1.03		C
Dicycle LUS SC	UIE / LC			1.97		Б	1.78	,	D		1.50		Б	2.52		U

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General Inform	nation							_	Inte	ersect	ion Inf	ormatio	on	4	****	≜ L <u>a</u>
Agency									Dur	ration.	h	0.250			7††	
Analyst				Analys	is Dat	e 4/12/2	023	_	Are	a Type	7 	CBD		 25		たみ
Jurisdiction				Time F		0 1/ 12/2	.020		PH	F	,	0.92		→ + +	w‡e	← <mark>⊱</mark>
Urban Street		Madison		Analys	is Yea	r 2023		_	Ana	alvsis F	Period	1> 8.0	00			+ +
Intersection		Madison & Olive		File Na	me	2 Jane	- Madis	on 2	024	PM.S		1. 0.0		- <u>-</u>		<u> </u>
Project Descrip	tion						- Madia	5011 2	024	1 10 0.2	<u></u>			-	ৰ কিপা	• (*
Troject Descrip	lion	2024110														
Demand Inform	nation				EB			W	/B			NB			SB	
Approach Move	ement			L	Т	R	L	1	Г	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				81	43	9	2	9			0		48	1037	46
<u>.</u>				1				1		1						
Signal Informa	tion				₽ ∱₹	5										
Cycle, s	80.0	Reference Phase	2		ľ ↑	\exists							1	2	3	
Offset, s	25	Reference Point	Begin	Green	54.1	14.5	0.0	0.0	0	0.0	0.0					
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0	0	0.0	0.0			1		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.7	2.3	0.0	0.0	0	0.0	0.0	-	5	6	7	8
					_	EDT							NDT	0.01		0.D.T.
Timer Results				EBL		EBI	WB		W	BI	NBI	-	NBI	SBL	-	SBI
Assigned Phase	e					4	<u> </u>	\rightarrow	5	8			6			2
Case Number						7.0		_	6.	.0			8.0			7.0
Phase Duration	i, S				_	20.0		\rightarrow	20	0.0			60.0			50.0
Change Period,	Period, (Y+R c), s w Headway (<i>MAH</i>), s				_	5.5			5.	.5			5.9		_	5.9
Max Allow Head	x Allow Headway (<i>MAH</i>), s eue Clearance Time (<i>q</i> _s), s				+	4.2			4.	.2			0.0			0.0
Queue Clearan	ueue Clearance Time (g_s), s				+	0.0	<u> </u>	-	0.	.0			0.0			0.0
Phase Call Pro	hahility	(ge), s			+	1.00		+	1 (.00			0.0			0.0
Max Out Proba	bility					0.04		-	0.0	07						
Max Out 100a	onity					0.04			0.0	01						
Movement Gro	oup Res	ults			EB			WE	3			NB			SB	
Approach Move	ement			L	Т	R	L	Т		R	L	Т	R	L	Т	R
Assigned Move	ment				4	14	3	8				6		5	2	12
Adjusted Flow F	Rate (<i>v</i>), veh/h			88	47	10	32				0		619	571	50
Adjusted Satura	ation Flo	ow Rate (<i>s</i>), veh/h/l	In		1678	1422	1178	178	9			1662		1688	1564	1456
Queue Service	Time (g	g s), s			4.0	2.5	0.7	1.3	3			0.0		0.0	11.1	0.6
Cycle Queue C	learance	e Time (<i>g c</i>), s			4.0	2.5	4.6	1.3	3			0.0		10.9	11.1	0.6
Green Ratio (g	/C)				0.18	0.18	0.18	0.18	8			0.68		0.68	0.68	0.68
Capacity (c), v	/eh/h				304	258	245	324	1			1124		1190	1057	985
Volume-to-Capa	acity Ra	tio (<i>X</i>)			0.289	0.181	0.040	0.09	97			0.000		0.520	0.540	0.051
Back of Queue	(Q), ft	In (95 th percentile	e)		82.7	43.2	9.3	27.	7			0		132.3	125.9	8
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		3.3	1.7	0.4	1.1		_		0.0		5.2	5.0	0.3
Queue Storage	Ratio (RQ) (95 th percent	tile)		0.00	0.54	0.00	0.00	0	-		0.00		0.00	0.00	0.16
Uniform Delay ((a1), s/	ven			31.9	31.3	34.1	30.	/ \	-		0.0		4.0	4.1	2.9
Incremental Delay (<i>d</i> ₂), s/veh					2.4	1.5	0.3	0.6) \	-		0.0		1.5	1.8	0.1
Initial Queue Delay (d 3), s/veh					0.0	0.0	0.0	0.0	2			0.0		0.0	0.0	0.0
Control Delay (<i>d</i>), s/veh					34.3	32.8	34.4	31.	3			0.0		0.5	5.9 ^	3.0
Approach Delay, s/yeh / LOS				33.0			32.4			_	0.0			A 5.6	A	A
Approach Delay, s/veh / LOS				55.0		0	0			5	0.0			Δ		~
Intersection Delay, s/ven / LOS						9										
Multimodal Results					EB			WE	3			NB			SB	
Pedestrian LOS	Pedestrian LOS Score / LOS					В	2.12	2	E	3	1.85	5	В	1.85		В
Bicycle LOS Sc	edestrian LOS Score / LOS cycle LOS Score / LOS					В	1.63	3	E	3	1.56	;	В	2.57	·	С

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General Inform	nation								Int	tersect	ion Inf	ormatio	on	4	at at at a t	<u>ه لړ</u>
Agency									Du	ration.	h	0.250)		.↓↓↓ /*	
Analyst				Analys	is Date	- 4/12/2	023		Are	ea Type	ر ۱۰	CBD		 		<u>ار</u> لک
Jurisdiction				Time F	Period	5 1/12/2	.020		PH	HE	,	0.95		-→ 	w‡e	← ∲-
Urban Street		Madison		Analys	is Yea	r 2023			An	n Nalvsis F	Period	1> 8.0	າດ			→ ¥ *
Intersection		Madison & Washing	nton	File Na	ame	2 Jane	- Madis	son 2	024	PM S		1. 0.		- <u>-</u>	•	<u>r</u>
Project Descrip	tion	2024 PM	gion	1 110 110			maan	2011	021					1	* 1 * * * 1	- r
	aon															
Demand Inform	nation				EB			N	/B			NB			SB	
Approach Move	ement			L	Т	R	L	-	Г	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				126	8	59	22	28			0		58	1011	40
				11												
Signal Informa	ition			-	₽ ₩3	5										
Cycle, s	80.0	Reference Phase	2		ľ ↑	B							1	2	3	
Offset, s	0	Reference Point	Begin	Green	55.0	14.0	0.0	0.0	0	0.0	0.0					
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0	0	0.0	0.0			1		Y
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.4	2.2	0.0	0.0	0	0.0	0.0		5	6	7	8
Timer Describe			_		_	EDT		_	14			_	NDT	0.01		ODT
Timer Results				EBL		EBI	VVB		VV	VRI	NBI		NBI	SBL	-	SBI
Assigned Phase	e				+	4	<u> </u>	\rightarrow	-	8		_	6			2
Case Number					_	8.0		\rightarrow	6	0.0			8.0			5.0
Phase Duration	, s			<u> </u>		19.4		\rightarrow	1	9.4			60.6	<u> </u>	_	50.6
Max Allow Hear	, (Y+Ra dway (A	(Y+R c), s way (MAH), s			-	5.4 1 2	<u> </u>	+	с л	0.4 1.2			0.0	<u> </u>		0.0
Queue Clearan	Allow Headway (<i>MAH</i>), s ue Clearance Time (<i>g</i> s), s				+	8.0	<u> </u>	+		+. <u>2</u> 22			0.0			0.0
Green Extensio	n Time	(g, s), s				1.5		-	1	1.4			0.0			0.0
Phase Call Pro	bability	(9,0),0		<u> </u>		1.00		\rightarrow	1.	.00			0.0	<u> </u>		0.0
Max Out Proba	bility					0.00			0.	.01						
-	,								-							
Movement Gro	oup Res	ults			EB			WE	3			NB			SB	
Approach Move	ement			L	Т	R	L	Т		R	L	Т	R	L	Т	R
Assigned Move	ment				4	14	3	8				6		5	2	12
Adjusted Flow F	Rate (v), veh/h			141		62	240	2			0		66	1158	46
Adjusted Satura	ation Flo	w Rate (s), veh/h/l	In		1682		1118	186	1			1689		1603	1670	1506
Queue Service	Time (g	g s), S			6.0		4.2	9.8	3			0.0		1.1	13.2	0.8
Cycle Queue C	learance	e Time (<i>g c</i>), s			6.0		10.2	9.8	3			0.0		1.1	13.2	0.8
Green Ratio (g	/C)				0.18		0.18	0.1	8			0.69		0.69	0.69	0.69
Capacity (c), v	/eh/h				295		202	326	6			1160		1192	2294	1035
Volume-to-Capa	acity Ra	tio (X)			0.478		0.308	0.73	35	_		0.000		0.056	0.505	0.044
Back of Queue	(Q), ft	/In (195 th percentile	e)		112.5		52.5	182	.1			0		13.8	164.4	9.3
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		4.4	<u> </u>	2.1	7.2	2			0.0		0.5	6.5	0.4
Queue Storage	Ratio (RQ) (95 th percent	tile)		0.00		0.81	0.0	0	_		0.00		0.09	0.00	0.00
Uniform Delay ((01), S/	ven			29.5		34.1	31.	0	_		0.0		4.0	5.9	4.0
Incremental De	ncremental Delay (<i>d</i> ₂), s/veh				1.2		0.5	2.1		_		0.0		0.1	0.6	0.1
nitial Queue Delay (d 3), s/veh					0.0		0.0	0.0	1			0.0		0.0	0.0	0.0
Control Delay (d), s/veh					30.7		34.7	33.	1			0.0		4.1	0.5	4.1
Level of Service (LOS) Approach Delay, s/yeb / LOS				20.7		<u> </u>				C	0.0			A	A	A
Approach Delay, s/veh / LOS				30.7		0	33.4 2 1	ŀ		U	0.0			0.3 D		A
Intersection Delay, s/ven / LOS						L.). I									
Multimodal Re	Multimodal Results				EB			WE	3			NB			SB	
Pedestrian LOS	edestrian LOS Score / LOS					В	2.29)		В	1.85	5	В	1.63	-	В
Bicycle LOS Sc	estrian LOS Score / LOS cle LOS Score / LOS					В	2.06	6		В	1.56	;	В	2.52	2	С

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HCS[™] Streets Version 2023

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General Inform	nation								Intersec	tion Info	ormatio	on	4	****	× L
Agency									Duration	. h	0.250)		Ļ	
Analyst				Analys	is Date	4/12/2	023		Area Tvr)e	CBD		 		۲. ۲.
Jurisdiction				Time F	Period				PHF		0.85		*	w∔e	 <!--</td-->
Urban Street		Fast		Analys	is Year	2023			Analysis	Period	1> 8:0	00	*		+ *
Intersection		East & Front		File Na	ame	2 lane	- East 2	2044	AM S.xus		1			st te	<u>_</u>
Project Descrip	tion	2044 AM											n n	414Y1	- (*
, ,		-													
Demand Inform	nation				EB			W	'B		NB			SB	
Approach Move	ement			L	Т	R	L	٦	R	L	Т	R	L	Т	R
Demand (v), v	eh/h			15	33			14	41 40	45	999	18		0	
				1						_					
Signal Informa	tion	r	1		↓	. 3 4	=						-+-		
Cycle, s	80.0	Reference Phase	2		51	•						1		3	→ 4
Offset, s	60	Reference Point	Begin	Green	52.4	16.8	0.0	0.0	0.0	0.0					5
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0	0.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.4	2.0	0.0	0.0	0.0	0.0		5	6	7	8
Timer Deculto			_	EDI	_	EDT		1		NDI	_	NDT	CDI	_	ODT
Accident Accident				EBL			VVB		0	INBL	·		SBL	· ·	SB1
Assigned Phase	3			<u> </u>		4	<u> </u>	-	0	<u> </u>		2		_	0
Phase Duration	s					0.0 22.0			22.0		-	0.0 58.0			0.0 58.0
Change Period	, 3 . (Y+R (c). S				5.2			5.2			5.6			5.6
Max Allow Head	ange Penod, (Y+R c), s ax Allow Headway (<i>MAH</i>), s					4.2			4.2			0.0			0.0
Queue Clearan	ce Time	(gs), s				10.2			9.0						
Green Extensio	n Time	(ge), s				0.5			0.6			0.0			0.0
Phase Call Pro	bability					1.00			1.00						
Max Out Proba	bility					0.25			0.13						
Movement Gro	oup Res	ults			FB			WF	3		NB			SB	
Approach Move	ement			L	T	R	L	Т	R	L	T	R	L	T	R
Assigned Move	ment			7	4			8	18	5	2	12		6	
Adjusted Flow I	Rate (v), veh/h		18	39			166	6 47	629		576		0	
Adjusted Satura	ation Flo	w Rate (s), veh/h/	In	1071	1759			165	7 1374	1663		1529		1635	
Queue Service	Time (g	g s), S		1.2	1.4			7.0	2.2	0.0		16.8		0.0	
Cycle Queue C	learance	e Time (<i>g</i> c), s		8.2	1.4			7.0	2.2	16.5		16.8		0.0	
Green Ratio (g	/C)			0.21	0.21			0.2	1 0.21	0.66		0.66		0.66	
Capacity (c), v	/eh/h			221	369			348	3 289	1138		1002		1071	
Volume-to-Cap	acity Ra	tio(X)		0.080	0.105			0.47	7 0.163	0.553		0.575		0.000	
Back of Queue	(Q), ft	/In (95 th percentile	e)	15.9	29.7			129.	9 34.1	224.8		213.7		0	
Back of Queue	(Q), ve	eh/In (95 th percent	ile)	0.6	1.2			5.0	1.3	9.0		8.5		0.0	
Queue Storage	Ratio (RQ) (95 th percen	tile)	0.18	0.00			0.00	0.34	0.00		0.00		0.00	
Uniform Delay	(d 1), si	/veh		31.3	25.5			27.7	7 25.8	7.7		7.8		0.0	
Incremental De	lay (<i>d</i> 2), s/veh		0.7	0.6			1.0	0.3	1.7		2.1		0.0	
Initial Queue Delay (d 3), s/veh				0.0	0.0			0.0	0.0	0.0		0.0		0.0	
Control Delay (<i>d</i>), s/veh				32.1	26.1			28.8	3 26.1	9.4		9.9		0.0	
Level of Service (LOS)				С	С			С	С	A		A			
Approach Delay		28.0		С	28.2	2	С	9.6		А	0.0				
Intersection Delay, s/veh / LOS						13	3.0						В		
Multimedal De	oulto				EB				>		ND			CD	
Pedestrian LOS	edestrian LOS Score / LOS					B	1.04		, R	1 96		B	1 97	30	B
Ricycle I OS So				1.93		B	1.94		B	2.50		C	1.07		B
		<i>,</i> ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		1.00		D	1.31		D	2.09		0	1.55		D

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HCS[™] Streets Version 2023

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General Inform	ation								Intersec	tion Info	ormatio	on	4	4 24 44 1 1	× L
Agency									Duration	. h	0.250)		Ļ	
Analyst				Analys	is Date	e 4/12/2	023		Area Tyr)e	CBD		 		۲. ۲.
Jurisdiction				Time P	Period				PHF		0.83		→ 	w∔e	
Urban Street		Fast		Analys	is Yea	r 2023			Analysis	Period	1> 8:0	00	*		+ ∵
Intersection		East & Jefferson		File Na	me	2 lane	- East 2	2044	AM S.xus		1			st te	<u>_</u>
Project Descripti	ion	2044 AM											1	114Y1	- (*
, ,		-													
Demand Inform	ation				EB			W	В		NB			SB	
Approach Mover	ment			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Demand (v), ve	eh/h			7	26			12	2 4	23	1047	7 29		0	
				In						_			_		
Signal Informat	tion				↓		-						-+-		
Cycle, s	80.0	Reference Phase	2		5th	,						1		3	→ 4
Offset, s	55	Reference Point	Begin	Green	57.8	11.8	0.0	0.0	0.0	0.0			•		5
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0	0.0	0.0			-		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.0	2.0	0.0	0.0	0.0	0.0		5	6	7	8
T . D			_	EDI		EDT			MOT	NIDI		NDT	0.01		ODT
Timer Results				EBL		EBI	WBI			NBL		NBI	SBL	-	SBI
Assigned Phase	;					4		+	8	<u> </u>	_	2		_	6
Case Number	_					8.0			8.0			8.0			8.0
Change Duration,	S (VID					5.2			F 2	<u> </u>		5.0			5.0
Max Allow Head	ange Period,(Y+ <i>R c</i>), s ax Allow Headway(<i>MAH</i>), s				-	5.2 4 1			5.2 4 1			5.Z			5.Z
Queue Clearanc	e Time	(q_s) s				3.6		-	2.8			0.0			0.0
Green Extension	n Time ((g_{e}) s				0.0			0.1			0.0			0.0
Phase Call Prob	ability	(9°), 0				0.73			1.00			0.0			0.0
Max Out Probab	oility					0.01			0.00						_
)														
Movement Grou	up Res	ults			EB			WB	3		NB			SB	
Approach Mover	ment			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Assigned Moven	nent			7	4			8	18	5	2	12		6	
Adjusted Flow R	late (v), veh/h			40			19		643		582		0	
Adjusted Satural	tion Flo	w Rate (<i>s</i>), veh/h/l	In		1705			1598	3	1660		1504		1622	
Queue Service 7	Time (g	1 s), S			0.0			0.8		0.0		14.1		0.0	
Cycle Queue Cle	earance	e Time (<i>g c</i>), s			1.6			0.8		14.0		14.1		0.0	
Green Ratio (g/	(C)				0.15			0.15	5	0.72		0.72		0.72	
Capacity (<i>c</i>), ve	eh/h				306			236	;	1246		1087		1172	
Volume-to-Capa	city Ra	tio (X)			0.130			0.08	2	0.516		0.536		0.000	
Back of Queue (Q), ft	/In (195 th percentile	e)		30.9			16.5	5	161.6		152		0	
Back of Queue ((Q), ve	h/ln (95 th percent	ile)		1.2			0.6		6.5		6.1		0.0	
Queue Storage I	Ratio (RQ) (95 th percen	tile)		0.00			0.00)	0.00		0.00		0.00	
Uniform Delay (d 1), s/	veh			29.7			29.4	L	5.1		5.1		0.0	
Incremental Dela	ay (<i>d</i> 2), s/veh			0.2			0.7		1.0		1.3		0.0	
Initial Queue Delay (d ₃), s/veh					0.0			0.0		0.0		0.0		0.0	
Control Delay (d), s/veh					29.9			30.1		6.1		6.4		0.0	
Level of Service			C			C		A		A					
Approach Delay,		29.9		U _	30.1		C	6.2		A	0.0				
Intersection Delay, s/veh / LOS						1	.3						4 		
Multimodal Res	Aultimodal Results				EB			WB	3		NB			SB	
Pedestrian LOS	edestrian LOS Score / LOS					В	1.94	L L	В	1.62		В	1.62		В
Bicycle LOS Sco	ore / LC	S		1.62		В	1.59)	В	2.65		С	1.55	,	В

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Agency	lation	1									ration	h	0 250	,,,,				
Apolyet				Analys	ie Dat	o 1/12	120	123		Δr.			CBD		 			۲. 4
Analyst				Time	Dariad		/20	125			еатуре че	5	0.07		$\rightarrow \xrightarrow{A}$	w‡e		} -
Jurisdiction		Foot				r 2022	>					Dariad	1 0.07	0				1.
Urban Street				Analys	as rea	0.100) 	Fact	044			Period	12 0.0	0				_
Intersection	tian			File Na	ame	Ziar	ie -	- Easi 2	2044	AIVI	IN.XUS				- 5			
Project Descrip	lion	2044 AM																
Demand Inform	nation				EB				V	∕B			NB			SE	,	
Approach Move	ement			L	Т	R		L		Т	R	L	Т	R	L	Т		R
Demand (v), v	eh/h			92	528	;			\square		1		931	103				
Signal Informa	tion							1										
Cycle, s	70.0	Reference Phase	2		1	,⊨⊂								4	V			4 _
Offset, s	23	Reference Point	Begin	Green	44 9	14 1		0.0	0	0	0.0	0.0	_	1	2			4
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	·	0.0	0.	0	0.0	0.0						
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.3	2.3		0.0	0.	0	0.0	0.0		5	6	7		8
Timer Results				EBL	-	EBT		WBI	-	V	VBT	NBL	-	NBT	SBL	_	SE	BT
Assigned Phas	e					4								2				
Case Number						12.0								8.0				
Phase Duration	i, s					19.6								50.4				
Change Period	ge Period, (Y+ <i>R</i> c), s Allow Headway (<i>MAH</i>), s					5.5								5.5				
Max Allow Head	Allow Headway (<i>MAH</i>), s					4.1								0.0				
Queue Clearan	x Allow Headway (<i>MAH</i>), s eue Clearance Time (<i>g</i> s), s					12.1												
Green Extensio	n Time	(ge),s				2.0	Τ							0.0				
Phase Call Pro	bability					1.00	Т											
Max Out Proba	bility					0.27												
	_						-			_								
Movement Gro	oup Res	sults			EB		+		W	B	-		NB	_		SB		_
Approach Move	ement			L	Т	R	4	L	T	4	R	L	T	R	L	T	+	R
Assigned Move	ment			7	4		4			_			2	12			+	
Adjusted Flow I	Rate (v), veh/h		250	462		4			4			630	609			4	
Adjusted Satura	ation Flo	ow Rate (s), veh/h/l	In	1622	1657		4			_			1657	1598			+	
Queue Service	Time (g	g s), S		10.1	9.0		4			4			18.1	16.1			_	
Cycle Queue C	learance	e Time (<i>g c</i>), s		10.1	9.0		4			4			18.1	16.1			+	
Green Ratio (g	/C)			0.20	0.20		4			_			0.64	0.64			_	
Capacity (c), v	/eh/h			327	668		4			\rightarrow			1062	1025			+	
Volume-to-Cap	acity Ra	itio (X)		0.765	0.692	2	4			_			0.593	0.594			+	
Back of Queue	(Q), ft	In (95 th percentile	e)	184.5	158.9)	4						217.3	206.3			+	
Back of Queue	(Q), ve	eh/In (95 th percent	ile)	7.3	6.2		4			_			8.4	8.3			+	
Queue Storage	Ratio (RQ) (95 th percen	tile)	0.00	0.00		+			\rightarrow			0.00	0.00			+	
Uniform Delay	(d1), s/	/veh		25.7	25.3	_	+			4	_		7.8	7.8			+	
Incremental De	lay (d 2), s/veh		4.4	1.3		+			+			1.9	2.0			+	
Initial Queue Delay (<i>d</i> ₃), s/veh				0.0	0.0		4			_			0.0	0.0			+	
Control Delay (d), s/veh				30.2	26.6								9.7	9.8			-	
Level of Service (LOS)				С	C								A	A				
Approach Delay, s/veh / LOS				27.8		С		0.0				9.8		A	0.0			
Intersection Delay, s/veh / LOS							16.	.4						l	В			
Multimodal Results					ED				10/	D			ND			CD		
Vultimodal Results				1 71		P	╉	1 70		0	B	1 96		B	1 05	38	P	
Ricycle I OS Sc	destrian LOS Score / LOS					R	╉	1.72	-		-	2.54		C	0.00		Δ	
				1.00		0						2.04		~	0.00		~	•

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General Inform	nation								Intersec	tion Inf	ormati	<u></u>		al Late ↓	þa lu
Agency	lation								Duration		0 250)		Ļ	
Analyst				Analys	is Dat	a 1/12/2	023	_			CBD		 		₹
Jurisdiction				Time E	oriod	e 4/12/2	025			56	0.82		→ 	w‡e	≯
Lirban Street		Fast				r 2023			Analysis	Period	1 2 8.	00			↓ ₹
Intersection		East & Market		File Nr		2 2 2	Fact	2044			12 0.	00	- E		
Project Descrip	tion						- Lasi 2	2044		5			- 5	া বা শক্ষাপ	te d
T Toject Descrip	lion	2044 AM													
Demand Inform	nation				EB			W	/B		NB			SB	
Approach Move	ement			L	Т	R	L	1 -	Г R	L	Т	R	L	Т	R
Demand (v), v	reh/h			65	0					88	951			0	
				1						_					
Signal Informa	tion			e	+								-		
Cycle, s	70.0	Reference Phase	2		5 T	\rightarrow						1	2	3	4
Offset, s	4	Reference Point	Begin	Green	48.5	10.6	0.0	0.0	0.0	0.0					
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0	0.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.3	2.2	0.0	0.0	0.0	0.0		5	6	7	8
				501	_	EDT						NET	0.01	_	0.5.7
Timer Results				EBL		EBI	WBI		WBI	NBI		NBI	SBI		SBI
Assigned Phase	e				-	4	<u> </u>	\rightarrow		<u> </u>		2			6
Case Number						12.0		\rightarrow			_	8.0			8.0
Change Duration						16.0		\rightarrow				54.0			54.0
Max Allow Hoa	Ange Period, (<i>Y+R c</i>), s (Allow Headway (<i>MAH</i>), s					0.4 4.2		+				0.0			5.5
	x Allow Headway (<i>MAH</i>), s eue Clearance Time (<i>q</i> s), s				-	4.Z		+		<u> </u>		0.0			0.0
Green Extensio	on Time	(q_e) s				0.1		-				0.0			0.0
Phase Call Pro	bability	(90),0		<u> </u>		1.00		+				0.0			0.0
Max Out Proba	bility					0.35									
Movement Gro	oup Res	ults			EB			WE	3		NB			SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Assigned Move	ment			7	4					5	2			6	
Adjusted Flow I	Rate (v), veh/h			79				_	654	613			0	
Adjusted Satura	ation Flo	w Rate (s), veh/h/l	In		1633				_	1521	1443			1825	
Queue Service	Time (g	g s), S			3.4				_	0.0	14.9			0.0	
Cycle Queue C		e Time (<i>g c</i>), s			3.4					5.1	14.9			0.0	
Green Ratio (g	<i>//C</i>)				0.15				_	0.69	0.69	<u> </u>		0.69	
Capacity (c), V	/en/n	tic (N)			247					1113	999			1264	
Volume-to-Capa	$\frac{1}{(0)}$	$\frac{100}{100}$	<u>,</u>		0.321				_	0.588	0.013			0.000	
Back of Queue	$(Q), \pi$	ph/ln (95 th percent	ila)		2.3					23	21			0.0	
Queue Storage	Ratio (RO) (95 th percent	tile)		0.00				_	0.00	0.00			0.0	
Uniform Delay	(d1), s	/veh			31.4					1.1	1.1			0.0	
Incremental De	lav (<i>d</i> 2). s/veh			3.3					2.3	2.8			0.0	
Initial Queue De	Incremental Delay (d ₂), s/veh				0.0					0.0	0.0			0.0	
Control Delay (<i>d</i>), s/veh					34.7					3.3	3.9			0.0	
Level of Service (LOS)					С					Α	A				
Approach Delay, s/veh / LOS				34.7	·	С	0.0			3.6		A	0.0		
Intersection Delay, s/veh / LOS						5	.4						A		
Multimodal Re	Multimodal Results				EB			WE	3		NB			SB	
Pedestrian LOS	edestrian LOS Score / LOS					В	1.95	5	В	1.33	3	Α	1.33	3	A
Bicycle LOS So	constrian LOS Score / LOS				6	В				2.60)	С	1.55	5	В

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General Inform	nation								Interse	ction In	forma	tion		14444	lte lu
Agency	lation								Duratio	n h	0.2	50		Ļ	
Analyst				Analys	is Date	4/12/2	023			(ne	CB	<u>ככ</u>			<u>₹</u>
lurisdiction				Time F	Period	, , , , , , , , , , , , , , , , , , , ,	.020		PHE	pe	0.8	2	→ ∻4	w‡e	2- 4-
Lirban Street		Fast			is Vea	2023			Analysi	s Perior		- 8∙00	*		+ *
Intersection		East & Monroe			ame	2020 2 Jane	- Fast (2011				5.00			
Project Descrip	tion	2044 AM					- Last 2	2044		3				- ¶] 1 ≼ ↑ ↔ ∿	141
Troject Descrip		2044710													
Demand Inform	nation				EB			N	/B		N	В		SB	
Approach Move	ement			L	Т	R	L	-	T R	L	1	- R	L	Т	R
Demand (v), v	eh/h			3	0					31	10	21		0	
Signal Informa	ition	1			↓	2							_		_
Cycle, s	80.0	Reference Phase	2		5 T	F						1		3	-4
Offset, s	61	Reference Point	Begin	Green	58.8	10.8	0.0	0.0	0 0.0	0.0)				
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.	0 0.0) 0.0)		↓ ∟		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.0	2.0	0.0	0.	0 0.0	0.0)	5	6	7	8
			_	1				. 1		1					
Timer Results				EBL		EBT	WBI		WBT	NE	BL	NBT	SB		SBT
Assigned Phase	e				\rightarrow	4		\rightarrow		-	_	2			6
Case Number					\rightarrow	12.0		\rightarrow		-		8.0			8.0
Phase Duration	i, s	```			\rightarrow	16.0		\rightarrow		-	_	64.0			64.0
Change Period	ange Period,(Y+ <i>R</i> c), s x Allow Headway(<i>MAH</i>), s				\rightarrow	5.2		\rightarrow		-		5.2			5.2
Max Allow Head	x Allow Headway (<i>MAH</i>), s				\rightarrow	4.2		\rightarrow		-		0.0			0.0
Queue Clearan		e (g s), s				2.2	<u> </u>	\rightarrow		-		0.0			0.0
Green Extensio	n lime	(ge), s			\rightarrow	0.0		\rightarrow		-	\rightarrow	0.0		-+	0.0
Phase Call Pro						1.00		\rightarrow		-					
Max Out Proba	DIIITY					0.00									
Movement Gro	oup Res	ults			EB			W	3		NE	3		SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Assigned Move	ment			7	4					5	2			6	
Adjusted Flow I	Rate (v), veh/h			4					616	565	5	<u> </u>	0	
Adjusted Satura	ation Flo	w Rate (<i>s</i>), veh/h/l	In		1657					1758	161	6		1599	
Queue Service	Time (g	g s), s			0.2					0.0	12.2	2		0.0	
Cycle Queue C	learance	e Time (<i>g c</i>), s			0.2					3.0	12.2	2		0.0	
Green Ratio (g	v/C)				0.13					0.74	0.74	1		0.74	
Capacity (c), v	/eh/h				224					1340	118	8		1175	
Volume-to-Cap	acity Ra	itio(X)			0.016					0.460	0.47	5		0.000	
Back of Queue	(Q), ft	/In (95 th percentile	e)		3.6					34.9	34.9	9		0	
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		0.1					1.4	1.4			0.0	
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00					0.00	0.00)		0.00	
Uniform Delay	(d 1), si	/veh			34.7					0.7	0.7			0.0	
Incremental De	lay (<i>d</i> 2), s/veh			0.1					0.9	1.0			0.0	
Initial Queue Delay (d 3), s/veh					0.0					0.0	0.0			0.0	
Control Delay (<i>d</i>), s/veh					34.8					1.6	1.8			0.0	
Level of Service (LOS)					С					Α	A				
Approach Delay, s/veh / LOS				34.8	3	С	0.0			1.	7	А	0.0)	
Intersection Delay, s/veh / LOS						1	.8						А		
Multimodal Re	Aultimodal Results				EB	_		W	5		NE	3		SB	
Pedestrian LOS	Score	/ LOS		1.95	,	В	1.96)	В	1.3	52	A	1.3	<u>კ</u>	A
BICYCLE LOS SC	ore / LC	5		1.56		В				2.6	2	С	1.5	5	В

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Jurisdiction				Time F	Period	0 1/ 12/2	020		PHF		0.92		→ ∻>	w‡e	
Urban Street		Fast		Analys	is Yea	r 2023			Analysis	Period	1> 8.	00			+ ₽
Intersection		East & Olive		File Na	ame	2 lane	- Fast 2	2044	AM S.xus	1 onou				.t t.	<u>~</u>
Project Description	on	2044 AM		1 110 110									- I	<u>ነ</u> 1414ም1	* (*
	•														
Demand Information	ation				EB			W	'B		NB			SB	
Approach Moven	nent			L	Т	R	L	Г	R	L	Т	R	L	Т	R
Demand (v), ve	h/h			58	52			1	7 26	31	1025	5 11		0	
				li-											
Signal Informati	ion		r		 	3 4	-						-+-		-
Cycle, s	80.0	Reference Phase	2		51	∼ ⊖⊂						1		3	4
Offset, s	55	Reference Point	Begin	Green	56.4	12.5	0.0	0.0	0.0	0.0					5
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0	0.0	0.0			-		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.4	2.3	0.0	0.0	0.0	0.0	_	5	6	7	8
				= = =	_	EDT			MOT			NET	0.01		0.D.T.
Timer Results				EBL	-	EBI	WB		WBI	NBL		NBI	SBL	-	SBT
Assigned Phase					\rightarrow	4	<u> </u>	_	8			2			6
Case Number					+	8.0		-	8.0		_	8.0			8.0
Phase Duration,	S	\ -			\rightarrow	18.0		-	18.0		_	62.0			o2.0
Change Period, (nge Period, (Y+R c), s Allow Headway (<i>MAH</i>), s					5.5		_	5.5	<u> </u>	_	5.0		_	5.0
	ax Allow Headway (MAH), s				+	4.2 7.9	<u> </u>	+	3.2		+	0.0			0.0
Green Extension	Time ((g_e) , s			-	0.2			0.3			0.0			0.0
Phase Call Proba	ability	(y °), o			+	1.00	<u> </u>	-	0.98			0.0			0.0
Max Out Probabi	ilitv		_			0.71		-	0.02						
						011 1			0.02						
Movement Grou	ıp Res	ults			EB			WE	3		NB			SB	
Approach Moven	nent			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Assigned Mover	nent			7	4			8	18	5	2	12		6	
Adjusted Flow Ra	ate (v), veh/h			120			18	28	606		553		0	
Adjusted Saturat	ion Flo	w Rate (<i>s</i>), veh/h/l	In		1447			178	4 1607	1544		1413		1825	
Queue Service T	ïme (g	1 s), S			4.1			0.7	1.2	0.0		15.4		0.0	
Cycle Queue Cle	earance	e Time (<i>g c</i>), s			5.9			0.7	1.2	15.2		15.4		0.0	
Green Ratio (g/0	C)				0.16			0.16	6 0.16	0.70		0.70		0.70	
Capacity (c), ve	eh/h				295			279	251	1136		996		1286	
Volume-to-Capac	city Ra	tio (X)			0.405			0.06	6 0.113	0.534		0.556		0.000	
Back of Queue (Q), ft	/In (195 th percentile	e)		111.5			14.2	2 21.5	192.6		181.9		0	
Back of Queue (Q), ve	eh/In (95 th percent	ile)		4.3			0.5	0.8	7.7		7.3		0.0	
Queue Storage F	≺atio (RQ) (95 th percen	tile)		0.00			0.00	0.00	0.00		0.00		0.00	
Uniform Delay (d 1), s/	veh			30.9			28.8	3 29.0	5.8		5.8		0.0	
Incremental Dela	ncremental Delay (d ₂), s/veh				4.1			0.1	0.2	1.8		2.2		0.0	
Initial Queue Delay (d ₃), s/veh					0.0			0.0	0.0	0.0		0.0		0.0	
Control Delay (d), s/veh					34.9			28.9	9 29.2	7.6		8.1		0.0	
Level of Service (LOS) Approach Delay, s/yeb / LOS				24.0	C	<u> </u>	20.4		C	A		A	0.0		
Approach Delay, s/veh / LOS				34.9		0	29.1		U	7.8		A	0.0		
Intersection Delay, s/ven / LOS						11	.0						د 		
Multimodal Res	Nultimodal Results				EB			WE	3		NB			SB	
Pedestrian LOS	edestrian LOS Score / LOS					В	1.94	F T	В	1.85		В	1.85		В
Bicycle LOS Sco	estrian LOS Score / LOS ′cle LOS Score / LOS					В	1.60)	В	2.52		С	1.55	,	В

5							,					
General Information						Intersec	tion Inf	ormatio	on	4	***	he l <u>u</u>
Agency						Duration.	, h	0.250			Ļ	
Analyst	Analys	is Date	4/12/2	023	_	Area Typ	e	CBD				1 A
Jurisdiction	Time F	Period				PHF		0.80			w + E	
Urban Street East	Analys	is Year	2023		_	Analysis	Period	1> 8:0	00			+ *
Intersection East & Washington	File Na	ame	2 lane	- East 2	2044	AM S.xus					<u> </u>	
Project Description 2044 AM			R							1	* 1 4 17 1	۲ <u>۲</u>
						-						
Demand Information	<u> </u>	EB		<u> </u>	W	/B	<u> </u>	NB			SB	
Approach Movement		1	R	L .			L		R			R
Demand (V), ven/n	11	189			26	51 85	26	993	58		0	
Signal Information			7									
Cvcle. s 80.0 Reference Phase 2	1		<u></u> ≉ ∈							*		<u> </u>
Offset, s 64 Reference Point Begin	1							_	1	2	3	4
Uncoordinated No Simult. Gap E/W On	Green	48.0	20.6	0.0	0.0		0.0	-				₩ A
Force Mode Fixed Simult. Gap N/S On	Red	2.8	2.2	0.0	0.0	0.0	0.0		5	6	7	8
											I	
Timer Results	EBL	.	EBT	WB	_	WBT	NBI	-	NBT	SBL		SBT
Assigned Phase			4			8			2			6
Case Number			6.0			7.0			5.0			8.0
Phase Duration, s			26.0			26.0			54.0			54.0
Change Period, (Y+R c), s			5.4			5.4			6.0			6.0
Max Allow Headway (MAH), s			4.2			4.2			0.0			0.0
Queue Clearance Time (g_s), s			17.9			16.8						
Green Extension Time (g e), s			0.8			1.1			0.0			0.0
Phase Call Probability			1.00			1.00						
Max Out Probability			1.00			1.00						
Movement Group Results		FB			WF	3		NB			SB	
Approach Movement	L	T	R	L	Т	R	L	Т	R	L	T	R
Assigned Movement	7	4			8	18	5	2	12		6	
Adjusted Flow Rate (v), veh/h	14	236			326	3 106	29	1105	65		0	
Adjusted Saturation Flow Rate (s), veh/h/ln	933	1762			163	5 1378	1578	1678	1537		1678	
Queue Service Time (g_s), s	1.1	9.2			14.8	8 5.0	0.6	15.7	1.4		0.0	
Cycle Queue Clearance Time (g c), s	15.9	9.2			14.8	8 5.0	0.6	15.7	1.4		0.0	
Green Ratio (g/C)	0.26	0.26			0.20	6 0.26	0.60	0.60	0.60		0.60	
Capacity (<i>c</i>), veh/h	158	454			421	I 355	1037	2013	922		1007	
Volume-to-Capacity Ratio (X)	0.087	0.521			0.77	5 0.299	0.028	0.549	0.070		0.000	
Back of Queue (Q), ft/In (95 th percentile)	13.4	188.3			278	3 74.8	8.8	217.4	19.4		0	
Back of Queue (Q), veh/In (95 th percentile)	0.5	7.3			10.8	8 2.9	0.3	8.4	0.8		0.0	
Queue Storage Ratio (RQ) (95 th percentile)	0.21	0.00			0.0	0.93	0.11	0.00	0.00		0.00	
Uniform Delay (d 1), s/veh	34.9	25.5			27.	5 23.9	6.5	9.5	6.7		0.0	
Incremental Delay (<i>d</i> ₂), s/veh	0.9	3.7			8.8	0.5	0.0	0.8	0.1		0.0	
Initial Queue Delay (d ȝ), s/veh	0.0	0.0			0.0	0.0	0.0	0.0	0.0		0.0	
Control Delay (<i>d</i>), s/veh	35.9	29.2			36.3	3 24.4	6.6	10.4	6.8		0.0	
Level of Service (LOS)	D	С			D	С	Α	В	Α			
Approach Delay, s/veh / LOS	29.5		С	33.4		С	10.1		В	0.0		
Intersection Delay, s/veh / LOS		_	18	.0						В		
Multimodal Results		FR			\//F	3		NB			SB	
Pedestrian LOS Score / LOS			B	1.03		B	1.87	,	В	1.88		В
	2.28		D	1.30			1.07			1.00		

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General Inform	nation									Inte	ersect	ion Inf	ormati	on	d.	╡Ҳф↓	be l <u>u</u>
Agency	lation									Du	iration	h	0 250)		4 4	
Analyst				Analys	is Da	to 1/1	2/2	023	_	Δre	ea Type	<u>.</u>	CBD		 		۲. ۸
Jurisdiction				Time E			2,2	020			ы п	,	0.75		→ 	w‡e	↓
Urban Street		Madison		Analys		ar 202	2			An	n alveie E	Doriod	1 2 8.	00	4		v - ↓ *
Intersection		Madison & Eront		Filo No		ai 202		Madia	200 2(<u></u>			12 0.	00			F
Breiget Deserie	tion				ame	Zla	me	- Maus		044	AIVI 5.	kus			- 5	<u>।</u> च 1 के स्ट्र	10
Project Descrip	uon	2044 AM															
Demand Inform	nation				EE	3			W	'B			NB			SB	
Approach Move	ement			L	Т	F	र	L	Г	-	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				66	5 1	7	23	3	5			0		64	778	37
Signal Informa	tion				11.	•											
Cycle, s	80.0	Reference Phase	2		P .	► ⊨	È										\rightarrow
Offset, s	4	Reference Point	Begin	Green	54.3		5	0.0		<u>ו</u>	0.0	0.0		1	2	3	Y 4
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	2	0.0	0.0)	0.0	0.0			1		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.5	2.3	}	0.0	0.0)	0.0	0.0		5	6	7	8
Timer Results				EBL	-	EBT		WB	L	W	/BT	NBI	-	NBT	SBL		SBT
Assigned Phase	Э					4				8	8			6			2
Case Number						8.0				6	6.0			8.0			8.0
Phase Duration	, S					20.0				20	0.0			60.0			60.0
Change Period,	(Y+R	c), S				5.5				5	5.5			5.7			5.7
Max Allow Head	Allow Headway (<i>MAH</i>), s					4.2				4	1.2			0.0			0.0
Queue Clearan	Ex Allow Headway (<i>MAH</i>), s leue Clearance Time (g_s), s					7.1				9	9.0						
Green Extensio	n Time	(g _e), s				0.3				0).3			0.0			0.0
Phase Call Prol	bability					1.00				0.	.98						
Max Out Proba	bility					0.11				0.	.42						
May amont Cre	un Dee								\^/	ר ר			ND			00	
Movement Gro	oup Res	SUITS			EB		4			5	Б		NB	D		<u>5</u> B	
Approach Move	ment				1	<u> </u>	4		0	+	ĸ	L	I 6	ĸ	E	ו ר	<u>Γ</u>
Adjusted Flow) yoh/h			4	14	-	0 21	0	+			0		- 5 - 611	2	1Z 562
Adjusted Flow P), ven/n w Rate (s) veh/h/l	In		1531	2	-	1162	47	0			1705		1621		1502
	Time ()	π_{α}) s			5 1	2	-	1 0	1/4	9	-		0.0		35		15 3
	learance	$g(a_{\alpha})$			5.1		-	7.0	1.0				0.0		1/ 9		15.3
Green Ratio (a	\sqrt{C}	c mile (g t), s			0.18	ξ	-	0.18	0.18	2			0.68		0.68		0.68
Capacity (c) y	/eh/h				278		-	226	317	,			1157		1152		1020
Volume-to-Cap	acity Ra	tio (X)			0.39	8	-	0.135	0.14	7			0.000		0.531		0.551
Back of Queue	(Q), ft	In (195 th percentile	e)		100.	7		25.5	35.3	3			0		195.6		184.5
Back of Queue	(Q), ve	eh/In (95 th percent	, ile)		3.9			1.0	1.4				0.0		7.7		7.4
Queue Storage	Ratio (RQ) (95 th percen	, tile)		0.00)		0.43	0.00	5			0.00		0.00		0.00
Uniform Delay ((d1), s	/veh			28.9)		32.0	27.6	3			0.0		6.5		6.6
Incremental De	lay (d 2), s/veh			4.2			0.3	0.2				0.0		1.5		1.8
nitial Queue Delay (<i>d</i> ₂), s/veh					0.0			0.0	0.0				0.0		0.0		0.0
Control Delay (<i>d</i>), s/veh					33.2	2		32.3	27.8	3			0.0		7.9		8.3
Level of Service (LOS)					С			С	С						A		A
Approach Delay, s/veh / LOS				33.2		С		29.6	3	(С	0.0			8.1		Α
Intersection Delay, s/veh / LOS							11	.4							В		
Multimodal Results					EB				WE	3			NB			SB	
Pedestrian LOS	Score	/LOS		1.93		В		1.93	3	E	В	1.85	5	В	1.63		В
Bicycle LOS Sc	ore / LC	DS		1.74		В		1.69)	E	В	1.56	5	В	2.53		С
			- 5									. ,					
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General Inform	nation									Inte	ersecti	ion Inf	ormatio	on	لير	4 <u>↓</u> 40 ↓	la la
Agency										Du	ration.	h	0.250			- ↓ ↓	
Analvst				Analys	is Da	te 4	4/12/20)23	_	Are	ea Type	;	CBD		 		بر ج
Jurisdiction				Time F	Period				_	PH	IF	-	0.76		_→ 	w∔e	<u>}</u> ≁
Urban Street		Madison		Analys	is Yea	ar (2023		_	Ana	alvsis F	Period	1> 8:0	00	- ₹		+ *
Intersection		Madison & Jefferso	n	File Na	ame		2 lane	- Madis	on 20	044	AM S.>	kus				+	
Project Descrip	tion	2044 AM								-					5	ৰ ↑ ক প	* *
, ,																	
Demand Inform	nation				EB	3			W	'B			NB			SB	
Approach Move	ement			L	Т		R	L	ר	Г	R	L	Т	R	L	Т	R
Demand (<i>v</i>), v	eh/h				17	7	4	4	1	3			0		13	903	12
				i	F 113			a 1									
Signal Informa	tion				144	•	÷										
Cycle, s	80.0	Reference Phase	2		ľ	t İ	₹_″							1	2	3	
Offset, s	11	Reference Point	Begin	Green	56.6	3	12.9	0.0	0.0)	0.0	0.0					
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2		3.2	0.0	0.0)	0.0	0.0	_		1		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.2		1.9	0.0	0.0)	0.0	0.0		5	6	7	8
Timer Deculto				ГРІ			рт	\\/DI	_	10/				NDT	<u>edi</u>		OPT
Assigned Phase				EDL	-		4	VVDI	-+-		0 0	INDL	-	6	JDL	-	2
Caso Number	5				-	<u>،</u>	+		+	0				8.0			2
Phase Duration	6					18	.0 2 0		-	18	8.0			62 0			62.0
Change Period	, 3 (V+R)	-) s			+	5	4		-	5	5.0 5.4			54	<u> </u>		5.4
Max Allow Head	Allow Headway (MAH), s					4	.1		+	4	.		-	0.0			0.0
Queue Clearan	x Allow Headway (<i>MAH</i>), s eue Clearance Time (<i>g</i> s), s					3	.2			2	2.8						
Green Extensio	eue Clearance Time (g s), s een Extension Time (g e), s					0	.1			0).1			0.0			0.0
Phase Call Pro	bability					0.	67			1.	.00						
Max Out Proba	bility					0.	00			0.	.00						
Mayamant Cra	un Dee	ulto			ГР	,			\ A /E	2	_		ND			<u>e</u> p	
Approach Move	mont	Suits					P	-		> 	P	1		P	1	<u>эр</u> т	P
Assigned Move	ment				4	+	14	3	8	+			6		5	2	12
Adjusted Flow F	Rate (v) veh/h			28	+			22	+		_	0		640	~	581
Adjusted Satura	ation Flo	w Rate (s) veh/h/	In		1592	2			176	3			1689		1643		1493
Queue Service	Time (a	as). S			1.2				0.0			_	0.0		0.0		14.8
Cvcle Queue C	learance	e Time (<i>q</i> c). s			1.2	+			0.8				0.0		14.8		14.8
Green Ratio (g	/C)				0.16	3			0.16	3			0.71		0.71		0.71
Capacity (c), v	/ /eh/h				251	╈			333	3			1195		1209		1056
Volume-to-Cap	acity Ra	tio(X)			0.110	0			0.06	7			0.000		0.529		0.550
Back of Queue	(Q), ft	/In (95 th percentile	e)		21.1	1			18.1	1			0		195.4		180.6
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		0.8				0.7				0.0		7.7		7.2
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00)			0.00)			0.00		0.00		0.00
Uniform Delay	(d 1), s	/veh			28.8	3			28.7	7			0.0		5.5		5.5
Incremental De	ncremental Delay (<i>d</i> 2), s/veh				0.2				0.4				0.0		1.7		2.1
Initial Queue De	nitial Queue Delay ($d $ 3), s/veh				0.0				0.0				0.0		0.0		0.0
Control Delay (Control Delay (<i>d</i>), s/veh				29.0)			29.0	2			0.0		7.2		7.6
Level of Service	evel of Service (LOS)				С				С						A		Α
Approach Delay	Approach Delay, s/veh / LOS			29.0		(C	29.0		(С	0.0			7.4		Α
Intersection De	ntersection Delay, s/veh / LOS						8.	2							A		
Multimodel De	Iultimodal Posults				ED					2			ND			CD	
Pedestrian LOS	Score	/1.05		1 02			A I	1 02		, .	B	1.60		B	1 62	30	B
Bicycle I OS Sc)S		1.55		F	-	1.60		F	B	1.56		B	2 57		C
, 200 00							-	1.00			-	1.00		-	2.07		-

			5							-	J					
General Inform	nation								Inte	ersect	ion Inf	ormatio	on	4	***	<u>ه لي</u>
Agency									Dur	ration.	h	0.250			7 † †	
Analyst				Analys	is Date	4/12/2	023		Are	a Type	, ,	CBD		 		۲. ۲.
Jurisdiction				Time P	eriod	.,,_			PHI	F		0.82		\rightarrow	w∔e	₹ •
Urban Street		Madison		Analys	is Year	· 2023			Ana	alvsis F	Period	1> 8.0	20	-4 -4		+ ∵
Intersection		Madison & Locust		File Na	me	2 Jane	- Madis	son 2	044					- <u>-</u>	*	<u> </u>
Project Descrip	tion	2044 AM					madic		0117		1010			5	4 1 4 Y 1	* (*
i rojoot Doconp		2011744														
Demand Inform	nation				EB			Ν	/B			NB			SB	
Approach Move	ement			L	Т	R	L	-	Г	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				459	57	4	5	1			0		164	1041	148
				h		_	_	- II								
Signal Informa	tion				₽ ₽₽	- E										—
Cycle, s	70.0	Reference Phase	2		ľ ↑	B							1	2	3	4
Offset, s	10	Reference Point	Begin	Green	45.7	16.3	0.0	0.	0	0.0	0.0					
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.0	3.0	0.0	0.	0	0.0	0.0			1	_	→
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	0.0	0.	0	0.0	0.0		5	6	7	Y 8
T D K			_	EDI		EDT				DT			NDT	0.01		ODT
Timer Results				EBL		EBI	WB	-	VVI	BI	NBI	-	NBT	SBL	-	SBT
Assigned Phase	9			<u> </u>		8			4	4		_	6	<u> </u>	_	2
Case Number						8.0		_	8.	.0			8.0		_	7.0
Phase Duration	, S	`		<u> </u>		20.3	<u> </u>	-	20).3		_	49.7	<u> </u>		49.7
Change Period,	ge Period, (<i>Y+R c</i>), s Allow Headway (<i>MAH</i>), s				_	4.0		\rightarrow	4.	.0			4.0			4.0
Max Allow Head	Allow Headway (<i>MAH</i>), s ue Clearance Time (<i>g</i> s), s				\rightarrow	4.0		\rightarrow	4.	.0		_	0.0		_	0.0
Queue Clearan	eue Clearance Time (g_s), s					14.8		\rightarrow	14	1.9			0.0	<u> </u>	_	0.0
Green Extensio	n lime	(ge), s			+	1.5		\rightarrow	1.	.4		_	0.0			0.0
Phase Call Pro	Dability					1.00		-	1.0	00		_		<u> </u>	_	
Max Out Proba	bility					0.52			0.5	53						
Movement Gro	up Res	ults			EB			W	3			NB			SB	
Approach Move	- ment			L	Т	R	L	Т		R	L	Т	R	L	Т	R
Assigned Move	ment				8	18	7	4	+			6		5	2	12
Adjusted Flow F	Rate (v), veh/h			318	311		67				0		760	709	172
Adjusted Satura	ation Flo	w Rate (s), veh/h/l	In		1657	1616		150	7			1710		1535	1508	1449
Queue Service	Time (g	g s), s			11.6	12.8		0.0)			0.0		21.0	21.4	3.3
Cycle Queue C	learance	e Time (g c), s			11.6	12.8		12.	9			0.0		23.4	21.4	3.3
Green Ratio (g	/C)				0.23	0.23		0.2	3			0.65		0.65	0.65	0.65
Capacity (c), v	/eh/h				386	376		406	3			1116		1067	984	946
Volume-to-Capa	acity Ra	tio(X)			0.824	0.827		0.16	65			0.000		0.713	0.721	0.182
Back of Queue	(Q), ft	/In (95 th percentile	e)		246.3	236.1		41.	2			0		275	267.7	39
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		9.5	9.4		1.6	;			0.0		10.8	10.4	1.6
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00	0.00		0.0	0			0.00		0.00	0.00	0.00
Uniform Delay ((d1), s	/veh			25.5	25.5		22.	9			0.0		8.1	7.8	4.8
Incremental De	cremental Delay (d_2), s/veh				8.6	9.0		0.0)			0.0		4.1	4.6	0.4
Initial Queue De	nitial Queue Delay (<i>d</i> ₃), s/veh				0.0	0.0		0.0)			0.0		0.0	0.0	0.0
Control Delay (<i>d</i>), s/veh					34.1	34.5		22.	9			0.0		12.1	12.3	5.2
Level of Service (LOS)					С	С		С						В	В	Α
Approach Delay, s/veh / LOS				34.3		С	22.9)	C	2	0.0			11.5		В
Intersection De	Intersection Delay, s/veh / LOS					17	7.9							В		
Multimodal Re	Iultimodal Results				EB			W	3			NB			SB	
Pedestrian LOS	Score	/LOS		1.92		В	2.12	2	E	3	1.86	5	В	1.86		В
Bicycle LOS Sc	ore / LC)S		2.08		В	0.60)	A	4	1.56	5	В	2.91		С

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General Inform	nation									In	tersect	ion Inf	ormatio	on	2	≠ **** †	Ja La
Agency	lation										uration	h	0 250			4	
Analyst				Analys	is Da	to 1/1	2/2	023		Δr	rea Type	<u>.</u>	CBD		 		€
Jurisdiction				Time E	Poriod		2,2	020			не турс	,	0.74		→ ∲>	w‡e	
Urban Street		Madison		Analys		ar 202	2				nalveje E	Pariod	1 2 20	20	4 1		
Intersection		Madison & Market		File Nr		ai 202		Madia	on 2				12 0.0	50			e e
Breiget Deserie	tion			File IN	ame	Z 18	me	- Maus		.044	+ AIVI IN.	xus			- 5	ি বা শক্ষাম্য	17 C
Project Descrip	lion	2044 AM															
Demand Inform	nation				EB	3			W	∕B			NB			SB	
Approach Move	ement			L	Т	F	र	L	1-	Т	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				71	8	9	11	8	33	1		0		7	833	93
				0													
Signal Informa	tion		14		14	•											
Cycle, s	70.0	Reference Phase	2		1	ı 🗟	È									2	\rightarrow
Offset, s	28	Reference Point	Begin	Green	46 1	13	∎ 3	0.0	0	0	0.0	0.0	_	1	2	3	M 4
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	2	0.0	0.	0	0.0	0.0			1		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.0	2.2	2	0.0	0.	0	0.0	0.0		5	6	7	8
Timer Results				EBL	-	EBT		WB	L	V	NBT	NBL	-	NBT	SBL		SBT
Assigned Phase	e					4					8			6			2
Case Number						8.0				(6.0			8.0			8.0
Phase Duration	, S					18.7				1	18.7			51.3			51.3
Change Period,	nge Period, (Y+R c), s Allow Headway (<i>MAH</i>), s					5.4				ļ	5.4			5.2			5.2
Max Allow Head	Allow Headway (<i>MAH</i>), s					4.3				4	4.3			0.0			0.0
Queue Clearan	Allow Headway (<i>MAH</i>), s ue Clearance Time (<i>g</i> s), s					11.7				1	12.7						
Green Extensio	n Time	(g e), s				0.7				(0.7			0.0			0.0
Phase Call Prol	bability					1.00				1	1.00						
Max Out Proba	bility					0.27				C).44						
										_							
Movement Gro	oup Res	sults			EB		_			В			NB		<u> </u>	SB	
Approach Move	ement			L	1	R	_	L		_	R	L	1	R			R
Assigned Move	ment	<u> </u>			4	14	-	3	8				6		5	2	12
Adjusted Flow I), ven/n			216	-	4	15	112	2			0		/14		630
Adjusted Satura		w Rate (s), ven/n/l	In		148	2	_	1032	1/2	27			1922		1570		1378
Queue Service	Time (g	js), S			9.7		4	1.0	4.0				0.0		0.0		20.1
Cycle Queue C	learance	e Time (<i>g c</i>), s			9.7	<u> </u>	_	10.7	4.0)			0.0		19.9		20.1
Green Ratio (g	/C)				0.19)	_	0.19	0.1	9			0.66		0.66		0.66
Capacity (c), v	en/n	() () () ()			283	-	_	156	32	9			1266		1086		907
Volume-to-Capa	$\frac{1}{\sqrt{2}}$		\		0.76	5	_	0.095	0.34	11 7			0.000		0.658		0.694
Back of Queue	<u>(Q)</u> , π	Vin (195 th percentile	e)		174.	3	-	11.4	74.	5			0		222.5		204.4
	(Q), ve Ratio (RO) (95 th percent	tile)		0.0	<u> </u>	-	0.4	2.8) 0			0.0		0.0		0.2
Uniform Delay ((d_1)	/veh			27.2	, ,	۲	32.3	24	0 G			0.00		7.4		7.5
Incremental De	niform Delay (d 1), s/veh				57	-		02.0	0.5	5			0.0		22		3.0
Initial Queue De	ncremental Delay (<i>d</i> ₂), s/veh nitial Queue Delay (<i>d</i> ₃), s/veh				0.0		-	0.2	0.0	2			0.0		0.0		0.0
Control Delay (Control Delay (d), s/veh				32.0	2		32.5	25	3			0.0		9.6		10.5
l evel of Service	evel of Service (LOS)				C		-	C.	20. C	-			0.0		Δ		R
Approach Delay	evel of Service (LOS)			32.0		C		26.2			C	0.0			10.0		B
Intersection Del	av s/vo	h/LOS		02.3			14	.2	-	-	-	0.0			B		
	ntersection Delay, s/veh / LOS														_		
Multimodal Re	ultimodal Results				EB				W	В			NB			SB	
Pedestrian LOS	Itimodal Results destrian LOS Score / LOS			1.92	2	В		1.92	2		В	1.85	5	В	1.63		В
Bicycle LOS Sc	ore / LC		1.92	2	В		1.77	7		В	1.56	6	В	2.60		С	

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General Inform	nation							_	Inte	ersect	ion Inf	ormatio	on	4	****	s la
Agency									Du	iration.	h	0.250			7††	
Analyst				Analys	is Date	- 4/12/2	023	_	Are	ea Type	<u>د</u>	CBD		 		<u>ار</u> الح
Jurisdiction				Time F	Period	5 1/12/2	.020		PH	IF	, 	0.92		→ + +	w‡e	← <mark>⊱</mark>
Urban Street		Madison		Analys	is Yea	r 2023		_	An	 alvsis I	Period	1> 8.0	00			+ ¥ +
Intersection		Madison & Olive		File Na	ame	2 Jane	- Madis	on 2	044			1. 0.		- <u>-</u>		<u> </u>
Project Descrip	tion	2044 AM					- Madic		0++	7 (101 0.2	Nuo			-	414Y1	- (*
r rojoot b ooonp		20117.00														
Demand Inform	nation				EB			W	/B			NB			SB	
Approach Move	ement			L	Т	R	L		Г	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				60	37	3	3	7			0		44	726	57
				ii	- 11:			11			-					
Signal Informa	ition		-		₽ ₩3	6										
Cycle, s	80.0	Reference Phase	2		ľ ↑	B							1	2	3	
Offset, s	21	Reference Point	Begin	Green	53.1	15.5	0.0	0.0	0	0.0	0.0					_
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0	0	0.0	0.0			1		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.7	2.3	0.0	0.0	0	0.0	0.0		5	6	7	8
				501	_	EDT				(D.T.		_	NDT	0.01		0.D.T.
Timer Results				EBL		EBI	WB		W	/BT	NBI	-	NBI	SBL	-	SBI
Assigned Phase	e			<u> </u>	\rightarrow	4	<u> </u>	\rightarrow	3	8			6	<u> </u>	_	2
Case Number						7.0		_	6	5.0			8.0			7.0
Phase Duration	i, S	\ \		<u> </u>	\rightarrow	21.0	<u> </u>	\rightarrow	21	1.0			59.0	<u> </u>		59.0
Change Period	nge Period, (Y+R c), s Allow Headway (<i>MAH</i>), s				_	5.5			5	0.5			5.9		_	5.9
Max Allow Head	Allow Headway (<i>MAH</i>), s eue Clearance Time (<i>g</i> s), s				+	4.2		\rightarrow	4	.2		_	0.0		_	0.0
Queue Clearan	eue Clearance Time (g_s), s				_	4.6		-	4	.8			0.0		_	0.0
Green Extensio	n Time	(ge), s			+	0.3		\rightarrow	0	0.3		\rightarrow	0.0			0.0
Phase Call Pro					_	1.00			0.	.96		_			_	
Max Out Proba	DIIITY					0.00			0.	.01						
Movement Gro	oup Res	ults			EB			WE	3			NB			SB	
Approach Move	ement			L	Т	R	L	Т	Т	R	L	Т	R	L	Т	R
Assigned Move	ment				4	14	3	8	+			6		5	2	12
Adjusted Flow I	Rate (v), veh/h			65	40	3	40				0		527	489	75
Adjusted Satura	ation Flo	w Rate (<i>s</i>), veh/h/l	In		1678	1422	1203	178	9			1662		1677	1564	1456
Queue Service	Time (g	gs), s			2.6	1.9	0.2	1.5	5			0.0		0.0	12.4	1.5
Cycle Queue C	learance	e Time (g c), s			2.6	1.9	2.8	1.5	5			0.0		12.0	12.4	1.5
Green Ratio (g	/C)	i			0.19	0.19	0.19	0.19	9			0.66		0.66	0.66	0.66
Capacity (c), v	/eh/h				325	276	284	347	7			1103		1163	1038	967
Volume-to-Cap	acity Ra	tio(X)			0.201	0.146	0.011	0.11	6			0.000		0.453	0.471	0.078
Back of Queue	(Q), ft	/In (95 th percentile	e)		52.5	32.4	2.4	29				0		173.3	166.1	18.5
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		2.1	1.3	0.1	1.1				0.0		6.8	6.5	0.7
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00	0.41	0.00	0.0	0			0.00		0.00	0.00	0.37
Uniform Delay	iform Delay (<i>d</i> 1), s/veh				27.2	26.9	28.4	26.	7			0.0		6.6	6.7	4.8
Incremental De	ncremental Delay (<i>d</i> ₂), s/veh				1.4	1.1	0.0	0.1				0.0		1.0	1.3	0.1
nitial Queue Delay ($d 3$), s/veh					0.0	0.0	0.0	0.0)			0.0		0.0	0.0	0.0
Control Delay (d), s/veh					28.6	28.0	28.4	26.9	9			0.0		7.7	7.9	5.0
Level of Service (LOS)					С	С	С	С						Α	Α	Α
Approach Dela	Approach Delay, s/veh / LOS					С	27.0)	(С	0.0			7.6		А
Intersection De	Intersection Delay, s/veh / LOS					1(0.0							В		
Multimodal Re	Iultimodal Results				EB			WE	3			NB			SB	
Pedestrian LOS	Score	/LOS		1.93		В	2.12	2	E	В	1.86	;	В	1.86		В
Bicycle LOS Sc	ore / LC	DS		1.73		В	1.63	3	E	В	1.56	5	В	2.30		В

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General Inform	nation								Int	tersect	ion Inf	ormatio	on	4	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	s la
Agency									Du	ration.	h	0.250)		7 † † ľ	
Analyst				Analys	is Dat	e 4/12/2	2023		Are	ea Type	<u>د</u>	CBD		 25		たみ
Jurisdiction				Time F	Period	.0 1/ 12/2	.020		PH	HE		0.78		→ \$	w‡e	↓ ↓
Urban Street		Madison		Analys	is Yea	r 2023			An	 nalvsis F	Period	1> 8.0	າດ			+ ¥ +
Intersection		Madison & Washing	notr	File Na	ame	2020 2 Jane	- Madie	son 2	044			1. 0.		- <u>-</u>		<u> </u>
Project Descrip	tion	2044 AM	gion				maan			7 401 0.7	100			-	* 1 4 * * 1	- (*
	lion	20117101														
Demand Inform	nation				EB			V	/B			NB			SB	
Approach Move	ement			L	Т	R	L	-	Т	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				107	' 10	38	2	08			0		41	845	25
				i			ш	_			-					
Signal Informa	ition			-	lst}∎											
Cycle, s	80.0	Reference Phase	2		ľ 1	· EĽ							1	2	3	
Offset, s	78	Reference Point	Begin	Green	36.4	32.6	0.0	0.	0	0.0	0.0					_
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.	0	0.0	0.0			1		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.4	2.2	0.0	0.	0	0.0	0.0		5	6	7	8
					_		14/5						NET	0.01		0.D.T.
Timer Results				EBL	· -	EBI	WB		N	VBI	NBI	-	NBI	SBL	-	SBI
Assigned Phase	e			<u> </u>	_	4	<u> </u>	\rightarrow	-	8			6	<u> </u>	_	2
Case Number				<u> </u>	_	8.0	<u> </u>	-	6	5.0 0.0			8.0	<u> </u>	_	5.0
Phase Duration	i, S	`		<u> </u>	_	38.0	<u> </u>	_	30	8.0			42.0	<u> </u>		42.0
Change Period	d, (Y+R c), s adway (MAH), s				_	5.4	<u> </u>	-	5	5.4 4.0			5.6		_	5.6
Max Allow Head	Headway (<i>MAH</i>), s arance Time (<i>g</i> s), s			<u> </u>	_	4.2		-	4	4.Z			0.0	<u> </u>		0.0
Queue Clearan	e Clearance Time (g_s), s				-	8.0		-	1	1.9			0.0		_	0.0
Green Extensio	n Time	(ge), s		<u> </u>		1.7	<u> </u>	\rightarrow	1	00			0.0			0.0
Max Out Broke						0.00	<u> </u>	-	1.	.00		_			_	
Max Out Proba	DIIILY					0.00			0.	.00						
Movement Gro	oup Res	ults			EB			W	В			NB			SB	
Approach Move	ement			L	Т	R	L	Т	Т	R	L	Т	R	L	Т	R
Assigned Move	ment				4	14	3	8				6		5	2	12
Adjusted Flow I	Rate (v), veh/h			150		49	26	7			0		54	1112	33
Adjusted Satura	ation Flo	w Rate (<i>s</i>), veh/h/l	In		1648	;	1096	183	3			1689		1578	1644	1509
Queue Service	Time (g	g s), S			6.0		3.2	9.9)			0.0		1.0	18.6	0.6
Cycle Queue C	learance	e Time (<i>g c</i>), s			6.0		9.0	9.9)			0.0		1.0	18.6	0.6
Green Ratio (g	/C)				0.41		0.41	0.4	1			0.45		0.45	0.45	0.45
Capacity (c), v	/eh/h				672		455	74	7			768		808	1496	687
Volume-to-Cap	acity Ra	tio(X)			0.223	3	0.107	0.35	57			0.000		0.067	0.743	0.048
Back of Queue	(Q), ft	/In (95 th percentile	e)		113.7	7	39.5	193	.1			0		17	212	10.1
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		4.4		1.5	7.5	5			0.0		0.7	8.2	0.4
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00		0.61	0.0	0			0.00		0.11	0.00	0.00
Uniform Delay	$\begin{bmatrix} d \\ 1 \end{bmatrix}, s/veh$				20.8		24.5	22.	3			0.0		8.0	11.2	8.0
Incremental De	tal Delay (<i>d</i> ₂), s/veh				0.8		0.3	0.9)			0.0		0.1	2.8	0.1
Initial Queue De	nitial Queue Delay (<i>d</i> ₃), s/veh				0.0		0.0	0.0)			0.0		0.0	0.0	0.0
Control Delay (Control Delay (<i>d</i>), s/veh				21.6		24.8	23.	2			0.0		8.2	13.9	8.1
Level of Service	evel of Service (LOS)				С		С	С						Α	В	Α
Approach Dela	Approach Delay, s/veh / LOS			21.6		С	23.4	1		С	0.0			13.5	5	В
Intersection De	Intersection Delay, s/veh / LOS					16	5.1							В		
Multimodal Re	Itimodal Results				EB			W	В	_		NB	_		SB	-
Pedestrian LOS	Score	Score / LOS				B	2.26	j		В	1.90)	В	1.67		В
Bicycle LOS Sc	ore / LC)S		1.81		В	2.08	3		В	1.56	5	В	2.52	2	С

5							,					
General Information						Intersec	tion Info	ormatio	on	2	4244	× l <u>x</u>
Agency						Duration	, h	0.250	-		Ļ	
Analyst	Analys	is Date	4/12/2	023		Area Tvp	e	CBD		 		r. 4
Jurisdiction	Time F	Period				PHF		0.87			w∔e	~ _ ∳
Urban Street East	Analys	is Year	2023			Analysis	Period	1> 8:0	00			
Intersection East & Front	File Na	ame	2 lane	- East 2	2044	PM S.xus					1 b	
Project Description 2044 PM											4 1 4 17 1	× (*
Demand Information		EB			W	В		NB			SB	
Approach Movement	L	Т	R	L	Т	R	L	Т	R	L	Т	R
Demand (<i>v</i>), veh/h	90	127			54	4 25	48	1005	5 32		0	
	1											
	-	+		=						KŤ2		~
Cycle, s 80.0 Reference Phase 2	-	1 5 †?							1	2	3	4
Unset, s 58 Reference Point Begin	Green	52.4	16.8	0.0	0.0) 0.0	0.0					5
Concoordinated No Simult Cap E/W On	Yellow	3.2	3.2	0.0	0.0	0.0	0.0	-			-	•
Force Mode Fixed Simult. Gap N/S On	Reu	2.4	2.0	0.0	0.0	0.0	0.0		5	6		8
Timer Results	EBI		EBT	WB		WBT	NBI		NBT	SBI		SBT
Assigned Phase		-	4		-	8		-	2			6
Case Number			6.0		+	7.0			8.0			8.0
Phase Duration, s			22.0		+	22.0			58.0			58.0
Change Period, (Y+R c), s			5.2			5.2			5.6			5.6
Max Allow Headway (MAH), s			4.3			4.3			0.0			0.0
Queue Clearance Time ($g s$), s			10.7		_	4.4						
Green Extension Time (g e), s			0.7		_	1.0			0.0			0.0
Phase Call Probability			1.00		\rightarrow	1.00						
Max Out Probability			0.41			0.02						
Movement Group Results		EB			WE	3		NB			SB	
Approach Movement	L	Т	R	L	Т	R	L	Т	R	L	Т	R
Assigned Movement	7	4			8	18	5	2	12		6	
Adjusted Flow Rate (v), veh/h	103	146			62	29	588		536		0	
Adjusted Saturation Flow Rate (s), veh/h/ln	1182	1787			1683	3 1390	1689		1546		1662	
Queue Service Time (<i>g</i> s), s	6.3	5.6			2.4	1.3	0.0		14.8		0.0	
Cycle Queue Clearance Time (g c), s	8.7	5.6			2.4	1.3	14.5		14.8		0.0	
Green Ratio (g/C)	0.21	0.21			0.21	0.21	0.66		0.66		0.66	
Capacity (<i>c</i>), veh/h	303	375			353	292	1155		1013		1089	
Volume-to-Capacity Ratio (X)	0.342	0.389			0.17	6 0.098	0.509		0.529		0.000	
Back of Queue (Q), ft/In (95 th percentile)	82.2	109.6			48.1	22.2	203.3		192.6		0	
Back of Queue (Q), veh/In (95 th percentile)	3.2	4.3			1.9	0.9	8.1		7.7		0.0	
Queue Storage Ratio (<i>RQ</i>) (95 th percentile)	0.91	0.00			0.00	0.22	0.00		0.00		0.00	
Uniform Delay (<i>d</i> 1), s/veh	29.5	27.2			25.9) 25.5	7.4		7.4		0.0	
Incremental Delay (d ₂), s/veh	0.7	0.7			1.1	0.7	1.4		1.7		0.0	
Initial Queue Delay (d ȝ), s/veh	0.0	0.0			0.0	0.0	0.0		0.0		0.0	
Control Delay (<i>d</i>), s/veh	30.2	27.8			27.0) 26.2	8.7		9.1		0.0	
Level of Service (LOS)	С	С			С	C	A		A			
Approach Delay, s/veh / LOS	28.8		С	26.7	·	С	8.9		A	0.0		
Intersection Delay, s/veh / LOS			13	.4						В		
Multimodal Results		FR				3		NR			SB	
Pedestrian LOS Score / LOS	1.93		В	1.94		B	1 86		В	1 87		В
	1.0					-						

			- 5							,					
General Inform	nation								Intersec	tion Info	ormatio	on	Į	****	× l <u>x</u>
Agency									Duration	. h	0.250)		Ļ	
Analyst				Analys	is Date	4/12/2	023		Area Tvr	e.	CBD		 		<u>بر</u> 4
Jurisdiction				Time F	Period				PHF		0.96		→ 	w‡e	-{
Urban Street		Fast		Analys	is Year	2023			Analysis	Period	1> 8:0	00	*		+ ∀
Intersection		East & Jefferson		File Na	ame	2 lane	- East 2	2044	PM S.xus		1			wî îr	
Project Descrip	tion	2044 PM											1	া ব ↑ ক প 1	- (*
, ,		-													
Demand Inform	nation				EB			W	В		NB			SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Demand (v), v	eh/h			29	28			2	5 23	23	1141	7		0	
				ih				_							
Signal Informa	tion	1	1		↓		=						-+-		_
Cycle, s	80.0	Reference Phase	2		517	, Filip						1		3	→ 4
Offset, s	55	Reference Point	Begin	Green	55.8	13.8	0.0	0.0	0.0	0.0			•		5
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0	0.0	0.0			-		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.0	2.0	0.0	0.0	0.0	0.0		5	6	7	8
T' D K			_	EDI		EDT				NIDI		NDT	0.01		ODT
Timer Results				EBL	-	EBI	WBI		WB1	NBL		NBI	SBL		SBI
Assigned Phase	e					4		+	8		_	2			6
Case Number						8.0		-	8.0			8.0			8.0
Change Duration				<u> </u>		19.0			19.0			5.2			51.0
Max Allow Hear	nange Period, (Y+R c), s ax Allow Headway (<i>MAH</i>), s					5.Z		-	0.2		-	5.Z			5.Z
	ax Allow Headway (<i>MAH</i>), s					4.2		+	4.2			0.0			0.0
Green Extensio	n Time	$(g_s), s$				۰.1 0.2			0.2	-		0.0			0.0
Phase Call Pro	hability	(ge), s		<u> </u>		1.00		+	0.2			0.0			0.0
Max Out Proba	hility			<u> </u>		0.01		-	0.01						
	onity					0.01			0.01						
Movement Gro	oup Res	ults			EB			WB	;		NB			SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Assigned Move	ment			7	4			8	18	5	2	12		6	
Adjusted Flow I	Rate(<i>v</i>), veh/h			59			50		626		571		0	
Adjusted Satura	ation Flo	ow Rate (<i>s</i>), veh/h/l	In		1480			1530)	1674		1528		1636	
Queue Service	Time (g	g s), s			0.4			2.2		0.0		14.3		0.0	
Cycle Queue C	learance	e Time (<i>g c</i>), s			2.7			2.2		14.3		14.3		0.0	
Green Ratio (g	/C)				0.17			0.17	'	0.70		0.70		0.70	
Capacity (c), v	/eh/h				323			264		1214		1066		1141	
Volume-to-Capa	acity Ra	tio(X)			0.184			0.18	9	0.516		0.536		0.000	
Back of Queue	(Q), ft	/In (95 th percentile	e)		48.9			37.8	3	179		169.4		0	
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		1.9			1.5		7.2		6.8		0.0	
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00			0.00)	0.00		0.00		0.00	
Uniform Delay	(d 1), si	/veh			28.4			28.3	3	5.8		5.8		0.0	
Incremental Delay (<i>d</i> ₂), s/veh					1.2			0.3		1.2		1.5		0.0	
nitial Queue Delay (d ȝ), s/veh					0.0			0.0		0.0		0.0		0.0	
Control Delay (d), s/veh					29.6			28.7		7.0		7.3		0.0	
Level of Service (LOS)					С			C		A		A			
Approach Delay, s/veh / LOS				29.6		С	28.7		С	7.2		A	0.0		
Intersection Delay, s/veh / LOS						9	.0						4		
Multimodal Results					ED									CD	
Pedestrian LOS	Iultimodal Results					B	1 04		B	1 60		B	1.62		B
Riovela LOS So				1.93		B	1.94		B	2.57		0	1.03	· · ·	B
				1.00		U	1.04		D	2.57		U	1.00		U

			e eigi				000	UNIX			Jam	mary					
General Inform	nation									Int	ersect	ion Info	ormatic	on	J.	≈↓ ↓↓	Ja l <u>a</u>
Agency	lation									Du	iration	h	0 250				
Analyst				Analys	is Da	te 4/	12/20	123		Are	ea Type	, 	CBD		 		۲. 4
lurisdiction				Time F	Period		12/2	520		PH	ie IE	,	0.03		->> >	w∔e	2- 4-
Lirban Street		Fast				ar 20	123			Δn	n alveie F	Pariod	1> 8.0	0			+ ₹
Intersection		East & Locust				20	lane	- East (2044		N vue	chou	12 0.0	,0			
Project Descrip	tion				ame		lane		2044		IN.AUS				5	* ₩≉↑►	tr r
T TOJECT Descrip	lion	2044110															
Demand Inform	nation				EE	3			W	/B			NB			SB	
Approach Move	ement			L	Т		R	L		Г	R	L	Т	R	L	Т	R
Demand (v), v	eh/h			113	599	9							1109	231			
				1							í.						
Signal Informa	tion						2								*		7
Cycle, s	70.0	Reference Phase	2		•	ta E	÷							1	2	3	4
Offset, s	10	Reference Point	Begin	Green	43.7	7 1	5.3	0.0	0.0	0	0.0	0.0			-		
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.	.2	0.0	0.0	0	0.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.3	2.	.3	0.0	0.0	0	0.0	0.0		5	6	7	8
T' D K			_	EDI			T I		_	10				NDT	0.01		ODT
Assigned Dhee				EBL	-	EBI	-	VVBI	-+	VV	/BT	NBL	-		SBL		SBI
Assigned Phase	e				\rightarrow	4			+					2			
Case Number					-	12.0)		+		-		_	8.0			
Phase Duration	i, s			<u> </u>		20.8	5		_					49.2			
Max Allow Hoor	$(1 + \mathbf{R})$	c), S		<u> </u>	-	5.5	·		-					0.0			
	uway (<i>1</i>	$(\alpha, \beta) \in \mathcal{O}$		<u> </u>	+	4.1	7		+		-		-	0.0		+	
Green Extensio	n Time	$(g_s), s$			-	2.7	,		-		-			0.0			
Phase Call Pro	hability	(ge), s		<u> </u>	-	1.00	2		+					0.0			
Max Out Proba	hility				-	0.08	R		+		-						
Max Out 1 10ba	onity					0.00	5										
Movement Gro	oup Res	sults			EB				WE	3			NB			SB	
Approach Move	ement			L	Т	F	R	L	Т		R	L	Т	R	L	Т	R
Assigned Move	ment			7	4								2	12			
Adjusted Flow I	Rate(<i>v</i>), veh/h		268	497	·							693	654			
Adjusted Satura	ation Flo	ow Rate (<i>s</i>), veh/h/l	In	1646	1683	3							1683	1574			
Queue Service	Time (🤉	g s), s		10.7	9.5								28.5	18.8			
Cycle Queue C	learanc	e Time (<i>g c</i>), s		10.7	9.5								28.5	18.8			
Green Ratio (g	/C)			0.22	0.22	2							0.62	0.62			
Capacity (c), v	/eh/h			360	737	·							1050	982			
Volume-to-Cap	acity Ra	itio(X)		0.745	0.67	5							0.660	0.666			
Back of Queue	(Q), ft	VIn (95 th percentile	e)	193.7	168.	7							231.4	220			
Back of Queue	(Q), ve	eh/In (95 th percent	ile)	7.6	6.6								9.1	8.8			
Queue Storage	Ratio (RQ) (95 th percent	tile)	0.00	0.00)				_			0.00	0.00			
Uniform Delay	Delay (d_1), s/veh			25.5	25.0)				4			8.5	8.6			
Incremental De	mental Delay (<i>d</i> ₂), s/veh			3.1	1.1	_				+			2.1	2.3			
Initial Queue De	tial Queue Delay (d 3), s/veh			0.0	0.0	_				4			0.0	0.0			
Control Delay (ontrol Delay (d), s/veh			28.6	26.1								10.6	10.9			
Level of Service	vel of Service (LOS)			С	С								В	В			
Approach Delay	Approach Delay, s/veh / LOS					С		0.0				10.7		В	0.0		
Intersection De	tersection Delay, s/veh / LOS						16	.6				_	_	E	3		
Multimodal Ba	sulte				ED				\\//	2			NP			QD	
Pedestrian LOS	Score	/1.05	1 71	28	R		1 70		ر ا	B	1 96		B	1 05	30	B	
Bicycle LOS Sc	core / I C)S	1 98		B		1.72			_	2 75		C	0.00		A	
2.0,00 200 00				1.00		5						2.70		~	0.00		

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General Inform	nation								Inters	ecti	ion Info	ormatio	on		l ad _a t _a eta ↓	له ل <u>ر</u>
Agency	lation								Durati	on	h	0 250			Ļ	
Analyst				Analys	is Dat	a 1/12/2	2023		Δrea T	- - - Vne	<u>، انا</u>	CBD		 		۲. 4
lurisdiction				Time F	Period		.020		PHE	урс	,	0.04		→ ⊹	w∔e	2. 4
Urban Street		Fast		Analys	is Ves	r 2023			Analys	eie F	Pariod	1> 8.	າດ			+ *
Intersection		East & Market			ame	2 Jane	- Fast '	2011	PM N v	110	criou	12 0.	50			<u>~</u>
Project Descrip	tion	2044 PM						2077		us					- \ ব ↑ ক \7	۲ r
Troject Descrip		2044110														
Demand Inform	nation				EB			N	/B			NB			SB	
Approach Move	ement			L	Т	R	L	-	T I	२	L	Т	R	L	Т	R
Demand (v), v	eh/h			152	0						139	1114			0	
Signal Informa	tion	v			†									_		_
Cycle, s	70.0	Reference Phase	2		51	E -							1		3	
Offset, s	64	Reference Point	Begin	Green	44.5	14.6	0.0	0.	0 0.	.0	0.0					
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.	0 0.	.0	0.0				_ │	
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.3	2.2	0.0	0.	0 0.	.0	0.0		5	6	7	8
														1		
Timer Results				EBL	-	EBT	WB		WBT	_	NBL	-	NBT	SBI	-	SBT
Assigned Phase	e			<u> </u>	-	4		_		4			2			6
Case Number						12.0		\rightarrow		4			8.0			8.0
Phase Duration	l, S					20.0		_		4			50.0			50.0
Change Period	nge Period, (Y+R c), s Allow Headway (MAH), s					5.4		\rightarrow		4			5.5			5.5
Max Allow Head	(Allow Headway (<i>MAH</i>), s					4.2		\rightarrow		4			0.0			0.0
Queue Clearan	ce Time	e (g s), s				9.0		_		4						
Green Extensio	on Time	(ge),s			\rightarrow	0.2		\rightarrow		4			0.0			0.0
Phase Call Pro	bability				_	1.00		\rightarrow		4						
Max Out Proba	bility					0.35										
Movement Gro	oup Res	sults			FB			W	3	T		NB			SB	
Approach Move	ement				Т	R		Т	R	T	1	Т	R	1	Т	R
Assigned Move	ment			7	4		_			+	5	2		_	6	
Adjusted Flow I	Rate (v), veh/h			162					T	686	647			0	
Adjusted Satura	ation Flo	w Rate (s), veh/h/	In		1607	1				+	1469	1418			1825	
Queue Service	Time (d	(s), S			7.0					T	2.4	20.1			0.0	
Cycle Queue C	learance	e Time (<i>g</i> c), s			7.0					1	7.2	20.1			0.0	
Green Ratio (g	/C)				0.21					T	0.64	0.64			0.64	
Capacity (c), v	/eh/h				335						996	902			1160	
Volume-to-Cap	acity Ra	tio(X)			0.483	3				T	0.688	0.718			0.000	
Back of Queue	(Q), ft	/In (95 th percentile	e)		148						79.7	88.3			0	
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		5.7					Т	3.2	3.4			0.0	
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00					Т	0.00	0.00			0.00	
Uniform Delay	inform Delay ($d \tau$), s/veh				31.2						1.3	1.3			0.0	
Incremental De	ncremental Delay (<i>d</i> 2), s/veh				3.4						3.9	4.9			0.0	
Initial Queue De	nitial Queue Delay (d 3), s/veh				0.0						0.0	0.0			0.0	
Control Delay (Control Delay (<i>d</i>), s/veh				34.6						5.2	6.2			0.0	
evel of Service (LOS)					С						А	А				
Approach Delay, s/veh / LOS				34.6	;	С	0.0				5.7		А	0.0		
Intersection Delay, s/veh / LOS						8	.8							A		
Multimedal De			EP			10/1	2						CD			
Pedestrian LOS	ultimodal Results					B	1 05		, p	-	1 24	IND	Δ	1 20	3B	Δ
Biovola LOS So	destrian LOS Score / LOS					B	1.90	,	D		2 66		<u>л</u>	1.50	, , , , , , , , , , , , , , , , , , ,	B
		<i>,</i> ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		1.02	·	D					2.00		0	1.50	,	U

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General Inform	nation								Inte	arsoct	ion Infr	ormatio	n		at _1, eta ↓ ,	þá l _{al}
Agency	lation								Dur	ration	h	0 250			Ļ	
Apolyet				Analys	vic Dot	0 1/12/2	0022		Aro		<u>.</u>	CRD		_7 _5		۲. 4
Analyst				Time	Dariad	.e 4/12/2	.023			атурс г		0.07		→ & _4	wŶF	2-
Jurisaiction		Foot				r 2022					Doriod	1 0.97	20			4
Under Street		Edst				ai 2023	- Feet (2044			enou	12 0.	00			<u> </u>
Breiget Deserin	tion				ame	Ziane	- East 2	2044	PIVI	S.xus				_	া া বিশক্ষা	× ۲
Project Descrip	lion	2044 1111														
Demand Inform	nation				EB			W	/B			NB			SB	
Approach Move	ement			L	Т	R	L	-	Т	R	L	Т	R	L	Т	R
Demand (v), v	eh/h			54	0						21	1165	5		0	
															<u>.</u>	n
Signal Informa	tion	v-	2			2										_
Cycle, s	80.0	Reference Phase	2		84	. ⊨⊂							1	N	-	-
Offset, s	59	Reference Point	Begin	Green	53.8	15.8	0.0	0	0	0.0	0.0				3	4
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.	0	0.0	0.0			_		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.0	2.0	0.0	0.	0	0.0	0.0		5	6	7	8
										_						
Timer Results				EBL	-	EBT	WBI		W	BT	NBL	-	NBT	SBI	-	SBT
Assigned Phase	e					4							2			6
Case Number						12.0							8.0			8.0
Phase Duration	i, S					21.0							59.0			59.0
Change Period	nge Period, (Y+R c), s Allow Headway (<i>MAH</i>), s					5.2							5.2			5.2
Max Allow Head	Allow Headway (<i>MAH</i>), s				\perp	4.2							0.0			0.0
Queue Clearan	eue Clearance Time (g_s), s					4.7										
Green Extensio	on Time	(g _e), s			\rightarrow	0.1		_		_			0.0		_	0.0
Phase Call Pro	bability					1.00										
Max Out Proba	bility		_			0.00										
Movement Gro	un Res	ults	_		FB			\//	R			NB			SB	_
Approach Move	ment				Т	R		Т		R	1	Т	R	1	Т	R
Assigned Move	ment			7	4			-	-		5	2		<u> </u>	6	
Adjusted Flow F	Rate (v) veh/h			56	-					638	- 583			0	
Adjusted Satura	ation Flo	w Rate (s) veh/h/	In		1632	,			+		1781	1628			1599	
Queue Service	Time (d	a s). S			2.7	-					0.0	13.7			0.0	
Cvcle Queue C	learance	e Time (2.7				-		3.4	13.7			0.0	
Green Ratio (g	/C)				0.20						0.67	0.67			0.67	
Capacity (c), v	, /eh/h				322						1244	1095			1075	
Volume-to-Cap	acity Ra	itio(X)			0.173	3					0.513	0.532			0.000	
Back of Queue	(Q), ft	/In (95 th percentile	e)		55.6						43.6	43.6			0	
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		2.1	1					1.7	1.7			0.0	
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00						0.00	0.00			0.00	
Uniform Delay ((d1), s	/veh			33.5						0.9	0.9			0.0	
Incremental De	ncremental Delay (<i>d z</i>), s/veh				1.2	1					1.2	1.5	İ		0.0	
Initial Queue De	nitial Queue Delay ($d 3$), s/veh				0.0						0.0	0.0			0.0	
Control Delay (Control Delay (<i>d</i>), s/veh				34.6						2.1	2.4			0.0	
Level of Service	evel of Service (LOS)				С						А	А				
Approach Delay, s/veh / LOS				34.6	6	С	0.0				2.2		А	0.0		
Intersection Delay, s/veh / LOS						3	.6							A		
Multimodal Re	ultimodal Results				EB			W	В			NB			SB	
Pedestrian LOS	lestrian LOS Score / LOS				5	В	1.96	;	B	3	1.34	·	A	1.34		А
Bicycle LOS So	ore / LC	DS		1.64		В					2.57		С	1.55	5	В

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General Inform	nation								Intersec	tion Info	ormatio	on	2	***	s l <u>a</u>
Agency									Duration	, h	0.250)		Ļ	
Analvst				Analys	is Date	4/12/2	023	_	Area Tvp	e	CBD		 		۲. ۲.
Jurisdiction				Time F	eriod				PHF		0.92			W TE	
Urban Street		East		Analys	is Year	2023			Analysis	Period	1> 8:0	00			+- *7
Intersection		East & Olive		File Na	me	2 lane	- East	2044	PM S.xus					st ta	
Project Descrip	tion	2044 PM											1	1 1 1 4 1 1	* (*
										14					
Demand Inform	nation				EB			W	Έ		NB			SB	
Approach Move	ement			L	Т	R	L	1	R	L	Т	R	L	Т	R
Demand (v), v	eh/h			87	67			3	9 30	18	917	16		0	
	tion			1					_						
		Deference Dhase	2		+	1.2 E	=						stz.		~
Cycle, s	60.0	Reference Priase	2 Dogin		- * †							1	2	3	4
Unset, s	04 N.a		Бедіп	Green	49.4	19.5	0.0	0.0	0.0	0.0					5
	Tixed	Simult Cap N/S	On	Yellow	3.2	3.2	0.0	0.0	0.0	0.0	-	_		-	•
Force Mode	Fixed	Simult. Gap N/S	On	Reu	2.4	2.3	0.0	0.0	0.0	0.0		5	6	7	8
Timer Results				EBI		EBT	WB		WBT	NBI		NBT	SBI		SBT
Assigned Phase	e					4			8		-	2			6
Case Number	<u> </u>					8.0		-	8.0			8.0		-	8.0
Phase Duration	I, S		_		-	25.0		-	25.0		-	55.0			55.0
Change Period	ange Period, (Y+R c), s					5.5		\rightarrow	5.5			5.6			5.6
Max Allow Head	ax Allow Headway (<i>MAH</i>), s					4.2			4.2			0.0			0.0
Queue Clearan	ce Time	(g s), s				10.0			3.4						
Green Extensio	on Time	(ge),s				0.6			0.7			0.0			0.0
Phase Call Pro	bability					1.00			1.00						
Max Out Proba	bility					0.04			0.00						
Movement Gra		ulto			ED			\\/	2		ND			CD	
Approach Move	mont	Suits			 	D						D		т	D
	ment				1			8	18	5	2	12		6	T.
Adjusted Flow F	Rate (v) veh/h			167			38	37	542	2	/02		0	
Adjusted Satura	ation Flo), ven/n w Rate (s) veh/h/	In		130/			178	1 1627	1551		1/00		1825	
	Time ((τ_{s}) s			6.5			1.3	1 4	0.0		16.3		0.0	
	learance	$a = Time(a_c) s$			8.0			1.3	1.1	16.2		16.3		0.0	
Green Ratio (o	V/C	5 mile (g c), 6			0.24			0.24	4 0.24	0.62		0.62		0.62	
Capacity (c), y	/eh/h				410			435	5 397	1004		870		1127	
Volume-to-Cap	acitv Ra	tio (X)			0.408			0.08	7 0.094	0.540		0.565		0.000	
Back of Queue	(Q), ft	/In (95 th percentile	e)		137.5			25.6	6 24.7	223.1		211		0	
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		5.3			1.0	1.0	8.9		8.4		0.0	
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00			0.00	0.00	0.00		0.00		0.00	
Uniform Delay	(d1), s	/veh			25.8			23.4	4 23.4	8.9		8.9		0.0	
Incremental Delay (d_2), s/veh					2.8			0.1	0.1	2.1		2.7		0.0	
Initial Queue Delay (d ȝ), s/veh					0.0			0.0	0.0	0.0		0.0		0.0	
Control Delay (d), s/veh					28.6			23.	5 23.5	11.0		11.6		0.0	
Level of Service (LOS)					С			С	С	В		В			
Approach Delay, s/veh / LOS				28.6		С	23.5	5	С	11.3		В	0.0		
Intersection Delay, s/veh / LOS						14	.3						В		
Multimedal Descrite															
Multimodal Re	Iultimodal Results				EB		4.00	WE	5	4.07	NB	P	4.07	SB	
Pedestrian LOS	Score	/ LUS		1.92		В	1.93	5	В	1.87		В	1.87		В
BICYCIE LOS SC	ore / LC	15		1.83		В	1.62	<u> </u>	В	2.41		В	1.55		В

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General Inform	nation								Ir	ntersect	ion Infe	ormatio	on	2	at 1/2 at 1 .	þa ly
Agency									D	uration,	h	0.250			Ļ	
Analyst				Analys	is Dat	e 4/12/	2023		A	rea Tvp	e	CBD		 		₹
Jurisdiction				Time F	Period	<u> </u>			P	PHF		0.93			W TE	 ↓ /ul>
Urban Street		East		Analys	is Yea	r 2023			A	nalvsis	Period	1> 8:0	00			→ 7
Intersection		East & Washington		File Na	ame	2 lan	e - Eas	st 204	4 PI	V S.xus					5 4 4 7	
Project Descrip	tion	2044 PM		I										ň	ব ↑ ক পি '	7
							Y				Y					
Demand Inform	nation				EB		+		WB			NB			SB	
Approach Move	ement			L	Т	R			Т	R	L	Т	R	L	T	R
Demand (v), v	eh/h			38	246	i			315	96	35	1001	79		0	
Signal Informa	tion			1												
	80.0	Reference Phase	2		•	_ <u></u>	-							572		
Offset s	60	Reference Point	Begin		1								1	2	3	4
Uncoordinated	No	Simult Gap F/W	On	Green	48.0	20.6	0.0	(0.0	0.0	0.0	_				₹.
Force Mode	Fixed	Simult, Gap N/S	On	Red	3.Z 2.8	3.2	0.0).U) ()	0.0	0.0	-	5	6	7	8
	Тіхоч	onnuit. Oup 100	OII	Ttou	2.0	2.2	0.0			0.0	0.0					
Timer Results				EBL		EBT	N	'BL	, I	WBT	NBL	_	NBT	SBL	-	SBT
Assigned Phase	e					4				8			2			6
Case Number						6.0				7.0			5.0			8.0
Phase Duration	, S					26.0				26.0			54.0			54.0
Change Period	nange Period, (Y+R c), s					5.4				5.4			6.0			6.0
Max Allow Head	ax Allow Headway (<i>MAH</i>), s					4.2				4.2			0.0			0.0
Queue Clearan	ce Time	(g s), s				20.6				17.2						
Green Extensio	n Time	(ge),s				0.0				1.1			0.0			0.0
Phase Call Pro	bability					1.00				1.00						
Max Out Proba	bility					1.00				1.00						
Movement Cre	un Dee	ulto			ГР			M				ND			<u>e</u> p	
Movement Gro	oup Res	suits			EB	D	<u>.</u>	V	vв т				D		<u>5</u> В т	В
Approach Move	mont				1	ĸ			ן ס	Г. 19	L 5	1 2	<u>г</u> 12		۱ ۴	ĸ
Adjusted Flow) voh/h		/	265			2	20 20	102	27	2 1053	92		0	
Adjusted Flow I		y, ven/n	In	41	1790		-	16	ເຄງ	1294	1620	1703	1529		1679	
		π (3) , veri/in		937 3.4	1/08	·	-	10	5.2	1304	0.7	1/ 3	1.920		0.0	
	learance	a Time (a_{α}) s		18.6	10.3		-	10	5.2	4.0	0.7	1/1 3	1.0		0.0	
Green Ratio ($\frac{1}{2}$	5 mile (<i>g t</i>), 3		0.26	0.26		-		26 26	0.26	0.7	0.60	0.60		0.0	
Capacity (c)	/o/) /eh/h			153	461	+	-	<u> </u>	28	356	1067	2044	917		1007	
Volume-to-Cap	acity Ra	tio (X)		0.267	0.574	L		07	20 791	0.290	0.035	0.515	0.091		0.000	
Back of Queue	(Q), ft	/In (195 th percentile	e)	36.4	192.8	3	-	28	6.1	71.4	10.8	200.9	25.4		0	
Back of Queue	(Q), ve	eh/In (95 th percent	, ile)	1.4	7.6			11	1.3	2.8	0.4	7.9	1.0		0.0	
Queue Storage	Ratio (RQ) (95 th percen	, tile)	0.56	0.00	+		0.	00	0.89	0.14	0.00	0.00		0.00	
Uniform Delay	(d1), s/	/veh		36.4	25.9			27	7.7	23.8	6.5	9.3	6.8		0.0	
Incremental Delay (d_2), s/veh				0.8	1.5			9	.7	0.4	0.0	0.7	0.2		0.0	
Initial Queue De		0.0	0.0			0	.0	0.0	0.0	0.0	0.0		0.0			
Control Delay (d), s/veh				37.2	27.4			37	7.4	24.3	6.6	10.0	6.9		0.0	
Level of Service (LOS)				D	С			1	D	С	Α	А	Α			
Approach Delay	Approach Delay, s/veh / LOS					С	34	1.4		С	9.7		А	0.0		
Intersection De	Intersection Delay, s/veh / LOS					1	8.4					4		B		
Multimodal Re	Aultimodal Results				EB			V	VB			NB			SB	
Pedestrian LOS	Score	/LOS		2.51		С	2.	33		В	2.13	;	В	2.10		В
Bicycle LOS Sc	ore / LC	DS		2.56		С	2.	87		С	3.45	5	С	2.10		В

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General Inform	nation								Inters	ectio	on Info	ormatic	on	J.	ad _a l _a aba ↓	be la
Agency	lution								Duratio	on h		0 250			44	
Analyst				Analys	is Dat	e 4/12/	2023		Area T	vne		CBD		 		₹. &
Jurisdiction				Time F	Period	.0 1/ 12/	_0_0		PHF	700		0.97		→ ∲- ,	w↓e	- }
Urban Street		Madison		Analys	is Yea	ar 2023			Analys	sis Pe	eriod	1> 8.0)()			→ *
Intersection		Madison & Front		File Na	ame	2 lan	e - Madi	son 20)44 PM	S.xi	JS				+	<u>r</u>
Project Descrip	tion	2044 PM				1								1	* 1 * *	۲ (*
· · ·) · · · · · · · ·																
Demand Inform	nation				EB			W	В			NB			SB	
Approach Move	ement			L	Т	R	L	Т	· F	र	L	Т	R	L	Т	R
Demand (v), v	eh/h				39	15	58	7	1			0		25	1134	30
				10												
Signal Informa	tion		r		₩		_									
Cycle, s	80.0	Reference Phase	2		ľ 1	• 🗟 '	2						1	2	3	
Offset, s	0	Reference Point	Begin	Green	59.2	9.6	0.0	0.0) 0.	0	0.0					_
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0) 0.	0	0.0			1		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.5	2.3	0.0	0.0) 0.	0	0.0		5	6	7	8
			_													
Timer Results				EBL	-	EBT	WB		WBT		NBL	-	NBT	SBL	+	SBT
Assigned Phase	9				_	4		_	8	_		_	6		\rightarrow	2
Case Number					_	8.0	<u> </u>		6.0	_		_	8.0		+	8.0
Phase Duration	, S	```			_	15.1	<u> </u>	_	15.1	_			64.9		\rightarrow	64.9
Change Period,	(Y+R)	c), S			_	5.5	<u> </u>		5.5	+			5.7		+	5.7
Max Allow Head	dway(A	ИАН), s		<u> </u>	_	4.2	<u> </u>	_	4.2	_		_	0.0		\rightarrow	0.0
Queue Clearan		(gs), s		<u> </u>	_	4.6		_	8.3	+		_			+	
Green Extensio	Phase Call Probability			<u> </u>	_	0.5		_	0.4	_		_	0.0	<u> </u>	+	0.0
Phase Call Probability				<u> </u>		0.98		_	0.98					<u> </u>	+	
Max Out Proba	Max Out Probability					0.00			0.06							
Movement Gro	oup Res	ults			EB			WE	;	Т		NB			SB	
Approach Move	ement			L	Т	R	L	Т	R		L	Т	R	L	Т	R
Assigned Move	ment				4	14	3	8			_	6		5	2	12
Adjusted Flow F	Rate (v), veh/h			56		60	73				0		713		646
Adjusted Satura	ation Flo	w Rate (s), veh/h/l	In		1548	;	1241	178	9			1705		1707		1549
Queue Service	Time (g	g s), S			2.6		3.7	3.0		Т		0.0		0.0		14.9
Cycle Queue C	learance	e Time (g c), s			2.6		6.3	3.0				0.0		14.7		14.9
Green Ratio (g	/C)				0.12	1	0.12	0.12	2			0.74		0.74		0.74
Capacity (c), v	/eh/h				185		198	214				1262		1311		1147
Volume-to-Capa	acity Ra	tio(X)			0.301	1	0.303	0.34	2			0.000		0.544		0.563
Back of Queue	(Q), ft	/In (95 th percentile	e)		46.1		52.1	60.7	'			0		174.9		162.8
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		1.8		2.1	2.4				0.0		6.9		6.5
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00	1	0.87	0.00)			0.00		0.00		0.00
Uniform Delay ((d1), s	/veh			32.2		35.0	32.3	3			0.0		4.6		4.6
Incremental De	lay (<i>d</i> 2), s/veh			0.9		0.9	0.9				0.0		1.3		1.6
Initial Queue De	elay (d	з), s/veh			0.0		0.0	0.0				0.0		0.0		0.0
Control Delay (d), s/ve	eh			33.1		35.9	33.3	3			0.0		5.9		6.2
Level of Service	e (LOS)				С		D	С						A		Α
Approach Delay	, s/veh	/LOS		33.1		С	34.4	4	С		0.0			6.1		A
Intersection De	lay, s/ve	h / LOS				9	9.5							A		
Multimodal Re	sults				EB			WE	6			NB			SB	
Pedestrian LOS	Score	/LOS		1.94		В	1.94	4	В		1.84		В	1.61		В
Bicycle LOS Sc	ore / LC)S		1.65		В	1.78	3	В		1.56		В	2.57		С

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General Inform	nation								Intersed	tion In	format	ion	J.	4↓4+↓	be la
Agency									Duration	. h	0.25	0			
Analyst				Analys	is Dat	e 4/12/2	2023		Area Tv	be	СВГ)	 		۲. ۲.
Jurisdiction				Time F	Period				PHE		0.85	-	→ ∲- ,	w‡e	
Urban Street		Madison		Analys	is Yea	r 2023			Analysis	Period	1> 8	·00	*		+ *
Intersection		Madison & Jefferso	n	File Na	ame	2 lane	- Madis	son 20)44 PM S					•	<u> </u>
Project Descrip	tion	2044 PM		1 110 110			maan	501120					5	414Y	ት / [*]
r rojoot b ooonp		201111													
Demand Inform	nation				EB			W	В		NE	3		SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				21	10	28	23	3		0		20	1156	19
				i						_			-		
Signal Informa	tion				14	5									
Cycle, s	80.0	Reference Phase	2		ľ ↑	\mathbb{R}^{2}						1	2	3	
Offset, s	0	Reference Point	Begin	Green	54.6	14.9	0.0	0.0	0.0	0.0					
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0	0.0	0.0			1		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.2	1.9	0.0	0.0	0.0	0.0		5	6	7	8
Timer Description			_		_	EDT				NIT		NDT		1	ODT
Accident Accounts				EBL		EBI	VVB		VVB1	NE	SL	NBI	SBL		SBI
Assigned Phase	9				+	4		-	8	<u> </u>		0	<u> </u>		2
Case Number						8.0		-	8.0			8.0			8.0
Change Duration	, S (V+D			<u> </u>	+	20.0		+	20.0	-		5.4	<u> </u>		5.4
Max Allow Head	dwav (A	c), s MAH), s	_		-	4.2		-	4.2			0.0		-	0.0
Queue Clearan	ce Time	e (q s), S			+	3.5		-	4.3			0.0			0.0
Green Extensio	n Time	(ge),s				0.2			0.2			0.0			0.0
Phase Call Probability						1.00			0.88						
Max Out Proba	Max Out Probability					0.00			0.00						
	_			_											
Movement Gro	oup Res	sults		<u> </u>	EB	-		WB		<u> </u>	NB			SB	
Approach Move	ement				1	R	L		R			R		1	R
Assigned Move)			4	14	3	8	_		6		5	2	12
Adjusted Flow F	tion Flo), ven/n	In		30			150	1	-	1690		131		009
			m		1007			100	·		1009	·	1654		14.2
Queue Service		f(s), S			1.5			0.0		-	0.0	_	14.2		14.3
Green Ratio (a		e fille (<i>g c</i>), s			0.18			2.3	2		0.0		0.68		0.68
Green Kato (g	/0) /eh/h				286			358	,	-	1153		1176		1026
Volume-to-Can	acity Ra	tio (X)			0 127	•		0.16	7		0.000)	0.627		0.652
Back of Queue	(Q), ft	/In (95 th percentile	e)		29.1	-		43.9)		0.000	,	152.6		143.9
Back of Queue	(Q), ve	eh/In (95 th percent	, ile)		1.1			1.7			0.0		6.0		5.8
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00			0.00)		0.00		0.00		0.00
Uniform Delay ((d1), s	/veh			27.1			27.4	+		0.0		3.8		3.8
Incremental De	lay (d 2), s/veh			0.9			0.2			0.0		2.5		3.2
Initial Queue De	elay (d	з), s/veh			0.0			0.0			0.0		0.0		0.0
Control Delay (d), s/ve	eh			28.0			27.6	6		0.0		6.3		7.0
Level of Service	e (LOS)				С			С					Α		Α
Approach Delay	/, s/veh	/ LOS		28.0		С	27.6	6	С	0.	0		6.6		А
Intersection De	lay, s/ve	h / LOS				8	.0						A		
M 101 · · · ·														05	
Multimodal Re	sults	11.00		4.00	EB	D	4.00	WB			NB		4.00	SB	
Pedestrian LOS	Score	/ LUS		1.93		В	1.93		В	1.6	3	В	1.63		в
BICYCIE LOS SC	ore / LC	15		1.62		В	1.66		В	1.5	00	В	2.72		C

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General Inform	nation								Inte	rsecti	ion Inf	ormatic	on	4	ad _1_abs \$ \$	ι Ļ _k
Agency									Dura	ation,	h	0.250			↓↓ Þ	
Analvst				Analvs	is Date	e 4/12/2	023		Area	a Type	;	CBD		 		r. 4.
Jurisdiction				Time P	eriod	+			PHF	=		0.94			Image: second secon	
Urban Street		Madison		Analvs	is Yea	2023			Ana	Ivsis F	Period	1> 8:0	00	*		 ∵
Intersection		Madison & Locust		File Na	me	2 lane	- Madis	son 20)44 F	PM N.:	xus				+	
Project Descrip	tion	2044 PM		И										1	 ☆↑\$\$\$	۲ (^۳
· ·											11					
Demand Inform	nation				EB			W	В			NB			SB	1
Approach Move	ement			L	Т	R	L	<u>۲</u>	-	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				491	69	4	6	3			0		216	1306	182
0	<u></u>			1		-										
Signal Informa	tion				kt∳a	5										
Cycle, s	70.0	Reference Phase	2		ľ ↑	- R_ [⊮]							1	2	3	4
Offset, s	45	Reference Point	Begin	Green	46.6	15.4	0.0	0.0)	0.0	0.0					
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.0	3.0	0.0	0.0)	0.0	0.0	_		1		\rightarrow
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	0.0	0.0)	0.0	0.0		5	6	7	8
						EDT							NET	0.01	1	0.0.7
Timer Results				EBL		EBI	WB		VVE	31	NBI	-	NBI	SBL	- <u> </u>	SBI
Assigned Phase	9			<u> </u>		8	<u> </u>	_	4				6	<u> </u>	_	2
Case Number				<u> </u>	_	8.0		_	8.0	0			8.0	<u> </u>		7.0
Phase Duration	, S	`		<u> </u>				_	19.	.4		-	50.6	<u> </u>		50.6
Change Period,	(Y+R)	c), S				4.0		_	4.0	0			4.0			4.0
Max Allow Head	dway(<i>I</i>	MAH), s			_	4.0		-	4.0	0		_	0.0		_	0.0
Queue Clearan	ce lime	(gs), s				14.0		-	14.	.0			0.0		_	0.0
Green Extension Time (<i>g</i> e), s Phase Call Probability						1.4		-	1.0	3			0.0		_	0.0
Phase Call Probability						0.52		_	1.0	.0 .0		_			_	
Max Out Probability						0.53			0.5	03						
Movement Gro	up Res	ults			EB			WE	3			NB			SB	
Approach Move	- ment			L	Т	R	L	Т		R	L	Т	R	L	Т	R
Assigned Move	ment				8	18	7	4	+			6		5	2	12
Adjusted Flow F	Rate (v), veh/h			301	294		71				0		846	773	194
Adjusted Satura	ation Flo	w Rate (s), veh/h/l	In		1683	1636		153	9			1710		1553	1532	1449
Queue Service	Time (g	g s), s			10.9	12.0		0.0				0.0		26.2	23.7	3.6
Cycle Queue C	learance	e Time (<i>g c</i>), s			10.9	12.0		12.0)			0.0		27.7	23.7	3.6
Green Ratio (g	/C)				0.22	0.22		0.22	2			0.67		0.67	0.67	0.67
Capacity (c), v	reh/h				371	361		394	-			1138		1099	1019	964
Volume-to-Capa	acity Ra	itio(X)			0.813	0.816		0.18	1			0.000		0.770	0.759	0.201
Back of Queue	(Q), ft	/In (95 th percentile	e)		231.5	224.9		44.1	1			0		317.3	289.4	41.4
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		9.1	9.0		1.7				0.0		12.5	11.4	1.7
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00	0.00		0.00)			0.00		0.00	0.00	0.00
Uniform Delay ((d 1), si	/veh			25.9	25.9		23.3	3			0.0		8.4	7.8	4.5
Incremental De	lay (<i>d</i> 2), s/veh			8.0	8.4		0.1				0.0		5.2	5.3	0.5
Initial Queue De	elay (<i>d</i>	з), s/veh			0.0	0.0		0.0				0.0		0.0	0.0	0.0
Control Delay (d), s/ve	eh			33.9	34.4		23.4	1			0.0		13.6	13.1	4.9
Level of Service	e (LOS)				С	С		С						В	В	А
Approach Delay	, s/veh	/LOS		34.1		С	23.4	ł	С	;	0.0			12.5		В
Intersection De	lay, s/ve	h / LOS				18	8.0							B		
Multimodal Re	sults				EB			WE	3			NB			SB	
Pedestrian LOS	Score	/LOS		1.92		В	2.11		В		1.85	;	В	1.85		В
Bicycle LOS Sc	ore / LC	DS		2.05		В	0.61		A		1.56	5	В	3.06		С

			- 5								J					
General Inform	ency								Inter	rsecti	on Infe	ormatio	on	لير	4 4 4 t	l,⊾ l,⊾
Agency									Dura	ation. I	h	0.250			-4 ↓×	
Analyst				Analys	is Date	4/12/2	023	_	Area	Type		CBD		 24		た 本
Jurisdiction				Time F	Period	, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	020		PHF	:		0.94		→ \$- ``	w↓e	↓ ↓
Urban Street		Madison		Analys	is Year	2023			Anal	lvsis F	Period	1> 8.0	0			+ ₹
Intersection		Madison & Market		File Na	ame	2 lane	- Madis	son 2)44 P	PM N.>	us				+	
Project Descrip	tion	2044 PM												1	4 1 4 Y	t* (*
Demand Inform	nation				EB			W	B			NB			SB	
Approach Move	ement			L	Т	R	L	Т	-	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				137	119	27	11	9			0		13	1029	162
				li.												
Signal Informa	ition	1	r		<mark>ellis</mark> .		-									÷
Cycle, s	70.0	Reference Phase	2		ľ ↑	- R ≝							1	2	3	4
Offset, s	64	Reference Point	Begin	Green	43.5	15.9	0.0	0.0)	0.0	0.0					
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0)	0.0	0.0			1		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.0	2.2	0.0	0.0)	0.0	0.0		5	6	7	Y 8
			_							_						
Timer Results				EBL	·	EBT	WB		WB	BT	NBL	-	NBT	SBL	+	SBT
Assigned Phase	e					8		_	4	_		\rightarrow	6		+	2
Case Number						8.0		_	6.0)			8.0		+	8.0
Phase Duration	i, S	```				21.3		_	21.3	3			48.7		\rightarrow	48.7
Change Period	, (Y+R a	c), S				5.4		_	5.4	1			5.2		+	5.2
Max Allow Head	ax Allow Headway (<i>MAH</i>), s ueue Clearance Time (g_s), s			<u> </u>	_	4.3		_	4.3	5			0.0		+	0.0
Queue Clearan	ueue Clearance Time (g_s), s			<u> </u>		13.6		-	15.	5			0.0	<u> </u>	\rightarrow	0.0
Green Extensio	Green Extension Time (g e), s				_	0.7	<u> </u>	\rightarrow	0.4	+		\rightarrow	0.0	<u> </u>	+	0.0
Phase Call Probability			<u> </u>	_	1.00		_	1.00	0				<u> </u>	\rightarrow		
Max Out Proba	bility					1.00			1.00	0						
Movement Gro	oup Res	ults			EB			WE	3			NB			SB	
Approach Move	ement			L	Т	R	L	Т		R	L	Т	R	L	Т	R
Assigned Move	ment				8	18	7	4				6		5	2	12
Adjusted Flow I	Rate (v), veh/h			272		29	127	,			0		782		685
Adjusted Satura	ation Flo	w Rate (s), veh/h/l	In		1547		996	175	4			1922		1595		1381
Queue Service	Time (g	g s), S			11.6		1.9	4.2				0.0		0.0		26.1
Cycle Queue C	learance	e Time (g c), s			11.6		13.5	4.2				0.0		25.4		26.1
Green Ratio (g	/C)				0.23		0.23	0.23	3			0.62		0.62		0.62
Capacity (c), v	/eh/h				351		164	398	3			1195		1044		858
Volume-to-Cap	acity Ra	tio(X)			0.775		0.175	0.31	8			0.000	<u> </u>	0.749		0.798
Back of Queue	(Q), ft	/In (95 th percentile	e)		216.1		21.5	78.2	2			0		290.4		270
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		8.5		0.8	3.1				0.0		11.4		10.8
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00		0.00	0.00)			0.00		0.00		0.00
Uniform Delay	(d1), s/	/veh			25.5		31.7	22.8	3			0.0		9.9		10.0
Incremental De	lay (<i>d</i> 2), s/veh			8.6		0.4	0.3				0.0		3.2		4.9
Initial Queue De	elay (d	з), s/veh			0.0		0.0	0.0				0.0		0.0		0.0
Control Delay (d), s/ve	eh			34.1		32.1	23.1	1			0.0		13.0		14.9
Level of Service	e (LOS)				С		С	С						В		В
Approach Dela	y, s/veh	/LOS		34.1		С	24.7	7	С		0.0			13.9		В
Intersection De	lay, s/ve	h / LOS				17	'.7							В		
Multimodal Re	sults				EB			WE	3			NB			SB	
Pedestrian LOS	Score	/LOS		1.92		В	1.92	2	В		1.86	;	В	1.64		В
Bicycle LOS Sc	ore / LC	DS		2.01		В	1.82	2	В		1.56	5	В	2.62		С

			s e.g.				•			V	J					
General Inform	nation								Inters	secti	on Infe	ormatio	on	4	*	<u>ه لړ</u>
Agency	lution								Durat	ion	h	0 250			5 † †	
Analyst				Analys	is Date	- 4/12/2	023	_	Area -	Tvne		CBD		 		بر 4
lurisdiction				Time F		5 1/12/2	020		PHE	Type	, 	0.02		→ ->	N w∔e	< ↓ ↓
Urban Street		Madison		Analys	is Yea	r 2023			Analy	is F	Period	1> 8.0	00	- 7 -4 >		→ ¥ *
Intersection		Madison & Olive		File Na	me	2 Jane	- Madia	on 20		1 5 1		12 0.0				<u>_</u>
Project Descrip	tion						- Maula	5011 20	J44 I IX	vi 0.7	lus			-	া বা কিপা	- (*
T Toject Descrip	lion	2044110														
Demand Inform	nation				EB			W	B			NB			SB	
Approach Move	ement			L	Т	R	L	Т	-	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				89	48	10	3	2			0		54	1145	51
				h										_		
Signal Informa	ition						-									
Cycle, s	80.0	Reference Phase	2		ľ 🕇	- R °							1	2	3	
Offset, s	16	Reference Point	Begin	Green	56.1	12.5	0.0	0.0) ()	0.0	0.0	_		-		N ~
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0) 0	0.0	0.0			1		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.7	2.3	0.0	0.0) ()	0.0	0.0		5	6	7	8
Timer Results				EBL		EBT	WB	L	WBT	Г	NBL	-	NBT	SBL	-	SBT
Assigned Phase	e					4			8				6			2
Case Number						7.0			6.0				8.0			7.0
Phase Duration	, S					18.0			18.0				62.0			62.0
Change Period	, (Y+R a	c), S				5.5			5.5				5.9			5.9
Max Allow Head	dway(A	<i>ИАН</i>), s				4.2			4.2				0.0			0.0
Queue Clearan	ce Time	(gs),s				6.1			6.8							
Green Extension Time (g e), s					0.3			0.3				0.0			0.0	
Phase Call Probability					1.00			1.00								
Max Out Proba	bility					0.22			0.35							
Movement Gro		ulte			EB			\ \ /E	2			NB			SB	
Approach Move	mont	Suits			<u></u> т	R			, R	2	1	T	R		т	R
Assigned Move	ment				4	14	3	8	<u> </u>	`	-	6		5	2	12
Adjusted Flow F	Rate (v) veh/h			97	52	11	35				0		685	631	56
Adjusted Satura	ation Flo	w Rate (s), veh/h/	In		1678	1422	1169	178	9	+		1636		1686	1564	1456
Queue Service	Time (a	as). S			4.1	2.6	0.7	1.3	-			0.0		0.0	16.2	1.0
Cvcle Queue C	learance	e Time (a c), s			4.1	2.6	4.8	1.3	+	-		0.0		15.9	16.2	1.0
Green Ratio (o	/C)	· ····· (3·), -			0.16	0.16	0.16	0.16	3			0.70		0.70	0.70	0.70
Capacity (c), y	/eh/h				262	222	212	280)			1147		1231	1097	1021
Volume-to-Cap	acity Ra	tio(X)			0.369	0.235	0.051	0.12	4	T		0.000		0.556	0.575	0.055
Back of Queue	(Q), ft	/In (95 th percentile	e)		87.2	46	9.7	28.8	3			0		209	200	11.2
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		3.4	1.8	0.4	1.1				0.0		8.2	7.9	0.4
Queue Storage	Ratio (RQ) (95 th percen	tile)		0.00	0.58	0.00	0.00)			0.00		0.00	0.00	0.22
Uniform Delay	(d 1), s/	/veh			30.1	29.4	32.2	28.9	9			0.0		6.0	6.0	3.7
Incremental De	lay (<i>d</i> 2), s/veh			4.0	2.5	0.5	0.9				0.0		1.6	1.9	0.1
Initial Queue De	elay (d	3), s/veh			0.0	0.0	0.0	0.0				0.0		0.0	0.0	0.0
Control Delay (d), s/ve	eh			34.0	31.9	32.7	29.8	3			0.0		7.5	7.9	3.8
Level of Service	e (LOS)				С	C	С	С						Α	А	А
Approach Dela	y, s/veh	/LOS		33.3		С	30.5	5	С		0.0			7.6		А
Intersection De	lay, s/ve	h / LOS				10).7							В		
								•								
Multimodal Re	sults	11.00		1.00	EB	-	0.44	WE	5	4	4.0-	NB	_	1.0-	SB	_
Pedestrian LOS	Score	/ LOS		1.93		В	2.12	<u> </u>	В	_	1.85		В	1.85		В
BICYCLE LOS SC	ore / LC	15		1.81		В	1.63	5	В		1.56		В	2.68		C

			s eigi													
General Inform	nation				-				Inter	secti	on Infe	ormati	on	2		≜ L <u>a</u>
Agency									Dura	tion.	h	0.250)		7 † † ľ	
Analyst				Analys	is Dat	e 4/12/2	023		Area	Type		CBD		 		۲. ۲.
Jurisdiction				Time P	Period	0 1/ 12/2	020		PHF	1990	·	0.95		→ \$- ,	w‡e	↓ ↓ ↓
Urban Street		Madison		Analys	is Yea	r 2023		_	Analy	vsis F	Period	1> 8	າດ	4		+ +
Intersection		Madison & Washing	aton	File Na	ame	2 Jane	- Madis	son 20)44 PI	MS		1. 0.		- <u>-</u>	•	<u> </u>
Project Descrip	tion	2044 PM	gton				maan	5011 20	,,,,,	WI 0.7	luo			-	* 1 * * * 1	- r
r rojoot b ooonp		201111														
Demand Inform	nation				EB			W	В			NB			SB	
Approach Move	ement			L	Т	R	L	Т	-	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				139	9	66	25	2			0		64	1117	45
				11	- II:		ы		1							
Signal Informa	ition			-	k †≱	5										
Cycle, s	80.0	Reference Phase	2		Î ↑	Ē							1	2	3	
Offset, s	0	Reference Point	Begin	Green	44.4	24.6	0.0	0.0) (0.0	0.0					_
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.2	3.2	0.0	0.0) (0.0	0.0			1		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.4	2.2	0.0	0.0) [(0.0	0.0		5	6	7	8
T D K			_	EDI	_	EDT				T			NDT	0.01	_	ODT
Timer Results				EBL	-	EBI	WB		WB	-	NBL	-	NBI	SBI	-	SBT
Assigned Phase	e			<u> </u>	\rightarrow	4		\rightarrow	8	-			6	<u> </u>	_	2
Case Number				<u> </u>		8.0	<u> </u>	\rightarrow	6.0				8.0	<u> </u>		5.0
Phase Duration	i, S	```		<u> </u>	_	30.0		\rightarrow	30.0)			50.0	<u> </u>		50.0
Change Period	, (Y+R (c), S			+	5.4	<u> </u>	\rightarrow	5.4				5.6		_	5.6
Max Allow Head	dway(<i>I</i>	MAH), S			+	4.2	<u> </u>	\rightarrow	4.2			_	0.0			0.0
Queue Clearan		e (gs), s			+	8.2	<u> </u>	\rightarrow	12.0)			0.0		_	0.0
Green Extension Time (g ∈), s Phase Call Probability					+	1.7		\rightarrow	1.5				0.0			0.0
Phase Call Probability					+	1.00	<u> </u>	\rightarrow	1.00	-					_	
Max Out Proba	Max Out Probability					0.01			0.05	5						
Movement Gro	oup Res	sults			EB			WE	3			NB			SB	
Approach Move	ement			L	Т	R	L	Т	F	R	L	Т	R	L	Т	R
Assigned Move	ment				4	14	3	8				6		5	2	12
Adjusted Flow I	Rate (v), veh/h			156		69	265				0		73	1280	52
Adjusted Satura	ation Flo	w Rate (<i>s</i>), veh/h/l	In		1682		1105	186 ⁻	1			1689		1603	1670	1505
Queue Service	Time (g	g s), S			6.2		4.6	9.9				0.0		1.4	19.8	1.0
Cycle Queue C	learance	e Time (<i>g c</i>), s			6.2		10.6	9.9				0.0		1.4	19.8	1.0
Green Ratio (g	/C)				0.31	1	0.31	0.31	1			0.55	1	0.55	0.55	0.55
Capacity (c), v	/eh/h				517		344	572	2			937		980	1853	835
Volume-to-Cap	acity Ra	itio(X)			0.301		0.202	0.46	4			0.000		0.075	0.691	0.062
Back of Queue	(Q), ft	VIn (95 th percentile	e)		121.7	7	58.4	191.	7			0		21.1	230.2	14.6
Back of Queue	(Q), ve	eh/In (95 th percent	ile)		4.8		2.3	7.5				0.0		0.8	9.1	0.6
Queue Storage	Ratio (RQ) (95 th percent	tile)		0.00		0.90	0.00)			0.00		0.14	0.00	0.00
Uniform Delay	(d 1), si	/veh			24.0		28.4	25.5	5			0.0		6.7	10.0	6.6
Incremental De	lay (<i>d</i> 2), s/veh			1.5		0.8	1.7				0.0		0.1	1.5	0.1
Initial Queue De	elay(d	3), s/veh			0.0		0.0	0.0				0.0		0.0	0.0	0.0
Control Delay (d), s/ve	eh			25.5		29.3	27.2	2			0.0		6.8	11.5	6.7
Level of Service	e (LOS)				С		С	С						А	В	А
Approach Delay	y, s/veh	/ LOS		25.5		С	27.6	3	С		0.0			11.1		В
Intersection De	lay, s/ve	eh / LOS				15	5.2							В		
Multimodal Re	sults				EB			WE	3			NB	_		SB	_
Pedestrian LOS	Score	/ LOS		2.32		В	2.53	\$	C		2.06		В	1.88	5	В
Bicycle LOS Sc	ore / LC	JS		1.97		В	2.45)	В		2.13	5	В	3.18	5	С

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Lanes, Volumes, Timings 3: N Madison St/N Center St & W Locust St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		† ‡			र्स						-f1	7
Traffic Volume (vph)	0	415	52	4	46	0	0	0	0	148	942	134
Future Volume (vph)	0	415	52	4	46	0	0	0	0	148	942	134
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	0		0	0		150
Storage Lanes	0		0	0		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00
Ped Bike Factor		1.00									1.00	
Frt		0.983										0.850
Flt Protected					0.996						0.993	
Satd. Flow (prot)	0	3407	0	0	1820	0	0	0	0	0	3447	1553
Flt Permitted					0.951						0.993	
Satd. Flow (perm)	0	3407	0	0	1737	0	0	0	0	0	3445	1553
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		19										163
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		659			244			860			526	
Travel Time (s)		15.0			5.5			19.5			12.0	
Confl. Peds. (#/hr)			2							4		
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Adi, Flow (vph)	0	506	63	5	56	0	0	0	0	180	1149	163
Shared Lane Traffic (%)	•			Ţ		•	· ·	•	•			
Lane Group Flow (vph)	0	569	0	0	61	0	0	0	0	0	1329	163
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0		-0.1	0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane					10			10				
Headway Factor	1 00	1 00	1 00	1 00	1 00	1 00	1 00	1 00	1 00	1 00	1 00	1 00
Turning Speed (mph)	15	1.00	9	15			15	1.00		15		9
Number of Detectors	10	2	U	1	2	Ū	10		Ū	1	2	1
Detector Template		Thru		l eft	– Thru					l eft	Thru	Right
Leading Detector (ft)		100		20	100					20	100	20
Trailing Detector (ft)		0		0	0					0	0	0
Detector 1 Position(ft)		0		0	0					0	0	0 0
Detector 1 Size(ft)		6		20	6					20	6	20
Detector 1 Type		Cl+Ex		CI+Ex	CI+Ex					CI+Ex	Cl+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 2 Position(ft)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 2 Sizo(ft)		54 6			34 6						54	
Detector 2 Tupo												
Detector 2 Type												
Detector 2 Channel		0.0			0.0						0.0	
Detector 2 Extend (S)		0.0			0.0						0.0	

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 3: N Madison St/N Center St & W Locust St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type		NA		Perm	NA					Perm	NA	Perm
Protected Phases		4			8						6	
Permitted Phases				8						6		6
Detector Phase		4		8	8					6	6	6
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	5.0
Minimum Split (s)		23.4		22.5	22.5					23.4	23.4	23.4
Total Split (s)		24.0		22.5	22.5					46.0	46.0	46.0
Total Split (%)		34.3%		32.1%	32.1%					65.7%	65.7%	65.7%
Maximum Green (s)		18.6		18.0	18.0					40.6	40.6	40.6
Yellow Time (s)		3.2		3.5	3.5					3.2	3.2	3.2
All-Red Time (s)		2.2		1.0	1.0					2.2	2.2	2.2
Lost Time Adjust (s)		0.0			0.0						0.0	0.0
Total Lost Time (s)		5.4			4.5						5.4	5.4
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	3.0
Recall Mode		None		None	None					C-Max	C-Max	C-Max
Walk Time (s)		7.0		7.0	7.0					7.0	7.0	7.0
Flash Dont Walk (s)		11.0		11.0	11.0					11.0	11.0	11.0
Pedestrian Calls (#/hr)		0		0	0					0	0	0
Act Effct Green (s)		16.4			17.3						42.8	42.8
Actuated g/C Ratio		0.23			0.25						0.61	0.61
v/c Ratio		0.70			0.14						0.63	0.16
Control Delay		28.5			25.9						10.8	1.7
Queue Delay		0.0			0.0						0.0	0.0
Total Delay		28.5			25.9						10.8	1.7
LOS		С			С						В	Α
Approach Delay		28.5			25.9						9.8	
Approach LOS		С			С						А	
90th %ile Green (s)		18.6		19.5	19.5					40.6	40.6	40.6
90th %ile Term Code		Max		Hold	Hold					Coord	Coord	Coord
70th %ile Green (s)		18.6		19.5	19.5					40.6	40.6	40.6
70th %ile Term Code		Max		Hold	Hold					Coord	Coord	Coord
50th %ile Green (s)		17.2		18.1	18.1					42.0	42.0	42.0
50th %ile Term Code		Gap		Hold	Hold					Coord	Coord	Coord
30th %ile Green (s)		15.5		16.4	16.4					43.7	43.7	43.7
30th %ile Term Code		Gap		Hold	Hold					Coord	Coord	Coord
10th %ile Green (s)		12.1		13.0	13.0					47.1	47.1	47.1
10th %ile Term Code		Gap		Hold	Hold					Coord	Coord	Coord
Stops (vph)		395			51						651	12
Fuel Used(gal)		7			1						10	1
CO Emissions (g/hr)		509			45						731	46
NOx Emissions (g/hr)		99			9						142	9
VOC Emissions (g/hr)		118			10						169	11
Dilemma Vehicles (#)		0			0						0	0
Queue Length 50th (ft)		112			27						174	0
Queue Length 95th (ft)		141			m52						212	16
Internal Link Dist (ft)		579			164			780			446	

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 3: N Madison St/N Center St & W Locust St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												150
Base Capacity (vph)		919			483						2106	1013
Starvation Cap Reductn		0			0						0	0
Spillback Cap Reductn		0			0						0	0
Storage Cap Reductn		0			0						0	0
Reduced v/c Ratio		0.62			0.13						0.63	0.16
Intersection Summary												
Area Type: 0	Other											
Cycle Length: 70												
Actuated Cycle Length: 70												
Offset: 47 (67%), Referenced	d to phase	2: and 6:	SBTL, Sta	art of Gre	en							
Natural Cycle: 60												
Control Type: Actuated-Coor	dinated											
Maximum v/c Ratio: 0.70												
Intersection Signal Delay: 15	.2			In	tersectior	n LOS: B						
Intersection Capacity Utilizat	ion 52.6%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
m Volume for 95th percent	ile queue i	s metered	l by upstr	eam sign	al.							
Splits and Phases: 3: N M	adison St/I	N Center	St & W Lo	ocust St								20

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	24 s	
Ø6 (R)	₹Ø8	
46 s	22.5 s	

Lanes, Volumes, Timings <u>6: N East St/N Main St & W Locust St/E Locust St</u>

02/22/2024

Lane Group EBL EBT EBR WBL WBR NBL NBR SBL SBT Lane Configurations	SBR
Lane Configurations	
	•
Traffic Volume (vph) 83 478 0 0 0 0 51 792 93 0 0	0
Future Volume (vph) 83 478 0 0 0 0 51 792 93 0 0	0
Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 1900 190	1900
Lane Util, Factor 0.95 0.95 1.00 1.00 1.00 1.00 0.95 0.95 0.95 1.00 1.00	1.00
Ped Bike Factor 1.00	
Frt 0.985	
Fit Protected 0.993 0.997	
Satd. Flow (prot) 0 3447 0 0 0 0 0 3409 0 0 0	0
Flt Permitted 0.993 0.997	
Satd. Flow (perm) 0 3446 0 0 0 0 0 3409 0 0 0	0
Right Turn on Red Yes Yes Yes	Yes
Satd. Flow (RTOR) 27	
Link Speed (mph) 30 30 30 30	
Link Distance (ft) 244 514 878 525	
Travel Time (s) 5.5 11.7 20.0 11.9	
Confl. Peds. (#/hr) 2	
Peak Hour Factor 0.87 0.87 0.87 0.87 0.87 0.87 0.87 0.87	0.87
Heavy Vehicles (%) 4% 4% 4% 4% 4% 4% 4% 4% 4% 4% 4%	4%
Adi. Flow (vph) 95 549 0 0 0 0 59 910 107 0 0	0
Shared Lane Traffic (%)	•
Lane Group Flow (yph) $0.644 \ 0.0 \ 0.0 \ 0.1076 \ 0.0 \ 0.0$	0
Enter Blocked Intersection No No No No No No No No No No	No
Lane Alignment Left Left Right Left Right Left Right Left Right Left Right Left Right Left Left Right Right Left Right Ri	Right
Median Width(ff) 0 0 0 0 0	·
Link Offset(ff) 0 0 0 0	
Crosswalk Width(ft) 16 16 16	
Two way Left Turn Lane	
Headway Factor 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0	1.00
Turning Speed (mph) 15 9 15 9 15 9 15	9
Number of Detectors 1 2 1 2	
Detector Template Left Thru Left Thru	
Leading Detector (ft) 20 100 20 100	
Trailing Detector (ft) 0 0 0 0 0	
Detector 1 Position(ft) 0 0 0 0	
Detector 1 Size(ft) 20 6 20 6	
Detector 1 Type CI+Ex CI+Ex CI+Ex	
Detector 1 Channel	
Detector 1 Extend (s) 0.0 0.0 0.0 0.0	
Detector 1 Queue (s) 0.0 0.0 0.0 0.0	
Detector 1 Delay (s) 0.0 0.0 0.0 0.0	
Detector 2 Position(ft) 94 94	
Detector 2 Size(ft) 6 6	
Detector 2 Type CI+Ex CI+Ex	
Detector 2 Channel	
Detector 2 Extend (s) 0.0 0.0	
Turn Type Perm NA Perm NA	
Protected Phases 4 2	
Permitted Phases 4 2	

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings	
6: N East St/N Main St & W Locust St/E Locus	st St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4					2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0					5.0	5.0				
Minimum Split (s)	23.5	23.5					23.5	23.5				
Total Split (s)	26.0	26.0					44.0	44.0				
Total Split (%)	37.1%	37.1%					62.9%	62.9%				
Maximum Green (s)	20.5	20.5					38.5	38.5				
Yellow Time (s)	3.2	3.2					3.2	3.2				
All-Red Time (s)	2.3	2.3					2.3	2.3				
Lost Time Adjust (s)		0.0						0.0				
Total Lost Time (s)		5.5						5.5				
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0				
Recall Mode	None	None					C-Max	C-Max				
Walk Time (s)	7.0	7.0					7.0	7.0				
Flash Dont Walk (s)	11.0	11.0					11.0	11.0				
Pedestrian Calls (#/hr)	0	0					0	0				
Act Effct Green (s)	-	18.3					-	40.7				
Actuated g/C Ratio		0.26						0.58				
v/c Ratio		0.72						0.54				
Control Delay		13.2						6.8				
Queue Delav		0.3						0.0				
Total Delay		13.5						6.8				
LOS		В						A				
Approach Delay		13.5						6.8				
Approach LOS		В						A				
90th %ile Green (s)	20.5	20.5					38.5	38.5				
90th %ile Term Code	Max	Max					Coord	Coord				
70th %ile Green (s)	20.5	20.5					38.5	38.5				
70th %ile Term Code	Max	Max					Coord	Coord				
50th %ile Green (s)	19.7	19.7					39.3	39.3				
50th %ile Term Code	Gap	Gap					Coord	Coord				
30th %ile Green (s)	17.1	17.1					41.9	41.9				
30th %ile Term Code	Gap	Gap					Coord	Coord				
10th %ile Green (s)	13.6	13.6					45.4	45.4				
10th %ile Term Code	Gap	Gap					Coord	Coord				
Stops (vph)		177						552				
Fuel Used(gal)		4						11				
CO Emissions (g/hr)		248						752				
NOx Emissions (g/hr)		48						146				
VOC Emissions (g/hr)		58						174				
Dilemma Vehicles (#)		0						0				
Queue Length 50th (ft)		48						52				
Queue Length 95th (ft)		63						62				
Internal Link Dist (ft)		164			434			798			445	
Turn Bay Length (ft)												
Base Capacity (vph)		1009						1994				
Starvation Cap Reductn		66						0				

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings
6: N East St/N Main St & W Locust St/E Locust St

02/22/2024	
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Spillback Cap Reductn		0						0				
Storage Cap Reductn		0						0				
Reduced v/c Ratio		0.68						0.54				
Intersection Summary												
Area Type:	Other											
Cycle Length: 70												
Actuated Cycle Length: 70)											
Offset: 50 (71%), Referen	ced to phase	2:NBTL a	and 6:, St	art of Gre	en							
Natural Cycle: 50												
Control Type: Actuated-Co	oordinated											
Maximum v/c Ratio: 0.72												
Intersection Signal Delay:	9.3			In	tersection	n LOS: A						
Intersection Capacity Utiliz	zation 51.1%			IC	U Level o	of Service	A					
Analysis Period (min) 15												
Solits and Phases: 6: N	East St/N Ma	ain St & V	V Locust :	St/E Locu	ist St							

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44 s		26 s	

Lanes, Volumes, Timings 9: N Madison St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f,		7	†						đ î þ	
Traffic Volume (vph)	0	65	81	10	75	0	0	0	0	6	753	84
Future Volume (vph)	0	65	81	10	75	0	0	0	0	6	753	84
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	11	12	12	11	12	12	12	12	12	12	12
Grade (%)		1%			-2%			-6%			4%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		0.99		1.00								
Frt		0.925									0.985	
Flt Protected				0.950								
Satd. Flow (prot)	0	1447	0	1578	1605	0	0	0	0	0	3016	0
Flt Permitted				0.531								
Satd. Flow (perm)	0	1447	0	878	1605	0	0	0	0	0	3016	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		85									30	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		705			247			400			860	
Travel Time (s)		16.0			5.6			9.1			19.5	
Confl. Peds. (#/hr)			3	3								
Confl. Bikes (#/hr)			1									
Peak Hour Factor	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	88	109	14	101	0	0	0	0	8	1018	114
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	197	0	14	101	0	0	0	0	0	1140	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.15	1.20	1.15	1.13	1.18	1.13	1.10	1.10	1.10	1.17	1.17	1.17
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		Cl+Ex	CI+Ex					Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		Cl+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 9: N Madison St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		15.4		15.4	15.4					47.2	47.2	
Total Split (s)		23.0		23.0	23.0					47.0	47.0	
Total Split (%)		32.9%		32.9%	32.9%					67.1%	67.1%	
Maximum Green (s)		17.6		17.6	17.6					41.8	41.8	
Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)		2.2		2.2	2.2					2.0	2.0	
Lost Time Adjust (s)		0.0		0.0	0.0						0.0	
Total Lost Time (s)		5.4		5.4	5.4						5.2	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		None	None					C-Max	C-Max	
Walk Time (s)		1.0		1.0	1.0					31.0	31.0	
Flash Dont Walk (s)		9.0		9.0	9.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		11.2		11.2	11.2					•	48.2	
Actuated g/C Ratio		0.16		0.16	0.16						0.69	
v/c Ratio		0.65		0.10	0.39						0.55	
Control Delay		25.3		23.8	29.2						1.7	
Queue Delav		0.0		0.0	0.0						0.0	
Total Delay		25.3		23.8	29.2						1.7	
LOS		C		C	C						A	
Approach Delay		25.3		-	28.6						1.7	
Approach LOS		C			C						A	
90th %ile Green (s)		17.3		17.3	17.3					42.1	42.1	
90th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
70th %ile Green (s)		13.7		13.7	13.7					45.7	45.7	
70th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
50th %ile Green (s)		11.0		11.0	11.0					48.4	48.4	
50th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
30th %ile Green (s)		8.4		8.4	8.4					51.0	51.0	
30th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
10th %ile Green (s)		5.8		5.8	5.8					53.6	53.6	
10th %ile Term Code		Hold		Hold	Hold					Coord	Coord	
Stops (vph)		77		9	62						52	
Fuel Used(gal)		2		0	1						6	
CO Emissions (g/hr)		138		8	65						436	
NOx Emissions (a/hr)		27		2	13						85	
VOC Emissions (a/hr)		32		2	15						101	
Dilemma Vehicles (#)		0		0	0						0	
Queue Length 50th (ft)		45		5	40						16	
Queue Length 95th (ft)		69		15	59						16	
Internal Link Dist (ft)		625			167			320			780	

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 9: N Madison St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)		427		220	403						2084	
Starvation Cap Reductn		0		0	0						0	
Spillback Cap Reductn		0		0	0						0	
Storage Cap Reductn		0		0	0						0	
Reduced v/c Ratio		0.46		0.06	0.25						0.55	
Intersection Summary												
Area Type: Cl	3D											
Cycle Length: 70												
Actuated Cycle Length: 70												
Offset: 64 (91%), Referenced	to phase	2: and 6:8	SBTL, Sta	art of Gre	en							
Natural Cycle: 65												
Control Type: Actuated-Coord	inated											
Maximum v/c Ratio: 0.65												
Intersection Signal Delay: 7.0				In	tersectior	n LOS: A						
Intersection Capacity Utilization	on 44.6%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
Splits and Phases: 9: N Ma	dison St &	& W Mark	et St									

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	23 s	
Ø6 (R)	₩ Ø8	
47 s	23 s	

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	5			≜ t⊾		
Traffic Volume (vnh)	59	0	80	861	0	0
Future Volume (vph)	59	0	80	861	0	0
Ideal Flow (vphpl)	1000	1000	1000	1000	1000	1000
Lane Width (ft)	1300	12	100	11	1000	12
Crade (%)	10	12	12	20/	۲۲ ۵۷/	12
Lano I Itil Easter	1.00	1 00	0.05	-2%	2%	1 00
Lane Ulli. Facilui Dod Piko Eastor	1.00	1.00	0.95	0.90	1.00	1.00
				1.00		
FIL FIL Drotoctod	0.050			0.000		
Fit Protected	0.950	0	•	0.996	0	0
Satd. Flow (prot)	1467	0	0	2761	0	0
Fit Permitted	0.950			0.996		
Satd. Flow (perm)	1467	0	0	2760	0	0
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)						
Link Speed (mph)	30			30	30	
Link Distance (ft)	263			127	878	
Travel Time (s)	6.0			2.9	20.0	
Confl. Peds. (#/hr)	0.0		1		_0.0	
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	በ 82
Heavy Vehicles (%)	30/	3%	30/2	3%	3%	30/2
Parking (#/br)	570	J /0	20	20	J /0	J /0
Parking (#/nr)	70	0	20	20	0	0
Adj. Flow (vpn)	12	0	98	1050	0	0
Shared Lane Traffic (%)		-				
Lane Group Flow (vph)	72	0	0	1148	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	13			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane	-			-	-	
Headway Factor	1 25	1 14	1 13	1 34	1 16	1 16
Turning Speed (mph)	1.20	0	1.10	1.04	1.10	0 0
Number of Detectors	1	J	1	2		J
Number of Detectors	1.04		1	Z Thru		
Delector Template	Len		Len	inru		
Leading Detector (ft)	20		20	100		
I railing Detector (ft)	0		0	0		
Detector 1 Position(ft)	0		0	0		
Detector 1 Size(ft)	20		20	6		
Detector 1 Type	CI+Ex		Cl+Ex	CI+Ex		
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0		
Detector 1 Queue (s)	0.0		0.0	0.0		
Detector 1 Delay (s)	0.0		0.0	0.0		
Detector 2 Position/ft)	0.0		0.0	94		
Detector 2 Size(ft)				6		
Detector 2 Dize(II)						
Detector 2 Type				U+⊏X		
Detector 2 Channel						
Detector 2 Extend (s)				0.0		

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Turn Type	Prot		Perm	NA			
Protected Phases	4			2			
Permitted Phases	-		2				
Detector Phase	4		2	2			
Switch Phase	-						
Minimum Initial (s)	5.0		5.0	5.0			
Minimum Split (s)	13.4		49.5	49.5			
Total Split (s)	15.0		55.0	55.0			
Total Split (%)	21.4%		78.6%	78.6%			
Maximum Green (s)	9.6		49.5	49.5			
Yellow Time (s)	3.2		3.2	3.2			
All-Red Time (s)	2.2		2.3	2.3			
Lost Time Adjust (s)	0.0			0.0			
Total Lost Time (s)	5.4			5.5			
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)	3.0		3.0	3.0			
Recall Mode	None		C-Max	C-Max			
Walk Time (s)	1.0		33.0	33.0			
Flash Dont Walk (s)	7.0		11.0	11.0			
Pedestrian Calls (#/hr)	0		0	0			
Act Effct Green (s)	8.1			54.2			
Actuated g/C Ratio	0.12			0.77			
v/c Ratio	0.42			0.54			
Control Delay	36.1			5.4			
Queue Delay	0.0			0.0			
Total Delay	36.1			5.4			
LOS	D			А			
Approach Delay	36.1			5.4			
Approach LOS	D			А			
90th %ile Green (s)	9.6		49.5	49.5			
90th %ile Term Code	Max		Coord	Coord			
70th %ile Green (s)	9.6		49.5	49.5			
70th %ile Term Code	Max		Coord	Coord			
50th %ile Green (s)	8.7		50.4	50.4			
50th %ile Term Code	Gap		Coord	Coord			
30th %ile Green (s)	7.3		51.8	51.8			
30th %ile Term Code	Gap		Coord	Coord			
10th %ile Green (s)	0.0		64.5	64.5			
10th %ile Term Code	Skip		Coord	Coord			
Stops (vph)	53			361			
Fuel Used(gal)	1			4			
CO Emissions (g/hr)	59			277			
NOx Emissions (g/hr)	12			54			
VOC Emissions (g/hr)	14			64			
Dilemma Vehicles (#)	0			0			
Queue Length 50th (ft)	29			97			
Queue Length 95th (ft)	59			124			
Internal Link Dist (ft)	183			47	798		

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Turn Bay Length (ft)						
Base Capacity (vph)	201			2138		
Starvation Cap Reductn	0			0		
Spillback Cap Reductn	0			0		
Storage Cap Reductn	0			0		
Reduced v/c Ratio	0.36			0.54		
Intersection Summary						
Area Type:	CBD					
Cycle Length: 70						
Actuated Cycle Length: 70)					
Offset: 29 (41%), Reference	ced to phase 2	2:NBTL a	nd 6:, St	art of Gre	en	
Natural Cycle: 65						
Control Type: Actuated-Co	oordinated					
Maximum v/c Ratio: 0.54						
Intersection Signal Delay:	7.2			In	tersection	LOS: A
Intersection Capacity Utiliz	zation 42.3%			IC	U Level c	of Service A
Analysis Period (min) 15						
Solits and Phases: 11.1	N Fast St & F	Market S	t			

Ø2 (R)	 ▶ _{Ø4}	
55 s	 15 s	

Lanes, Volumes, Timings 12: N Madison St & W Monroe St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,			र्स						đ þ	
Traffic Volume (vph)	0	9	0	6	4	0	0	0	0	17	818	9
Future Volume (vph)	0	9	0	6	4	0	0	0	0	17	818	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	11	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			-3%			6%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt											0.998	
Flt Protected					0.970						0.999	
Satd. Flow (prot)	0	1605	0	0	1610	0	0	0	0	0	3051	0
Flt Permitted					0.970						0.999	
Satd. Flow (perm)	0	1605	0	0	1610	0	0	0	0	0	3051	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		699			209			307			400	
Travel Time (s)		15.9			4.8			7.0			9.1	
Confl. Peds. (#/hr)			3	3						4		1
Confl. Bikes (#/hr)			1									
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	0	12	0	8	5	0	0	0	0	22	1062	12
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	12	0	0	13	0	0	0	0	0	1096	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.19	1.14	1.14	1.14	1.14	1.12	1.12	1.12	1.19	1.19	1.19
Turning Speed (mph)	15	-	9	15	-	9	15		9	15		9
Sign Control		Stop			Stop			Stop			Free	
Intersection Summary												
Area Type: C	BD											
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 38.8%			IC	CU Level	of Service	A					
Analysis Period (min) 15												

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	3			≜ ta		
Traffic Volume (vph)	3	0	28	924	0	0
Future Volume (vph)	3	0	28	924	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	11	12	12
Grade (%)	-2%	12	12	-3%	4%	12
Lane I Itil Factor	1 00	1 00	0.95	0.95	1 00	1 00
Ped Rike Factor	1.00	1.00	0.00	1.00	1.00	1.00
Frt				1.00		
Flt Protected	0 950			0 000		
Satd Flow (prot)	1/3/	0	Ο	2783	0	0
Salu. Flow (pibl)	0.050	U	U	2100	U	U
Fit Permitted	0.950	0	^	0.999	0	0
Satu. Flow (perm)	1434	U	U	2183	U	U
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)						
Link Speed (mph)	30			30	30	
Link Distance (ft)	221			318	266	
Travel Time (s)	5.0			7.2	6.0	
Confl. Peds. (#/hr)			1			
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%
Parking (#/hr)	0		20	20		
Adj. Flow (vph)	4	0	34	1127	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	4	0	0	1161	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	l eft	Left	Right
Median Width(ft)	12	i ugin	Lon	0	0	i tigrit
Link Offeet(ft)	12			0	0	
Crocewalk Width/#	16			16	16	
	01			01	10	
I wo way Left Turn Lane	4.00	4.40	4.40	4.04	4 47	4 47
Headway Factor	1.29	1.13	1.12	1.34	1.17	1.17
Turning Speed (mph)	15	9	15	-		9
Number of Detectors	1		1	2		
Detector Template	Left		Left	Thru		
Leading Detector (ft)	20		20	100		
Trailing Detector (ft)	0		0	0		
Detector 1 Position(ft)	0		0	0		
Detector 1 Size(ft)	20		20	6		
Detector 1 Type	CI+Ex		CI+Ex	CI+Ex		
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0		
Detector 1 Queue (s)	0.0		0.0	0.0		
Detector 1 Delay (s)	0.0		0.0	0.0		
Detector 2 Position(ft)	0.0		0.0	0.0		
Detector 2 Fusition(it)				34		
Detector 2 Size(II)						
Detector 2 Type				UI+EX		
Detector 2 Channel						
Detector 2 Extend (s)				0.0		

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Turn Type	Prot		Perm	NA			
Protected Phases	4			2			
Permitted Phases			2	_			
Detector Phase	4		2	2			
Switch Phase			-	_			
Minimum Initial (s)	50		50	50			
Minimum Split (s)	13.2		49.2	49.2			
Total Split (s)	15.0		65.0	65.0			
Total Split (%)	18.8%		81.3%	81.3%			
Maximum Green (s)	9.8		59.8	59.8			
Vellow Time (s)	3.0		3.0	3.0			
All-Red Time (s)	2.0		2.0	2.0			
Lost Time Adjust (s)	2.0		2.0	2.0			
Total Lost Time (c)	0.0 5 0			5.2			
	0. Z			0.Z			
Load Lag Optimize?							
	2.0		2.0	20			
Venicle Extension (S)	J.U		S.U	S.U			
	INONE						
vvaik Time (s)	1.0		33.0	33.0			
Flash Dont Walk (S)	7.0		11.0	11.0			
Pedestrian Calls (#/nr)	0		0	0			
Act Effet Green (s)	5.9			/6.6			
Actuated g/C Ratio	0.07			0.96			
V/C Ratio	0.04			0.44			
Control Delay	35.0			0.5			
Queue Delay	0.0			0.0			
Total Delay	35.0			0.5			
LOS	С			A			
Approach Delay	35.0			0.5			
Approach LOS	С			A			
90th %ile Green (s)	6.6		63.0	63.0			
90th %ile Term Code	Gap		Coord	Coord			
70th %ile Green (s)	0.0		74.8	74.8			
70th %ile Term Code	Skip		Coord	Coord			
50th %ile Green (s)	0.0		74.8	74.8			
50th %ile Term Code	Skip		Coord	Coord			
30th %ile Green (s)	0.0		74.8	74.8			
30th %ile Term Code	Skip		Coord	Coord			
10th %ile Green (s)	0.0		74.8	74.8			
10th %ile Term Code	Skip		Coord	Coord			
Stops (vph)	5			2			
Fuel Used(gal)	0			2			
CO Emissions (g/hr)	4			172			
NOx Emissions (g/hr)	1			33			
VOC Emissions (a/hr)	1			40			
Dilemma Vehicles (#)	0			0			
Queue Length 50th (ff)	2			0			
Queue Lenath 95th (ft)	10			2			
Internal Link Dist (ft)	141			238	186		

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Turn Bay Length (ft)							
Base Capacity (vph)	175			2665			
Starvation Cap Reductn	0			0			
Spillback Cap Reductn	0			0			
Storage Cap Reductn	0			0			
Reduced v/c Ratio	0.02			0.44			
Intersection Summary							
Area Type:	CBD						
Cycle Length: 80							
Actuated Cycle Length: 80							
Offset: 60 (75%), Reference	ed to phase 2	2:NBTL a	nd 6:, St	art of Gre	en		
Natural Cycle: 65							
Control Type: Actuated-Co	ordinated						
Maximum v/c Ratio: 0.44							
Intersection Signal Delay: (0.6			In	tersectior	n LOS: A	
Intersection Capacity Utiliz	ation 42.1%			IC	U Level o	of Service A	
Analysis Period (min) 15							
Splits and Phases: 13: N	I East St & M	onroe St					

Ø2 (R)	<u></u>	
65 s	15 s	

Lanes, Volumes, Timings 15: N Madison St & W Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1.			र्स						4 P	
Traffic Volume (vph)	0	15	4	4	12	0	0	0	0	12	817	11
Future Volume (vph)	0	15	4	4	12	0	0	0	0	12	817	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	16	12	12	14	12	12	12	12	12	12	12
Grade (%)		4%			-3%			2%			2%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		1.00			1.00							
Frt		0.973									0.998	
Flt Protected					0.988						0.999	
Satd. Flow (prot)	0	1787	0	0	1598	0	0	0	0	0	3113	0
Flt Permitted					0.952						0.999	
Satd. Flow (perm)	0	1787	0	0	1537	0	0	0	0	0	3113	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		5									3	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		692			253			310			307	
Travel Time (s)		15.7			5.8			7.0			7.0	
Confl. Peds. (#/hr)			3	3								
Peak Hour Factor	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Parking (#/hr)					0					20		20
Adj. Flow (vph)	0	20	5	5	16	0	0	0	0	16	1075	14
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	25	0	0	21	0	0	0	0	0	1105	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.17	1.00	1.17	1.12	1.18	1.12	1.16	1.16	1.16	1.16	1.16	1.16
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		Cl+Ex	CI+Ex					Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		CI+Ex			Cl+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 15: N Madison St & W Jefferson St

Lane Group EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR Turn Type NA Perm NA Perm NA Perm NA Protected Phases 4 8 6 6 6 Permitted Phases 8 6 6 6 6 Detector Phase 4 8 8 6 6 Switch Phase 5.0 5.0 5.0 5.0 5.0 Minimum Initial (s) 5.0 5.0 5.0 5.0 5.0 Minimum Split (s) 15.1 15.4 15.4 47.4 47.4 Total Split (s) 21.0 21.0 21.0 59.0 59.0 Total Split (%) 26.3% 26.3% 26.3% 73.8% 73.8%
Turn Type NA Perm NA Protected Phases 4 8 6 Permitted Phases 8 6 Detector Phase 4 8 6 Switch Phase 4 8 6 Minimum Initial (s) 5.0 5.0 5.0 Minimum Split (s) 15.1 15.4 15.4 Total Split (s) 21.0 21.0 21.0 Total Split (%) 26.3% 26.3% 73.8%
Protected Phases 4 8 6 Permitted Phases 8 6 Detector Phase 4 8 8 6 Switch Phase 4 8 8 6 6 Minimum Initial (s) 5.0 5.0 5.0 5.0 5.0 Minimum Split (s) 15.1 15.4 15.4 47.4 47.4 Total Split (s) 21.0 21.0 21.0 59.0 59.0 Total Split (%) 26.3% 26.3% 26.3% 73.8% 73.8%
Permitted Phases 8 6 Detector Phase 4 8 8 6 Switch Phase
Detector Phase 4 8 8 6 6 Switch Phase
Switch Phase Minimum Initial (s) 5.0 5.0 5.0 Minimum Split (s) 15.1 15.4 15.4 47.4 Total Split (s) 21.0 21.0 21.0 59.0 Total Split (%) 26.3% 26.3% 26.3% 73.8%
Minimum Initial (s)5.05.05.05.0Minimum Split (s)15.115.415.447.447.4Total Split (s)21.021.021.059.059.0Total Split (%)26.3%26.3%26.3%73.8%73.8%
Minimum Split (s)15.115.415.447.447.4Total Split (s)21.021.021.059.059.0Total Split (%)26.3%26.3%26.3%73.8%73.8%
Total Split (s) 21.0 21.0 21.0 59.0 59.0 Total Split (%) 26.3% 26.3% 26.3% 73.8% 73.8%
Total Split (%) 26.3% 26.3% 26.3% 73.8% 73.8%
Maximum Green (s) 15.9 15.6 15.6 53.6 53.6
Yellow Time (s) 3.2 3.2 3.2 3.2 3.2
All-Red Time (s) 1.9 2.2 2.2 2.2 2.2
Lost Time Adjust (s) 0.0 0.0 0.0
Total Lost Time (s) 5.1 5.4 5.4
Lead/Lag
Lead-Lag Optimize?
Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0
Recall Mode Max None None C-Max C-Max
Walk Time (s) 1.0 1.0 1.0 31.0 31.0
Flash Dont Walk (s) 9.0 9.0 9.0 11.0 11.0
Pedestrian Calls (#/hr) 0 0 0 0 0 0
Act Effct Green (s) 15.9 15.6 53.6
Actuated g/C Ratio 0.20 0.20 0.67
v/c Ratio 0.07 0.07 0.53
Control Delay 23.1 27.1 7.9
Queue Delay 0.0 0.0 0.0
Total Delay 23.1 27.1 7.9
LOS C C A
Approach Delay 23.1 27.1 7.9
Approach LOS C C A
90th %ile Green (s) 15.9 15.6 15.6 53.6 53.6
90th %ile Term Code MaxR Hold Hold Coord Coord
70th %ile Green (s) 15.9 15.6 15.6 53.6 53.6
70th %ile Term Code MaxR Hold Hold Coord Coord
50th %ile Green (s) 15.9 15.6 15.6 53.6 53.6
50th %ile Term Code MaxR Hold Hold Coord Coord
30th %ile Green (s) 15.9 15.6 15.6 53.6 53.6
30th %ile Term Code MaxR Hold Hold Coord Coord
10th %ile Green (s) 15.9 15.6 15.6 53.6 53.6
10th %ile Term Code MaxR Hold Hold Coord Coord
Stops (vph) 14 15 392
Fuel Used(gal) 0 0 6
CO Emissions (a/hr) 19 14 386
NOx Emissions (g/hr) 4 3 75
VOC Emissions (g/hr) 4 3 89
Dilemma Vehicles (#) 0 0 0
Queue Length 50th (ft) 8 9 127
Queue Length 95th (ft) 23 23 130
Internal Link Dist (ft) 612 173 230 227

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE
Lanes, Volumes, Timings 15: N Madison St & W Jefferson St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)		359			299						2086	
Starvation Cap Reductn		0			0						0	
Spillback Cap Reductn		0			0						0	
Storage Cap Reductn		0			0						0	
Reduced v/c Ratio		0.07			0.07						0.53	
Intersection Summary												
Area Type: C	BD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 10 (13%), Referenced	to phase	2: and 6:8	SBTL, Sta	art of Gre	en							
Natural Cycle: 65												
Control Type: Actuated-Coord	linated											
Maximum v/c Ratio: 0.53												
Intersection Signal Delay: 8.6				In	tersection	n LOS: A						
Intersection Capacity Utilization	on 43.0%			IC	U Level c	of Service	A					
Analysis Period (min) 15												
Splits and Phases: 15: N M	adison St	& W Jeffe	erson St									

	→ Ø4
	21 s
▼ Ø6 (R)	Ø8
59 s	21 s

Lanes, Volumes, Timings 16: N East St & E Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्स			f,			đ þ				
Traffic Volume (vph)	6	23	0	0	11	4	21	947	26	0	0	0
Future Volume (vph)	6	23	0	0	11	4	21	947	26	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	11	11	12	12	12
Grade (%)		-3%			3%			0%			3%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor		0.99			0.99			1.00				
Frt					0.962			0.996				
Flt Protected		0.990						0.999				
Satd. Flow (prot)	0	1501	0	0	1602	0	0	3032	0	0	0	0
Flt Permitted		0.952						0.999				
Satd. Flow (perm)	0	1432	0	0	1602	0	0	3031	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					5			8				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		268			710			308			318	
Travel Time (s)		6.1			16.1			7.0			7.2	
Confl. Peds. (#/hr)	17					17	8		8			
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Parking (#/hr)		0					20		20			
Adj. Flow (vph)	7	28	0	0	13	5	25	1141	31	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	35	0	0	18	0	0	1197	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.12	1.28	1.12	1.17	1.12	1.17	1.14	1.19	1.19	1.17	1.17	1.17
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2			2		1	2				
Detector Template	Left	Thru			Thru		Left	Thru				
Leading Detector (ft)	20	100			100		20	100				
Trailing Detector (ft)	0	0			0		0	0				_
Detector 1 Position(ft)	0	0			0		0	0				
Detector 1 Size(ft)	20	6			6		20	6				
Detector 1 Type	CI+Ex	CI+Ex			CI+Ex		CI+Ex	CI+Ex				
Detector 1 Channel		0.0			• •		0.0	• •				_
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0				
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 16: N East St & E Jefferson St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA			NA		Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4						2					
Detector Phase	4	4			8		2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0		5.0	5.0				
Minimum Split (s)	15.2	15.2			15.2		47.2	47.2				
Total Split (s)	18.0	18.0			18.0		62.0	62.0				
Total Split (%)	22.5%	22.5%			22.5%		77.5%	77.5%				
Maximum Green (s)	12.8	12.8			12.8		56.8	56.8				
Yellow Time (s)	3.2	3.2			3.2		3.2	3.2				
All-Red Time (s)	2.0	2.0			2.0		2.0	2.0				
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.2			5.2			5.2				
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0				
Recall Mode	Max	Max			None		C-Max	C-Max				
Walk Time (s)	1.0	1.0			1.0		31.0	31.0				
Flash Dont Walk (s)	9.0	9.0			9.0		11.0	11.0				
Pedestrian Calls (#/hr)	0	0			0		0	0				
Act Effct Green (s)		12.8			12.8			56.8				
Actuated g/C Ratio		0.16			0.16			0.71				
v/c Ratio		0.15			0.07			0.56				
Control Delay		30.9			24.6			2.2				
Queue Delav		0.0			0.0			0.0				
Total Delay		30.9			24.6			2.2				
LOS		С			C			А				
Approach Delay		30.9			24.6			2.2				
Approach LOS		С			C			А				
90th %ile Green (s)	12.8	12.8			12.8		56.8	56.8				
90th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
70th %ile Green (s)	12.8	12.8			12.8		56.8	56.8				
70th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
50th %ile Green (s)	12.8	12.8			12.8		56.8	56.8				
50th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
30th %ile Green (s)	12.8	12.8			12.8		56.8	56.8				
30th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
10th %ile Green (s)	12.8	12.8			12.8		56.8	56.8				
10th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
Stops (vph)		27			13			78				
Fuel Used(gal)		0			0			3				
CO Emissions (a/hr)		27			16			227				
NOx Emissions (a/hr)		5			3			44				
VOC Emissions (g/hr)		6			4			53				
Dilemma Vehicles (#)		0			0			0				
Queue Length 50th (ft)		15			6			24				
Queue Length 95th (ft)		37			21			34				
Internal Link Dist (ft)		188			630			228			238	_

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 16: N East St & E Jefferson St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)		229			260			2154				
Starvation Cap Reductn		0			0			93				
Spillback Cap Reductn		0			0			0				
Storage Cap Reductn		0			0			0				
Reduced v/c Ratio		0.15			0.07			0.58				
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 56 (70%), Reference	ed to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 65												
Control Type: Actuated-Coc	ordinated											
Maximum v/c Ratio: 0.56												
Intersection Signal Delay: 3	.3			In	tersectior	n LOS: A						
Intersection Capacity Utiliza	ation 50.8%			IC	CU Level o	of Service	А					
Analysis Period (min) 15												

Splits and Phases: 16: N East St & E Jefferson St

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62 s	18 s
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	18 s

Lanes, Volumes, Timings 19: N Madison St & W Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f,		5	1					5	≜ †₽	
Traffic Volume (vph)	0	97	9	34	188	0	0	0	0	37	765	22
Future Volume (vph)	0	97	9	34	188	0	0	0	0	37	765	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	13	12	12	13	12	12	12	12	12	12	12
Grade (%)		3%			-3%			2%			-2%	
Storage Length (ft)	0		0	65		0	0		0	150		0
Storage Lanes	0		0	1		0	0		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95
Frt		0.988									0.996	
Flt Protected				0.950						0.950		
Satd. Flow (prot)	0	1488	0	1585	1552	0	0	0	0	1262	3143	0
Flt Permitted				0.670						0.950		
Satd. Flow (perm)	0	1488	0	1118	1552	0	0	0	0	1262	3143	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		7									5	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		690			253			324			310	
Travel Time (s)		15.7			5.8			7.4			7.0	
Peak Hour Factor	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)		0			0					20		20
Adj. Flow (vph)	0	124	12	44	241	0	0	0	0	47	981	28
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	136	0	44	241	0	0	0	0	47	1009	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12	Ŭ		12	J		12	Ū		12	Ū
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane					Yes							
Headway Factor	1.17	1.28	1.17	1.12	1.23	1.12	1.16	1.16	1.16	1.49	1.13	1.13
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		Cl+Ex	CI+Ex					CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		CI+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 19: N Madison St & W Washington St

02/22/	2024
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Lane Group	EBL EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0			0.0						0.0	
Turn Type	NA		Perm	NA					Perm	NA	
Protected Phases	4			8						6	
Permitted Phases			8						6		
Detector Phase	4		8	8					6	6	
Switch Phase											
Minimum Initial (s)	5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)	15.4		25.4	25.4					37.6	37.6	
Total Split (s)	33.0		33.0	33.0					47.0	47.0	
Total Split (%)	41.3%		41.3%	41.3%					58.8%	58.8%	
Maximum Green (s)	27.6		27.6	27.6					41.4	41.4	
Yellow Time (s)	3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)	2.2		2.2	2.2					2.4	2.4	
Lost Time Adjust (s)	0.0		0.0	0.0					0.0	0.0	
Total Lost Time (s)	5.4		5.4	5.4					5.6	5.6	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)	3.0		3.0	3.0					3.0	3.0	
Recall Mode	None		None	None					C-Max	C-Max	
Walk Time (s)	1.0		9.0	9.0					21.0	21.0	
Flash Dont Walk (s)	9.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)	0		0	0					0	0	
Act Effct Green (s)	17.9		17.9	17.9					51.1	51.1	
Actuated g/C Ratio	0.22		0.22	0.22					0.64	0.64	
v/c Ratio	0.40		0.18	0.70					0.06	0.50	
Control Delay	27.3		22.0	36.3					7.1	8.2	
Queue Delay	0.0		0.0	0.1					0.0	0.2	
Total Delay	27.3		22.0	36.4					7.1	8.3	
LOS	С		С	D					А	А	
Approach Delay	27.3			34.2						8.3	
Approach LOS	С			С						А	
90th %ile Green (s)	25.1		25.1	25.1					43.9	43.9	
90th %ile Term Code	Hold		Gap	Gap					Coord	Coord	
70th %ile Green (s)	20.6		20.6	20.6					48.4	48.4	
70th %ile Term Code	Hold		Gap	Gap					Coord	Coord	
50th %ile Green (s)	17.8		17.8	17.8					51.2	51.2	
50th %ile Term Code	Hold		Gap	Gap					Coord	Coord	
30th %ile Green (s)	14.9		14.9	14.9					54.1	54.1	
30th %ile Term Code	Hold		Gap	Gap					Coord	Coord	
10th %ile Green (s)	10.9		10.9	10.9					58.1	58.1	
10th %ile Term Code	Hold		Gap	Gap					Coord	Coord	
Stops (vph)	80		15	90					13	288	
Fuel Used(gal)	2		0	2					0	5	
CO Emissions (g/hr)	112		21	158					15	336	
NOx Emissions (g/hr)	22		4	31					3	65	
VOC Emissions (g/hr)	26		5	37					3	78	
Dilemma Vehicles (#)	0		0	0					0	0	
Queue Length 50th (ft)	55		11	64					8	111	
Queue Length 95th (ft)	79		17	62					m19	117	

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 19: N Madison St & W Washington St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		610			173			244			230	
Turn Bay Length (ft)				65						150		
Base Capacity (vph)		517		385	535					806	2010	
Starvation Cap Reductn		0		0	26					0	277	
Spillback Cap Reductn		12		0	0					0	0	
Storage Cap Reductn		0		0	0					0	0	
Reduced v/c Ratio		0.27		0.11	0.47					0.06	0.58	
Intersection Summary												
Area Type: Cl	BD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 79 (99%), Referenced	to phase	2: and 6:8	SBTL, Sta	art of Gree	en							
Natural Cycle: 65												
Control Type: Actuated-Coord	linated											
Maximum v/c Ratio: 0.70												
Intersection Signal Delay: 15.0)			In	tersectior	n LOS: B						
Intersection Capacity Utilization	on 44.4%			IC	U Level o	of Service	A					
Analysis Period (min) 15												
m Volume for 95th percentile	e queue is	s metered	by upstr	eam sign	al.							

Splits and Phases: 19: N Madison St & W Washington St

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	33 s	
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47 s	33 s	

Lanes, Volumes, Timings 20: N East St & E Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	1			•	1	2	≜ ↑₽				
Traffic Volume (vph)	10	171	0	0	236	77	23	899	53	0	0	0
Future Volume (vph)	10	171	0	0	236	77	23	899	53	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	11	10	11	11	11	12	12	12
Grade (%)		-3%			2%			-3%			1%	
Storage Length (ft)	65		0	0		80	80		0	0		0
Storage Lanes	1		0	0		1	1		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	1.00					0.98	1.00	1.00				
Frt						0.850		0.992				
Flt Protected	0.950						0.950					
Satd. Flow (prot)	1585	1502	0	0	1574	1291	1226	3037	0	0	0	0
Flt Permitted	0.388						0.950					
Satd. Flow (perm)	645	1502	0	0	1574	1266	1220	3037	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)						75		12				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		262			724			322			308	
Travel Time (s)		6.0			16.5			7.3			7.0	
Confl. Peds. (#/hr)	5					5	5		2			
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)		0					20		20			
Adj. Flow (vph)	13	214	0	0	295	96	29	1124	66	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	13	214	0	0	295	96	29	1190	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			11			11	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	_
I wo way Left I urn Lane	4.40	Yes	4.40		Yes	4.00						
Headway Factor	1.12	1.28	1.12	1.16	1.21	1.26	1.54	1.17	1.1/	1.15	1.15	1.15
Turning Speed (mph)	15	0	9	15	<u>^</u>	9	15	0	9	15		9
Number of Detectors	1	2			2	1	1	2				
Detector Template	Left	I hru			I hru	Right	Left	I hru				
Leading Detector (ft)	20	100			100	20	20	100				
Trailing Detector (ft)	0	0			0	0	0	0				
Detector 1 Position(ft)	0	0			0	0	0	0				
Detector 1 Size(ft)	20	01.5.			0	20	20	0				
Detector 1 Type	CI+EX	CI+EX			CI+EX	CI+EX	CI+EX	CI+EX				
Detector 1 Channel	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Extend (S)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Queue (S)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Delay (S)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 2 Position(It)		94			94			94				
Detector 2 Size(ft)		6			6			6				

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 20: N East St & E Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA	Perm	Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4					8	2					
Detector Phase	4	4			8	8	2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0	5.0	5.0	5.0				
Minimum Split (s)	31.4	31.4			21.4	21.4	47.0	47.0				
Total Split (s)	29.0	29.0			29.0	29.0	51.0	51.0				
Total Split (%)	36.3%	36.3%			36.3%	36.3%	63.8%	63.8%				
Maximum Green (s)	23.6	23.6			23.6	23.6	45.0	45.0				
Yellow Time (s)	3.2	3.2			3.2	3.2	3.2	3.2				
All-Red Time (s)	2.2	2.2			2.2	2.2	2.8	2.8				
Lost Time Adjust (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Total Lost Time (s)	5.4	5.4			5.4	5.4	6.0	6.0				
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0	3.0	3.0	3.0				
Recall Mode	None	None			None	None	C-Max	C-Max				
Walk Time (s)	15.0	15.0			5.0	5.0	30.0	30.0				
Flash Dont Walk (s)	11.0	11.0			11.0	11.0	11.0	11.0				
Pedestrian Calls (#/hr)	0	0			0	0	0	0				
Act Effct Green (s)	19.5	19.5			19.5	19.5	49.1	49.1				
Actuated g/C Ratio	0.24	0.24			0.24	0.24	0.61	0.61				
v/c Ratio	0.08	0.59			0.77	0.26	0.04	0.64				
Control Delay	29.0	29.2			41.7	9.9	3.7	6.3				
Queue Delay	0.0	0.2			0.0	0.0	0.0	0.1				
Total Delay	29.0	29.3			41.7	9.9	3.7	6.4				
LOS	С	С			D	А	А	А				
Approach Delay		29.3			33.9			6.3				
Approach LOS		С			С			А				
90th %ile Green (s)	23.6	23.6			23.6	23.6	45.0	45.0				
90th %ile Term Code	Hold	Hold			Max	Max	Coord	Coord				
70th %ile Green (s)	23.6	23.6			23.6	23.6	45.0	45.0				
70th %ile Term Code	Hold	Hold			Max	Max	Coord	Coord				
50th %ile Green (s)	20.1	20.1			20.1	20.1	48.5	48.5				
50th %ile Term Code	Hold	Hold			Gap	Gap	Coord	Coord				
30th %ile Green (s)	17.2	17.2			17.2	17.2	51.4	51.4				
30th %ile Term Code	Hold	Hold			Gap	Gap	Coord	Coord				
10th %ile Green (s)	12.8	12.8			12.8	12.8	55.8	55.8				
10th %ile Term Code	Hold	Hold			Gap	Gap	Coord	Coord				
Stops (vph)	8	89			211	22	3	379				
Fuel Used(gal)	0	2			5	1	0	6				
CO Emissions (g/hr)	9	130			315	50	6	399				
NOx Emissions (g/hr)	2	25			61	10	1	78				
VOC Emissions (g/hr)	2	30			73	12	1	92				
Dilemma Vehicles (#)	0	0			0	0	0	0				

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 20: N East St & E Washington St

02/22/20	24
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)	3	43			136	8	2	45				
Queue Length 95th (ft)	16	113			180	33	m5	54				
Internal Link Dist (ft)		182			644			242			228	
Turn Bay Length (ft)	65					80	80					
Base Capacity (vph)	190	443			464	426	749	1869				
Starvation Cap Reductn	0	20			0	0	0	49				
Spillback Cap Reductn	0	0			0	0	0	0				
Storage Cap Reductn	0	0			0	0	0	0				
Reduced v/c Ratio	0.07	0.51			0.64	0.23	0.04	0.65				
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 64 (80%), Reference	d to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 80												
Control Type: Actuated-Coo	rdinated											
Maximum v/c Ratio: 0.77												
Intersection Signal Delay: 15	5.0			In	tersectior	LOS: B						
Intersection Capacity Utilizat	tion 59.3%			IC	U Level o	of Service	В					
Analysis Period (min) 15												
m Volume for 95th percent	tile queue i	s metered	l by upstr	eam sign	al.							

Splits and Phases: 20: N East St & E Washington St

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51 s	29 s	
3.00.93	*	10 million (1986)
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Lanes, Volumes, Timings 23: S Center St/N Madison St & W Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		¢Î,		2	*						đ þ	
Traffic Volume (vph)	0	60	15	21	31	0	0	0	0	58	704	33
Future Volume (vph)	0	60	15	21	31	0	0	0	0	58	704	33
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	13	12	12	12	12	12	12	12	12
Grade (%)		3%			-3%			1%			-1%	
Storage Length (ft)	0		0	60		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	0		0
Taper Length (ft)	25			0			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		0.99		0.98							1.00	
Frt		0.973									0.994	
Flt Protected				0.950							0.996	
Satd. Flow (prot)	0	1394	0	1623	1488	0	0	0	0	0	3075	0
Flt Permitted				0.692							0.996	
Satd. Flow (perm)	0	1394	0	1155	1488	0	0	0	0	0	3075	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		16									9	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		684			247			660			324	
Travel Time (s)		15.5			5.6			15.0			7.4	
Confl. Peds. (#/hr)			11	11						3		5
Peak Hour Factor	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%
Parking (#/hr)		0			0					20		20
Adj. Flow (vph)	0	80	20	28	41	0	0	0	0	77	939	44
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	100	0	28	41	0	0	0	0	0	1060	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		13			13			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.17	1.33	1.17	1.08	1.28	1.12	1.15	1.15	1.15	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		CI+Ex		CI+Ex	CI+Ex					Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 23: S Center St/N Madison St & W Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Type		Cl+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	
Permitted Phases				8	-					6	-	
Detector Phase		4		8	8					6	6	
Switch Phase				-							-	
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		15.5		25.5	25.5					37.7	37.7	
Total Split (s)		30.0		30.0	30.0					50.0	50.0	
Total Split (%)		37.5%		37.5%	37.5%					62.5%	62.5%	
Maximum Green (s)		24.5		24.5	24.5					44.3	44.3	
Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)		2.3		2.3	2.3					2.5	2.5	
Lost Time Adjust (s)		0.0		0.0	0.0						0.0	
Total Lost Time (s)		5.5		5.5	5.5						5.7	
Lead/Lag		0.0		0.0	0.0						0.1	
Lead-Lag Optimize?												
Vehicle Extension (s)		30		30	30					30	30	
Recall Mode		None		None	None					C-Max	C-Max	
Walk Time (s)		1.0		9.0	9.0					21.0	21.0	
Flash Dont Walk (s)		9.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		10.2		10.2	10.2					Ű	61.9	
Actuated g/C Ratio		0.13		0.13	0.13						0 77	
v/c Ratio		0.52		0.19	0.22						0.45	
Control Delay		36.2		32.3	32.2						1 1	
Queue Delay		0.0		0.0	0.0						0.0	
Total Delay		36.2		32.3	32.2						1 1	
		D		C	C						Δ	
Approach Delay		36.2		Ũ	32.3						11	
Approach LOS		D			C						Α	
90th %ile Green (s)		15.1		15 1	15.1					53 7	53.7	
90th %ile Term Code		Gan		Hold	Hold					Coord	Coord	
70th %ile Green (s)		12.2		12.2	12.2					56.6	56.6	
70th %ile Term Code		Gan		Hold	Hold					Coord	Coord	
50th %ile Green (s)		10.2		10.2	10.2					58.6	58.6	
50th %ile Term Code		Gan		Hold	Hold					Coord	Coord	
30th %ile Green (s)		8.2		8.2	8.2					60.6	60.6	
30th %ile Term Code		Gan		Hold	Hold					Coord	Coord	
10th %ile Green (s)		0.0		0.0	0.0					74.3	74 3	
10th %ile Term Code		Skin		Skip	Skin					Coord	Coord	
Stops (vph)		57		20	28					00010	35	
Fuel Used(gal)		1		_0	0						_2	
CO Emissions (a/br)		80		20	29						166	
NOv Emissions (g/hr)		17		20	6						32	
VOC Emissions (g/hr)		21		т 5	7						20	
Dilemma Vehicles (#)				0	0						0	
		0		0	0						0	

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 23: S Center St/N Madison St & W Front St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)		40		13	19						11	
Queue Length 95th (ft)		65		28	36						12	
Internal Link Dist (ft)		604			167			580			244	
Turn Bay Length (ft)				60								
Base Capacity (vph)		438		353	455						2381	
Starvation Cap Reductn		0		0	0						20	
Spillback Cap Reductn		0		0	0						0	
Storage Cap Reductn		0		0	0						0	
Reduced v/c Ratio		0.23		0.08	0.09						0.45	
Intersection Summary												
Area Type: (CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 4 (5%), Referenced to	o phase 2:	and 6:SB	TL, Start	of Green								
Natural Cycle: 65												
Control Type: Actuated-Cool	rdinated											
Maximum v/c Ratio: 0.52												
Intersection Signal Delay: 5.	7			In	tersectior	n LOS: A						
Intersection Capacity Utilizat	ion 44.0%			IC	U Level o	of Service	A					
Analysis Period (min) 15												
Analysis Period (min) 15												

Splits and Phases: 23: S Center St/N Madison St & W Front St

	→ Ø4	35
	30 s	
Ø6 (R)	₹ø8	1.30
50 s	30 s	

02/22/2024

Lanes, Volumes, Timings 24: S East St/N East St & E Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	•			1	1		đ î þ				
Traffic Volume (vph)	13	30	0	0	127	36	40	904	16	0	0	0
Future Volume (vph)	13	30	0	0	127	36	40	904	16	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	16	12	12	10	10	13	11	11	12	12	12
Grade (%)		-1%			0%			-1%			2%	
Storage Length (ft)	90		0	0		100	0		0	0		0
Storage Lanes	1		0	0		1	0		0	0		0
Taper Length (ft)	0			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.98					0.96		1.00				
Frt						0.850		0.997				
Flt Protected	0.950							0.998				
Satd. Flow (prot)	1518	1873	0	0	1535	1304	0	3019	0	0	0	0
Flt Permitted	0.620							0.998				
Satd. Flow (perm)	966	1873	0	0	1535	1250	0	3017	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)						42		3				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		256			710			633			322	
Travel Time (s)		5.8			16.1			14.4			7.3	
Confl. Peds. (#/hr)	15					15	19		1			
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)							5					
Adj. Flow (vph)	15	35	0	0	149	42	47	1064	19	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	15	35	0	0	149	42	0	1130	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		11			11			11			11	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.19	0.97	1.14	1.14	1.25	1.25	1.09	1.19	1.19	1.16	1.16	1.16
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2			2	1	1	2				
Detector Template	Left	Thru			Thru	Right	Left	Thru				
Leading Detector (ft)	20	100			100	20	20	100				
Trailing Detector (ft)	0	0			0	0	0	0				
Detector 1 Position(ft)	0	0			0	0	0	0				
Detector 1 Size(ft)	20	6			6	20	20	6				
Detector 1 Type	CI+Ex	Cl+Ex			CI+Ex	Cl+Ex	CI+Ex	CI+Ex				
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 24: S East St/N East St & E Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Type		CI+Ex			Cl+Ex			Cl+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA	Perm	Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4					8	2					
Detector Phase	4	4			8	8	2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0	5.0	5.0	5.0				
Minimum Split (s)	27.2	27.2			17.2	17.2	50.6	50.6				
Total Split (s)	29.0	29.0			29.0	29.0	51.0	51.0				
Total Split (%)	36.3%	36.3%			36.3%	36.3%	63.8%	63.8%				
Maximum Green (s)	23.8	23.8			23.8	23.8	45.4	45.4				
Yellow Time (s)	3.2	3.2			3.2	3.2	3.2	3.2				
All-Red Time (s)	2.0	2.0			2.0	2.0	2.4	2.4				
Lost Time Adjust (s)	0.0	0.0			0.0	0.0		0.0				
Total Lost Time (s)	5.2	5.2			5.2	5.2		5.6				
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0	3.0	3.0	3.0				
Recall Mode	None	None			None	None	C-Max	C-Max				
Walk Time (s)	11.0	11.0			1.0	1.0	34.0	34.0				
Flash Dont Walk (s)	11.0	11.0			11.0	11.0	11.0	11.0				
Pedestrian Calls (#/hr)	0	0			0	0	0	0				
Act Effct Green (s)	13.1	13.1			13.1	13.1		56.1				
Actuated g/C Ratio	0.16	0.16			0.16	0.16		0.70				
v/c Ratio	0.10	0.11			0.60	0.18		0.53				
Control Delay	27.4	27.3			40.2	10.6		4.8				
Queue Delay	0.0	0.0			0.0	0.0		0.0				
Total Delay	27.4	27.3			40.2	10.6		4.8				
LOS	С	С			D	В		А				
Approach Delay		27.3			33.7			4.8				
Approach LOS		С			С			А				
90th %ile Green (s)	18.4	18.4			18.4	18.4	50.8	50.8				
90th %ile Term Code	Hold	Hold			Gap	Gap	Coord	Coord				
70th %ile Green (s)	15.3	15.3			15.3	15.3	53.9	53.9				
70th %ile Term Code	Hold	Hold			Gap	Gap	Coord	Coord				
50th %ile Green (s)	13.1	13.1			13.1	13.1	56.1	56.1				
50th %ile Term Code	Hold	Hold			Gap	Gap	Coord	Coord				
30th %ile Green (s)	10.8	10.8			10.8	10.8	58.4	58.4				
30th %ile Term Code	Hold	Hold			Gap	Gap	Coord	Coord				
10th %ile Green (s)	7.7	7.7			7.7	7.7	61.5	61.5				
10th %ile Term Code	Hold	Hold			Gap	Gap	Coord	Coord				
Stops (vph)	12	26			113	11		264				
Fuel Used(gal)	0	0			2	0		7				
CO Emissions (g/hr)	12	26			165	24		499				
NOx Emissions (g/hr)	2	5			32	5		97				
VOC Emissions (g/hr)	3	6			38	5		116				
Dilemma Vehicles (#)	0	0			0	0		0				

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 24: S East St/N East St & E Front St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)	6	15			70	0		67				
Queue Length 95th (ft)	20	34			110	22		80				
Internal Link Dist (ft)		176			630			553			242	
Turn Bay Length (ft)	90					100						
Base Capacity (vph)	287	557			456	401		2118				
Starvation Cap Reductn	0	0			0	0		0				
Spillback Cap Reductn	0	0			0	0		41				
Storage Cap Reductn	0	0			0	0		0				
Reduced v/c Ratio	0.05	0.06			0.33	0.10		0.54				
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 64 (80%), Reference	ed to phase	2:NBTL a	ind 6:, St	art of Gre	en							
Natural Cycle: 80												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.60												
Intersection Signal Delay: 9	9.7			In	tersectior	n LOS: A						
Intersection Capacity Utilization	ation 61.5%			IC	U Level o	of Service	В					
Analysis Period (min) 15												

Splits and Phases: 24: S East St/N East St & E Front St

Ø2 (R)	<u></u>
51s	29 s
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	Ø8
	29 s

Lanes, Volumes, Timings 27: N Center St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,			र्स						đ þ	
Traffic Volume (vph)	0	38	21	12	101	0	0	0	0	4	126	12
Future Volume (vph)	0	38	21	12	101	0	0	0	0	4	126	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt		0.951									0.987	
Flt Protected					0.995						0.999	
Satd. Flow (prot)	0	1534	0	0	1445	0	0	0	0	0	2871	0
Flt Permitted					0.995						0.999	
Satd. Flow (perm)	0	1534	0	0	1445	0	0	0	0	0	2871	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		247			266			264			413	
Travel Time (s)		5.6			6.0			6.0			9.4	
Confl. Peds. (#/hr)			23	23						6		6
Confl. Bikes (#/hr)			1									
Peak Hour Factor	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Heavy Vehicles (%)	6%	6%	6%	6%	6%	6%	6%	6%	6%	6%	6%	6%
Parking (#/hr)					0						0	
Adj. Flow (vph)	0	48	27	15	128	0	0	0	0	5	159	15
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	75	0	0	143	0	0	0	0	0	179	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.14	1.14	1.14	1.30	1.14	1.14	1.14	1.14	1.14	1.22	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: 0	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 26.1%			IC	CU Level of	of Service	Α					
Analysis Period (min) 15												

Lanes, Volumes, Timings 29: N Center St & W Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ĥ			د						đ þ	
Traffic Volume (vph)	0	30	1	11	16	0	0	0	0	18	94	3
Future Volume (vph)	0	30	1	11	16	0	0	0	0	18	94	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt		0.997									0.996	
Flt Protected					0.980						0.992	
Satd. Flow (prot)	0	1408	0	0	1384	0	0	0	0	0	2798	0
Flt Permitted					0.980						0.992	
Satd. Flow (perm)	0	1408	0	0	1384	0	0	0	0	0	2798	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		253			255			306			278	
Travel Time (s)		5.8			5.8			7.0			6.3	
Confl. Peds. (#/hr)			7	7						13		9
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80
Heavy Vehicles (%)	9%	9%	9%	9%	9%	9%	9%	9%	9%	9%	9%	9%
Parking (#/hr)		0			0						0	
Adj. Flow (vph)	0	38	1	14	20	0	0	0	0	23	118	4
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	39	0	0	34	0	0	0	0	0	145	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.14	1.14	1.14	1.22	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type:	CBD											
Control Type: Unsignalized												
Intersection Canacity I Itiliza	ation 21.1%			10	ر امریم ا ا ۱	of Service	Δ					

Analysis Period (min) 15

Lanes, Volumes, Timings 30: N Center St & W Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,		5	+						đ î þ	
Traffic Volume (vph)	0	149	6	21	194	0	0	0	0	45	55	6
Future Volume (vph)	0	149	6	21	194	0	0	0	0	45	55	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	65		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		1.00		1.00							0.99	
Frt		0.994									0.991	
Flt Protected				0.950							0.979	
Satd. Flow (prot)	0	1416	0	1504	1425	0	0	0	0	0	2766	0
Flt Permitted				0.563							0.979	
Satd. Flow (perm)	0	1416	0	889	1425	0	0	0	0	0	2755	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		4									8	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		253			254			326			306	
Travel Time (s)		5.8			5.8			7.4			7.0	
Confl. Peds. (#/hr)			3	3						4		10
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80
Heavy Vehicles (%)	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%
Parking (#/hr)		0			0						0	
Adj. Flow (vph)	0	186	8	26	243	0	0	0	0	56	69	8
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	194	0	26	243	0	0	0	0	0	133	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.14	1.14	1.14	1.22	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		Cl+Ex	CI+Ex					Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		Cl+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 30: N Center St & W Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		23.0		23.0	23.0					23.0	23.0	
Total Split (s)		50.0		50.0	50.0					30.0	30.0	
Total Split (%)		62.5%		62.5%	62.5%					37.5%	37.5%	
Maximum Green (s)		45.0		45.0	45.0					25.0	25.0	
Yellow Time (s)		4.0		4.0	4.0					4.0	4.0	
All-Red Time (s)		1.0		1.0	1.0					1.0	1.0	
Lost Time Adjust (s)		0.0		0.0	0.0						0.0	
Total Lost Time (s)		5.0		5.0	5.0						5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		None	None					C-Max	C-Max	
Walk Time (s)		7.0		7.0	7.0					7.0	7.0	
Flash Dont Walk (s)		11.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		19.8		19.8	19.8						50.2	
Actuated g/C Ratio		0.25		0.25	0.25						0.63	
v/c Ratio		0.55		0.12	0.69						0.08	
Control Delay		18.2		3.3	14.5						7.0	
Queue Delay		0.0		0.0	0.1						0.0	
Total Delay		18.2		3.3	14.5						7.0	
LOS		В		А	В						А	
Approach Delay		18.2			13.4						7.0	
Approach LOS		В			В						А	
90th %ile Green (s)		28.3		28.3	28.3					41.7	41.7	
90th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
70th %ile Green (s)		23.2		23.2	23.2					46.8	46.8	
70th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
50th %ile Green (s)		19.7		19.7	19.7					50.3	50.3	
50th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
30th %ile Green (s)		16.3		16.3	16.3					53.7	53.7	
30th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
10th %ile Green (s)		11.6		11.6	11.6					58.4	58.4	
10th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
Stops (vph)		91		5	131						40	
Fuel Used(gal)		1		0	2						1	
CO Emissions (g/hr)		97		6	117						44	
NOx Emissions (g/hr)		19		1	23						9	
VOC Emissions (g/hr)		22		1	27						10	
Dilemma Vehicles (#)		0		0	0						0	
Queue Length 50th (ft)		37		2	16						11	
Queue Length 95th (ft)		49		4	17						25	

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 30: N Center St & W Washington St

02/22/2024	ŀ
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		173			174			246			226	
Turn Bay Length (ft)				65								
Base Capacity (vph)		798		500	801						1730	
Starvation Cap Reductn		50		0	68						0	
Spillback Cap Reductn		0		0	0						0	
Storage Cap Reductn		0		0	0						0	
Reduced v/c Ratio		0.26		0.05	0.33						0.08	
Intersection Summary												
Area Type: 0	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 20 (25%), Referenced	to phase	2: and 6:	SBTL, Sta	art of Gree	en							
Natural Cycle: 50												
Control Type: Actuated-Coor	dinated											
Maximum v/c Ratio: 0.69												
Intersection Signal Delay: 13	.6			In	tersectior	n LOS: B						
Intersection Capacity Utilizati	on 46.9%			IC	U Level o	of Service	А					
Analysis Period (min) 15												

Splits and Phases: 30: N Center St & W Washington St

	→ Ø4	
	50 s	
Ø6 (R)	▼ Ø8	
30 s	50 s	

Lanes, Volumes, Timings 31: W Front St & N Center St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f,		٦	•			\$		7	ĥ	
Traffic Volume (vph)	0	115	19	20	56	0	0	0	0	52	13	5
Future Volume (vph)	0	115	19	20	56	0	0	0	0	52	13	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	8	12	12	12	12
Storage Length (ft)	0		0	70		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.981									0.961	
Flt Protected				0.950						0.950		
Satd. Flow (prot)	0	1398	0	1504	1425	0	0	1235	0	1354	1369	0
Flt Permitted				0.950						0.950		
Satd. Flow (perm)	0	1398	0	1504	1425	0	0	1235	0	1354	1369	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		247			262			221			326	
Travel Time (s)		5.6			6.0			5.0			7.4	
Confl. Peds. (#/hr)			35	35			19		8	8		19
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%
Parking (#/hr)		0			0			0		0	0	
Adj. Flow (vph)	0	125	21	22	61	0	0	0	0	57	14	5
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	146	0	22	61	0	0	0	0	57	19	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		13			13			0			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane					Yes							
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.55	1.14	1.30	1.30	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: C	BD											
Control Type: Unsignalized												
Intersection Capacity Utilization	on 34.4%			IC	U Level o	of Service	А					
Analysis Period (min) 15												

Lanes, Volumes, Timings 34: N Main St & W Market St/E Market St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ŧ			ħ			đ þ				
Traffic Volume (vph)	7	37	0	0	87	5	29	26	18	0	0	0
Future Volume (vph)	7	37	0	0	87	5	29	26	18	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.993			0.964				
Flt Protected		0.992						0.980				
Satd. Flow (prot)	0	1414	0	0	1415	0	0	2700	0	0	0	0
Flt Permitted		0.992						0.980				
Satd. Flow (perm)	0	1414	0	0	1415	0	0	2700	0	0	0	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		266			263			249			404	
Travel Time (s)		6.0			6.0			5.7			9.2	
Confl. Peds. (#/hr)	13					13	1		7			
Confl. Bikes (#/hr)						1						
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Heavy Vehicles (%)	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%
Parking (#/hr)		0			0			0				
Adj. Flow (vph)	9	48	0	0	113	6	38	34	23	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	57	0	0	119	0	0	95	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.22	1.14	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: 0	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 20.9%			IC	CU Level o	of Service	А					
Analysis Period (min) 15												

Lanes, Volumes, Timings <u>36: N Main St & W Jefferson St/E Jefferson St</u>

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ŧ			ĥ			đ î ja				
Traffic Volume (vph)	14	14	0	0	15	10	23	61	10	0	0	0
Future Volume (vph)	14	14	0	0	15	10	23	61	10	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.945			0.984				
Flt Protected		0.976						0.988				
Satd. Flow (prot)	0	1366	0	0	1322	0	0	2728	0	0	0	0
Flt Permitted		0.976						0.988				
Satd. Flow (perm)	0	1366	0	0	1322	0	0	2728	0	0	0	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		255			268			306			272	
Travel Time (s)		5.8			6.1			7.0			6.2	
Confl. Peds. (#/hr)	11					11	2		24			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	10%	10%	10%	10%	10%	10%	10%	10%	10%	10%	10%	10%
Parking (#/hr)		0			0			0				
Adj. Flow (vph)	15	15	0	0	16	11	25	66	11	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	30	0	0	27	0	0	102	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.22	1.14	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: 0	CBD											
Control Type: Unsignalized												

Intersection Capacity Utilization 23.9%

ICU Level of Service A

Analysis Period (min) 15

Lanes, Volumes, Timings	
37: N Main St & W Washington St/E Washington St	t

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٢	+			ţ,			र्स	1			
Traffic Volume (vph)	1	176	0	0	229	27	5	22	9	0	0	0
Future Volume (vph)	1	176	0	0	229	27	5	22	9	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	65		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00				1.00			1.00	0.97			
Frt					0.986				0.850			
Flt Protected	0.950							0.991				
Satd. Flow (prot)	1504	1425	0	0	1403	0	0	1412	1211	0	0	0
Flt Permitted	0.406							0.991				
Satd. Flow (perm)	641	1425	0	0	1403	0	0	1408	1171	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					13				27			
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		254			262			326			306	
Travel Time (s)		5.8			6.0			7.4			7.0	
Confl. Peds. (#/hr)	3					3	7		6			
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80
Heavy Vehicles (%)	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%
Parking (#/hr)		0			0			0	0			
Adj. Flow (vph)	1	220	0	0	286	34	6	28	11	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	1	220	0	0	320	0	0	34	11	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12	Ū		12	Ū		0	J		0	Ŭ
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.30	1.30	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2			2		1	2	1			
Detector Template	Left	Thru			Thru		Left	Thru	Right			
Leading Detector (ft)	20	100			100		20	100	20			
Trailing Detector (ft)	0	0			0		0	0	0			
Detector 1 Position(ft)	0	0			0		0	0	0			
Detector 1 Size(ft)	20	6			6		20	6	20			
Detector 1 Type	Cl+Ex	Cl+Ex			CI+Ex		Cl+Ex	Cl+Ex	CI+Ex			
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			CI+Ex			Cl+Ex				
Detector 2 Channel												

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings	
37: N Main St & W Washington St/E Washington	St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA		Perm	NA	Perm			
Protected Phases		4			8			2				
Permitted Phases	4						2		2			
Detector Phase	4	4			8		2	2	2			
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0		5.0	5.0	5.0			
Minimum Split (s)	23.0	23.0			23.0		23.0	23.0	23.0			
Total Split (s)	51.0	51.0			51.0		29.0	29.0	29.0			
Total Split (%)	63.8%	63.8%			63.8%		36.3%	36.3%	36.3%			
Maximum Green (s)	46.0	46.0			46.0		24.0	24.0	24.0			
Yellow Time (s)	4.0	4.0			4.0		4.0	4.0	4.0			
All-Red Time (s)	1.0	1.0			1.0		1.0	1.0	1.0			
Lost Time Adjust (s)	0.0	0.0			0.0			0.0	0.0			
Total Lost Time (s)	5.0	5.0			5.0			5.0	5.0			
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0	3.0			
Recall Mode	None	None			None		C-Max	C-Max	C-Max			
Walk Time (s)	7.0	7.0			7.0		7.0	7.0	7.0			
Flash Dont Walk (s)	11.0	11.0			11.0		11.0	11.0	11.0			
Pedestrian Calls (#/hr)	0	0			0		0	0	0			
Act Effct Green (s)	24.6	24.6			24.6			45.4	45.4			
Actuated g/C Ratio	0.31	0.31			0.31			0.57	0.57			
v/c Ratio	0.01	0.50			0.73			0.04	0.02			
Control Delay	15.0	29.2			11.5			13.2	5.3			
Queue Delay	0.0	0.1			0.1			0.0	0.0			
Total Delay	15.0	29.3			11.6			13.2	5.3			
LOS	В	С			В			В	А			
Approach Delay		29.3			11.6			11.3				
Approach LOS		С			В			В				
90th %ile Green (s)	35.2	35.2			35.2		34.8	34.8	34.8			
90th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
70th %ile Green (s)	28.6	28.6			28.6		41.4	41.4	41.4			
70th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
50th %ile Green (s)	24.4	24.4			24.4		45.6	45.6	45.6			
50th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
30th %ile Green (s)	20.2	20.2			20.2		49.8	49.8	49.8			
30th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
10th %ile Green (s)	14.4	14.4			14.4		55.6	55.6	55.6			
10th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
Stops (vph)	1	99			42			19	4			
Fuel Used(gal)	0	2			1			0	0			
CO Emissions (g/hr)	1	136			95			17	4			
NOx Emissions (g/hr)	0	26			18			3	1			
VOC Emissions (g/hr)	0	31			22			4	1			
Dilemma Vehicles (#)	0	0			0			0	0			
Queue Length 50th (ft)	0	78			12			11	0			
Queue Length 95th (ft)	m1	48			13			m28	m5			

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 37: N Main St & W Washington St/E Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		174			182			246			226	
Turn Bay Length (ft)	65											
Base Capacity (vph)	368	819			812			799	677			
Starvation Cap Reductn	0	120			39			0	0			
Spillback Cap Reductn	0	27			7			0	0			
Storage Cap Reductn	0	0			0			0	0			
Reduced v/c Ratio	0.00	0.31			0.41			0.04	0.02			
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 7 (9%), Referenced t	o phase 2:1	VBTL and	l 6:, Start	of Green								
Natural Cycle: 50												
Control Type: Actuated-Coo	rdinated											
Maximum v/c Ratio: 0.73												
Intersection Signal Delay: 18	3.2			In	tersectior	n LOS: B						
Intersection Capacity Utilizat	tion 46.9%			IC	U Level o	of Service	Α					
Analysis Period (min) 15												
m Volume for 95th percent	tile queue is	s metered	l by upstr	eam sign	al.							

Splits and Phases: 37: N Main St & W Washington St/E Washington St

1 √ Ø2 (R)		
29 s	51 s	
	← Ø8	
	51 s	

Lanes, Volumes, Timings 38: W Front St/E Front St & N Main St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	ţ,			4			4				
Traffic Volume (vph)	30	135	0	0	81	26	0	0	0	0	0	0
Future Volume (vph)	30	135	0	0	81	26	0	0	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	8	12	12	12	12
Storage Length (ft)	70		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.968							
Flt Protected	0.950											
Satd. Flow (prot)	1504	1425	0	0	1379	0	0	1308	0	0	0	0
Flt Permitted	0.950											
Satd. Flow (perm)	1504	1425	0	0	1379	0	0	1308	0	0	0	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		262			256			210			326	
Travel Time (s)		6.0			5.8			4.8			7.4	
Confl. Peds. (#/hr)	17		2	2		17	7		36			
Peak Hour Factor	0.83	0.83	0.92	0.92	0.83	0.83	0.92	0.92	0.92	0.83	0.92	0.83
Heavy Vehicles (%)	8%	8%	2%	2%	8%	8%	2%	2%	2%	8%	2%	8%
Parking (#/hr)		0			0			0				
Adj. Flow (vph)	36	163	0	0	98	31	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	36	163	0	0	129	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.55	1.14	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type: 0	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 26.7%			IC	U Level	of Service	А					
Analysis Period (min) 15												

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		1	≜ î∌			
Traffic Volume (vph)	0	15	925	6	0	0
Future Volume (vph)	0	15	925	6	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	11	11	12	12
Grade (%)	0%		-2%			2%
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Frt		0.865	0.999			
Flt Protected						
Satd. Flow (prot)	0	1292	3077	0	0	0
Flt Permitted						
Satd. Flow (perm)	0	1292	3077	0	0	0
Link Speed (mph)	30		30			30
Link Distance (ft)	558		266			127
Travel Time (s)	12.7		6.0			2.9
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%
Parking (#/hr)		0				
Adj. Flow (vph)	0	18	1128	7	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	18	1135	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		0			0
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.14	1.30	1.18	1.18	1.16	1.16
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Stop
Intersection Summary						
Area Type:	CBD					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 38.6%			IC	U Level o	of Service A

Analysis Period (min) 15

Lanes, Volumes, Timings 47: S Center St & W Olive St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1	1	5	1						41	1
Traffic Volume (vph)	0	55	33	3	33	0	0	0	0	39	657	52
Future Volume (vph)	0	55	33	3	33	0	0	0	0	39	657	52
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	11	10	12	12	12	12	12	12	12	12	12
Grade (%)		1%			-3%			3%			-1%	
Storage Length (ft)	0		80	0		0	0		0	0		50
Storage Lanes	0		1	1		0	0		0	0		1
Taper Length (ft)	25			25		-	25		-	25		-
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00
Frt			0.850							0.00		0.850
Elt Protected			0.000	0.950							0 997	
Satd Flow (prot)	0	1792	1470	1796	1891	0	0	0	0	0	3546	1591
Elt Permitted	Ű			0 718	1001	Ŭ	Ŭ	Ŭ	Ŭ	Ŭ	0.997	1001
Satd Flow (perm)	0	1792	1470	1358	1891	0	0	0	0	0	3546	1591
Right Turn on Red	Ū	11.52	Yes	1000	1001	Yes	U	U	Yes	U	00-10	Yes
Satd Flow (RTOR)			40			103			103			55
Link Speed (mph)		30	-0		30			30			30	00
Link Distance (ff)		777			330			770			660	
Travel Time (s)		17.7			7.5			17.5			15.0	
Peak Hour Factor	0 02	0.02	0 02	0.02	0.02	0 02	0 02	0.02	0 02	0.02	0.02	0 02
Adi Elow (uph)	0.92	60	0.92	0.92	0.92	0.52	0.92	0.92	0.92	0.92	71/	0.92
Shared Lane Traffic (%)	U	00	50	J	50	U	0	0	0	42	/ 14	51
Lano Group Flow (uph)	٥	60	36	3	36	٥	٥	٥	٥	٥	756	57
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lano Alignment	Loft	Loft	Dight	Loft	Loft	Dight	Loft	Loft	Dight	Loft	Loft	Diaht
Lane Algriment Modion Width(ft)	Leit	12	Right	Leit	12	Right	Leit		Right	Leit	Leit	Right
Link Offect(ft)		12			12			0			0	
Crosswalk Width(ft)		16			16			16			16	
		10			10			10			10	
Hoodway Easter	1 01	1.05	1 10	0.08	0.08	0.08	1.02	1 02	1 02	0.00	0.00	0.00
Turning Speed (mph)	1.01	1.05	1.10	0.90	0.90	0.90	1.02	1.02	1.02	0.99	0.99	0.99
Number of Detectors	15	2	9	10	2	9	10		9	10	2	9
Detector Templete		Z	Diaht	l off	Z					 0#	Z	Diaht
Leading Detector (ft)		100	Right	Leit	100					Leit	100	Right
Leading Detector (ft)		100	20	20	100					20	100	20
Trailing Detector (II)		0	0	0	0					0	0	0
Detector 1 Position(it)		0	0	0	0					0	0	0
Detector 1 Size(π)		0	20	20	0					20	0	20
Detector 1 Type		CI+EX	CI+EX	CI+EX	CI+EX					CI+EX	CI+EX	CI+EX
Detector 1 Channel		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 1 Extend (s)		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 1 Queue (s)		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 1 Delay (s)		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		Cl+Ex			Cl+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA	Perm	Perm	NA					Perm	NA	Perm

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 47: S Center St & W Olive St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases		4			8						6	
Permitted Phases			4	8						6		6
Detector Phase		4	4	8	8					6	6	6
Switch Phase												
Minimum Initial (s)		5.0	5.0	5.0	5.0					5.0	5.0	5.0
Minimum Split (s)		13.5	13.5	13.5	13.5					37.9	37.9	37.9
Total Split (s)		22.0	22.0	22.0	22.0					58.0	58.0	58.0
Total Split (%)		27.5%	27.5%	27.5%	27.5%					72.5%	72.5%	72.5%
Maximum Green (s)		16.5	16.5	16.5	16.5					52.1	52.1	52.1
Yellow Time (s)		3.2	3.2	3.2	3.2					3.2	3.2	3.2
All-Red Time (s)		2.3	2.3	2.3	2.3					2.7	2.7	2.7
Lost Time Adjust (s)		0.0	0.0	0.0	0.0						0.0	0.0
Total Lost Time (s)		5.5	5.5	5.5	5.5						5.9	5.9
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0	3.0	3.0	3.0					3.0	3.0	3.0
Recall Mode		Max	Max	None	None					C-Max	C-Max	C-Max
Walk Time (s)		1.0	1.0	1.0	1.0					21.0	21.0	21.0
Flash Dont Walk (s)		7.0	7.0	7.0	7.0					11.0	11.0	11.0
Pedestrian Calls (#/hr)		0	0	0	0					0	0	0
Act Effct Green (s)		16.5	16.5	16.5	16.5						52.1	52.1
Actuated g/C Ratio		0.21	0.21	0.21	0.21						0.65	0.65
v/c Ratio		0.16	0.11	0.01	0.09						0.33	0.05
Control Delay		27.5	9.2	25.7	25.7						3.9	0.7
Queue Delay		0.0	0.0	0.0	0.0						0.0	0.0
Total Delay		27.5	9.2	25.7	25.7						3.9	0.7
LOS		С	А	С	С						А	A
Approach Delay		20.7			25.7						3.6	
Approach LOS		С			С						А	
90th %ile Green (s)		16.5	16.5	16.5	16.5					52.1	52.1	52.1
90th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
70th %ile Green (s)		16.5	16.5	16.5	16.5					52.1	52.1	52.1
70th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
50th %ile Green (s)		16.5	16.5	16.5	16.5					52.1	52.1	52.1
50th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
30th %ile Green (s)		16.5	16.5	16.5	16.5					52.1	52.1	52.1
30th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
10th %ile Green (s)		16.5	16.5	16.5	16.5					52.1	52.1	52.1
10th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
Stops (vph)		45	10	4	29						226	5
Fuel Used(gal)		1	0	0	0						5	0
CO Emissions (g/hr)		62	22	3	29						376	21
NOx Emissions (g/hr)		12	4	1	6						73	4
VOC Emissions (g/hr)		14	5	1	7						87	5
Dilemma Vehicles (#)		0	0	0	0						0	0
Queue Length 50th (ft)		25	0	1	15						88	3
Queue Length 95th (ft)		56	22	m5	m38						21	0
Internal Link Dist (ft)		697			250			690			580	
Turn Bay Length (ft)			80									50

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 47: S Center St & W Olive St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Base Capacity (vph)		369	334	280	390						2309	1055
Starvation Cap Reductn		0	0	0	0						0	0
Spillback Cap Reductn		0	0	0	0						0	0
Storage Cap Reductn		0	0	0	0						0	0
Reduced v/c Ratio		0.16	0.11	0.01	0.09						0.33	0.05
Intersection Summary												
Area Type:	Other											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 21 (26%), Reference	ed to phase	2: and 6:	SBTL, Sta	art of Gre	en							
Natural Cycle: 55	•											
Control Type: Actuated-Cod	ordinated											
Maximum v/c Ratio: 0.33												
Intersection Signal Delay: 6	.3			In	tersectior	n LOS: A						
Intersection Capacity Utiliza	ation 41.7%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
m Volume for 95th percer	ntile queue i	s metered	l by upstr	eam sign	al.							
Splits and Phases: 47: S	Center St 8	W Olive	St									
-%								~	0.00			55



Lanes, Volumes, Timings 48: S East St & E Olive St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		÷.			† 1 ₂			đ þ				
Traffic Volume (vph)	53	47	0	0	15	23	28	928	10	0	0	0
Future Volume (vph)	53	47	0	0	15	23	28	928	10	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	12	12	12	12	12	12	12	12	12
Grade (%)		2%			-4%			4%			-4%	
Storage Length (ft)	0		0	0		0	300		0	0		0
Storage Lanes	0		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00
Frt					0.909			0.998				
Flt Protected		0.974						0.999				
Satd, Flow (prot)	0	1675	0	0	3188	0	0	3359	0	0	0	0
Elt Permitted	Ū	0.810	,	Ţ		Ū	•	0.999	, ,	,	Ţ	Ū
Satd Flow (perm)	0	1393	0	0	3188	0	0	3359	0	0	0	0
Right Turn on Red	Ŭ	1000	Yes	Ű	0100	Yes	Ū	0000	Yes	Ŭ	Ŭ	Yes
Satd Flow (RTOR)			100		25	100		3	100			100
Link Speed (mph)		30			30			30			30	
Link Distance (ff)		330			647			682			633	
Travel Time (s)		7.5			1/ 7			15.5			1//	
Peak Hour Factor	0 02	0.02	0 02	0 02	0.02	0 02	0 02	0.02	0 02	0 02	0.02	0.02
	0.9Z	0.9Z	0.9Z	0.9Z	0.9Z	0.9Z	0.9Z	0.9Z	0.9Z	0.9Z	0.9Z	0.9Z
Darking (#/br)	J /0	J /0	J /0	J /0	J /0	J /0	J /0	J /0	J /0	J /0	J /0	J /0
Adi Elow (vph)	59	51	٥	٥	16	25	30	1000	11	٥	٥	٥
Auj. Flow (vpi) Sharad Lana Traffia (%)	50	51	0	U	10	20	50	1009	11	0	0	0
	٥	100	0	٥	11	٥	٥	1050	٥	0	٥	٥
Earle Group Flow (vpri)	U	109	U No	U	4 I	U	U	1000	U No	U No	U No	U
	INO	INO	N0 Dialat	INO	INO L off	NU Dialat	INO Loft	INO	N0 Dialat	INO	INO	NU Dialat
Lane Alignment	Len	Len	Right	Len	Len	Right	Len	Len	Right	Len	Leit	Right
Median Width(ft)		0			0			0			0	
		0			10			10			0	
		16			10			10			10	
I wo way Left Turn Lane	4.04	4.07	4.04	0.07	0.07	0.07	4.00	4 00	4.00	0.07	0.07	0.07
Headway Factor	1.01	1.07	1.01	0.97	0.97	0.97	1.03	1.03	1.03	0.97	0.97	0.97
Turning Speed (mpn)	15	0	9	15	0	9	15	0	9	15		9
Number of Detectors	1	2			2		1	2				
Detector Template	Left	Thru			Ihru		Left	Thru				
Leading Detector (ft)	20	100			100		20	100				
Trailing Detector (ft)	0	0			0		0	0				
Detector 1 Position(ft)	0	0			0		0	0				
Detector 1 Size(ft)	20	6			6		20	6				
Detector 1 Type	Cl+Ex	Cl+Ex			Cl+Ex		CI+Ex	Cl+Ex				
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0				
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex				
Detector 2 Channel												

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 48: S East St & E Olive St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA		Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4						2					
Detector Phase	4	4			8		2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0		5.0	5.0				
Minimum Split (s)	17.5	17.5			17.5		60.6	60.6				
Total Split (s)	18.6	18.6			18.6		61.4	61.4				
Total Split (%)	23.3%	23.3%			23.3%		76.8%	76.8%				
Maximum Green (s)	13.1	13.1			13.1		55.8	55.8				
Yellow Time (s)	3.2	3.2			3.2		3.2	3.2				
All-Red Time (s)	2.3	2.3			2.3		2.4	2.4				
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.5			5.5			5.6				
Lead/Lag		0.0			0.0			0.0				
Lead-Lag Optimize?												
Vehicle Extension (s)	30	30			30		30	30				
Recall Mode	None	None			Max		C-Max	C-Max				
Walk Time (s)	1 0	1.0			1.0		44 0	44 0				
Flash Dont Walk (s)	11.0	11.0			11.0		11.0	11.0				
Pedestrian Calls (#/hr)	0	0			0		0	0				
Act Effct Green (s)	Ű	13 1			13 1		Ū	55.8				
Actuated g/C Ratio		0.16			0.16			0.70				
v/c Ratio		0.10			0.10			0.45				
Control Delay		38.0			16.6			6.0				
Queue Delay		0.0			0.0			0.0				
Total Delay		38.0			16.6			6.0				
		D			B			Δ				
Approach Delay		38.0			16.6			60				
Approach LOS		D			B			Δ				
90th %ile Green (s)	13.1	13 1			13.1		55.8	55.8				
90th %ile Term Code	Max	Max			MaxR		Coord	Coord				
70th %ile Green (s)	13.1	13.1			13.1		55.8	55.8				
70th %ile Term Code	Max	Max			MaxR		Coord	Coord				
50th %ile Green (s)	13.1	13.1			13.1		55.8	55.8				
50th %ile Term Code	Hold	Hold			MaxR		Coord	Coord				
30th %ile Green (s)	13.1	13.1			13.1		55.8	55.8				
30th %ile Term Code	Hold	Hold			MaxR		Coord	Coord				
10th %ile Green (s)	13.1	13.1			13.1		55.8	55.8				
10th %ile Term Code	Hold	Hold			MaxR		Coord	Coord				
Stops (vph)	TIOIG	98			19		00010	382				
Fuel Lised(gal)		2			0			8				
CO Emissions (a/hr)		110			30			590				
NOv Emissions (g/m)		21			6			115				
VOC Emissions (g/lir)		21			7			127				
Dilemma Vehicles (#)		20			0			137				
		56			2			101				
		110			17			101				
		110			17			155				

2024 AM Downtown Bloomington 3:29 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 48: S East St & E Olive St

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Lane Group	EBL	EBT	EBR	• WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		250			567			602			553	
Turn Bay Length (ft)												
Base Capacity (vph)		228			542			2343				
Starvation Cap Reductn		0			0			0				
Spillback Cap Reductn		0			0			0				
Storage Cap Reductn		0			0			0				
Reduced v/c Ratio		0.48			0.08			0.45				
Intersection Summary												
Area Type:	Other											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 51 (64%), Reference	ed to phase 2	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 80												
Control Type: Actuated-Coc	ordinated											
Maximum v/c Ratio: 0.48												
Intersection Signal Delay: 9	.3			In	tersectior	LOS: A						
Intersection Capacity Utiliza	tion 48.1%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
Splits and Phases: 48: S	East St & E	Olive St										

Ø2 (R)	6	A ₀₄	-
61.4s		18.6 s	
	6)	← Ø8	
		18.6 s	

02/22/2024

Lanes, Volumes, Timings 3: N Madison St/N Center St & W Locust St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		≜ î∌			ŧ						-۠	1
Traffic Volume (vph)	0	444	63	4	57	0	0	0	0	196	1182	165
Future Volume (vph)	0	444	63	4	57	0	0	0	0	196	1182	165
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	0		0	0		150
Storage Lanes	0		0	0		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00
Ped Bike Factor		1.00									1.00	
Frt		0.981										0.850
Flt Protected					0.997						0.993	
Satd. Flow (prot)	0	3465	0	0	1857	0	0	0	0	0	3514	1583
Flt Permitted					0.965						0.993	
Satd. Flow (perm)	0	3465	0	0	1798	0	0	0	0	0	3507	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		22										176
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		659			244			860			526	
Travel Time (s)		15.0			5.5			19.5			12.0	
Confl. Peds. (#/hr)			4							12		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	0	472	67	4	61	0	0	0	0	209	1257	176
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	539	0	0	65	0	0	0	0	0	1466	176
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0	Ū		0	Ū		0	Ŭ		0	Ū
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	1
Detector Template		Thru		Left	Thru					Left	Thru	Right
Leading Detector (ft)		100		20	100					20	100	20
Trailing Detector (ft)		0		0	0					0	0	0
Detector 1 Position(ft)		0		0	0					0	0	0
Detector 1 Size(ft)		6		20	6					20	6	20
Detector 1 Type		Cl+Ex		Cl+Ex	CI+Ex					Cl+Ex	Cl+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		CI+Ex			CI+Ex						Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	Perm

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE
Lanes, Volumes, Timings 3: N Madison St/N Center St & W Locust St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases		4			8						6	
Permitted Phases				8						6		6
Detector Phase		4		8	8					6	6	6
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	5.0
Minimum Split (s)		23.4		22.5	22.5					23.4	23.4	23.4
Total Split (s)		24.0		22.5	22.5					46.0	46.0	46.0
Total Split (%)		34.3%		32.1%	32.1%					65.7%	65.7%	65.7%
Maximum Green (s)		18.6		18.0	18.0					40.6	40.6	40.6
Yellow Time (s)		3.2		3.5	3.5					3.2	3.2	3.2
All-Red Time (s)		2.2		1.0	1.0					2.2	2.2	2.2
Lost Time Adjust (s)		0.0			0.0						0.0	0.0
Total Lost Time (s)		5.4			4.5						5.4	5.4
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	3.0
Recall Mode		None		None	None					C-Max	C-Max	C-Max
Walk Time (s)		7.0		7.0	7.0					7.0	7.0	7.0
Flash Dont Walk (s)		11.0		11.0	11.0					11.0	11.0	11.0
Pedestrian Calls (#/hr)		0		0	0					0	0	0
Act Effct Green (s)		15.7			16.6						43.5	43.5
Actuated g/C Ratio		0.22			0.24						0.62	0.62
v/c Ratio		0.68			0.15						0.67	0.17
Control Delay		28.1			29.8						11.1	1.6
Queue Delay		0.0			0.0						0.0	0.0
Total Delay		28.2			29.8						11.1	1.6
LOS		С			С						В	A
Approach Delay		28.2			29.8						10.1	
Approach LOS		С			С						В	
90th %ile Green (s)		18.6		19.5	19.5					40.6	40.6	40.6
90th %ile Term Code		Max		Hold	Hold					Coord	Coord	Coord
70th %ile Green (s)		18.1		19.0	19.0					41.1	41.1	41.1
70th %ile Term Code		Gap		Hold	Hold					Coord	Coord	Coord
50th %ile Green (s)		16.4		17.3	17.3					42.8	42.8	42.8
50th %ile Term Code		Gap		Hold	Hold					Coord	Coord	Coord
30th %ile Green (s)		13.9		14.8	14.8					45.3	45.3	45.3
30th %ile Term Code		Gap		Hold	Hold					Coord	Coord	Coord
10th %ile Green (s)		11.4		12.3	12.3					47.8	47.8	47.8
10th %ile Term Code		Gap		Hold	Hold					Coord	Coord	Coord
Stops (vph)		423			63						852	13
Fuel Used(gal)		8			1						13	1
CO Emissions (g/hr)		549			58						942	56
NOx Emissions (g/hr)		107			11						183	11
VOC Emissions (g/hr)		127			14						218	13
Dilemma Vehicles (#)		0			0						0	0
Queue Length 50th (ft)		105			31						195	0
Queue Length 95th (ft)		148			m49						290	22
Internal Link Dist (ft)		579			164			780			446	
Turn Bay Length (ft)												150

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 3: N Madison St/N Center St & W Locust St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Base Capacity (vph)		936			500						2180	1050
Starvation Cap Reductn		0			0						0	0
Spillback Cap Reductn		4			0						0	0
Storage Cap Reductn		0			0						0	0
Reduced v/c Ratio		0.58			0.13						0.67	0.17
Intersection Summary												
Area Type:	Other											
Cycle Length: 70												
Actuated Cycle Length: 70												
Offset: 33 (47%), Reference	ed to phase	2: and 6:	SBTL, Sta	art of Gree	en							
Natural Cycle: 60												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.68												
Intersection Signal Delay: 7	15.0			In	tersectior	n LOS: B						
Intersection Capacity Utiliz	ersection Capacity Utilization 61.8% ICU Level of Service B											
Analysis Period (min) 15												
m Volume for 95th perce	ntile queue is	s metered	l by upstr	eam sign	al.							

Splits and Phases: 3: N Madison St/N Center St & W Locust St



02/22/2024

Lanes, Volumes, Timings <u>6: N East St/N Main St & W Locust St/E Locust St</u>

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		.at≜						41.				
Traffic Volume (vph)	102	542	0	0	0	0	60	944	209	0	0	0
Future Volume (vph)	102	542	0	0	0	0	60	944	209	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	0.95	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor		1.00						1.00				
Frt								0.974				
Flt Protected		0.992						0.998				
Satd. Flow (prot)	0	3511	0	0	0	0	0	3431	0	0	0	0
Flt Permitted		0.992						0.998				
Satd. Flow (perm)	0	3508	0	0	0	0	0	3431	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)								57				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		244			514			878			525	
Travel Time (s)		5.5			11.7			20.0			11.9	
Confl. Peds. (#/hr)	5								3			
Confl. Bikes (#/hr)									1			
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	110	583	0	0	0	0	65	1015	225	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	693	0	0	0	0	0	1305	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0	Ū		0	Ū		0	Ū		0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2					1	2				
Detector Template	Left	Thru					Left	Thru				
Leading Detector (ft)	20	100					20	100				
Trailing Detector (ft)	0	0					0	0				
Detector 1 Position(ft)	0	0					0	0				
Detector 1 Size(ft)	20	6					20	6				
Detector 1 Type	CI+Ex	Cl+Ex					CI+Ex	CI+Ex				
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0				
Detector 1 Queue (s)	0.0	0.0					0.0	0.0				
Detector 1 Delay (s)	0.0	0.0					0.0	0.0				
Detector 2 Position(ft)		94						94				
Detector 2 Size(ft)		6						6				
Detector 2 Type		Cl+Ex						CI+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0				
Turn Type	Perm	NA					Perm	NA				
Protected Phases		4						2				
Permitted Phases	4						2					

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings
6: N East St/N Main St & W Locust St/E Locust St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4					2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0					5.0	5.0				
Minimum Split (s)	23.5	23.5					23.5	23.5				
Total Split (s)	25.0	25.0					45.0	45.0				
Total Split (%)	35.7%	35.7%					64.3%	64.3%				
Maximum Green (s)	19.5	19.5					39.5	39.5				
Yellow Time (s)	32	32					32	32				
All-Red Time (s)	2.3	2.3					2.3	2.3				
Lost Time Adjust (s)		0.0						0.0				
Total Lost Time (s)		5.5						5.5				
Lead/Lag		0.0						0.0				
Lead-Lag Ontimize?												
Vehicle Extension (s)	30	3.0					3.0	3.0				
Recall Mode	None	None					C-Max	C-Max				
Walk Time (s)	7.0	7.0					7.0	7.0				
Flash Dont Walk (s)	11.0	11.0					11.0	11.0				
Pedestrian Calls (#/hr)	0	0					0	0				
Act Effet Green (s)	0	18.1					0	40.9				
Actuated a/C Ratio		0.26						0.58				
v/c Ratio		0.20						0.50				
Control Delay		20.1						73				
		20.1						1.5				
Total Delay		20.4						73				
		20.4						7.5				
LUG Approach Dolay		20.4						73				
Approach LOS		20.4						7.5				
Approach LOS	10 5	10.5					20.5	20.5				
90th %ile Green (s)	19.0 Mov	19.0 Mox					Coord	Coord				
70th %ile Croon (c)	10.5	10.5					20.5	20.5				
70th %ile Green (S)	19.5 Mov	19.0 Mox					Coord	Coord				
Foth %ile Croop (a)	10 5	10.5					20.5	20.5				
50th %ile Green (S)	19.5 Mov	19.0 Mox					Coord	Coord				
20th %ile Crean (a)	17.7	17 7					44.2	41.2				
20th %ile Green (S)	17.7 Con	17.7 Con					41.3 Coord	41.J				
10th %ile Croop (a)												
10th %ile Green (S)	14.Z	14.Z					44.0 Coord	44.0				
Stone (uph)	Gap	Gap					Coord	C0010				
		299						504 12				
Fuel Used(gal)		205						13				
CO Emissions (g/nr)		300						902				
NOX Emissions (g/hr)		75						1/5				
VUC Emissions (g/nr)		89						209				
Dilemma Venicles (#)		0						0				
		65						/6				
Queue Length 95th (ft)		126			40.4			92			445	
Internal LINK Dist (ft)		164			434			798			445	
Turn Bay Length (tt)		6						0000				
Base Capacity (vph)		977						2029				
Starvation Cap Reductn		50						0				

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings
6: N East St/N Main St & W Locust St/E Locust St

02/22/2024	
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Spillback Cap Reductn		0						0				
Storage Cap Reductn		0						0				
Reduced v/c Ratio		0.75						0.64				
Intersection Summary												
Area Type:	Other											
Cycle Length: 70												
Actuated Cycle Length: 70)											
Offset: 30 (43%), Referen	ced to phase	2:NBTL a	and 6:, St	art of Gre	en							
Natural Cycle: 60												
Control Type: Actuated-Co	oordinated											
Maximum v/c Ratio: 0.76												
Intersection Signal Delay:	11.9			In	tersectior	n LOS: B						
Intersection Capacity Utiliz	zation 61.7%			IC	U Level o	of Service	В					
Analysis Period (min) 15												
Solits and Phases: 6: N	East St/N Ma	ain St & V	V Locust :	St/E Locu	st St							

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Lanes, Volumes, Timings 9: N Madison St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,		۲	*						đ þ	
Traffic Volume (vph)	0	124	108	24	108	0	0	0	0	12	931	146
Future Volume (vph)	0	124	108	24	108	0	0	0	0	12	931	146
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	11	12	12	11	12	12	12	12	12	12	12
Grade (%)		1%			-2%			-6%			4%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		0.99		1.00							1.00	
Frt		0.937									0.980	
Flt Protected				0.950							0.999	
Satd. Flow (prot)	0	1501	0	1609	1637	0	0	0	0	0	3047	0
Flt Permitted				0.449							0.999	
Satd. Flow (perm)	0	1501	0	759	1637	0	0	0	0	0	3047	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		61									42	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		705			247			400			860	
Travel Time (s)		16.0			5.6			9.1			19.5	
Confl. Peds. (#/hr)			1	1								4
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	0	132	115	26	115	0	0	0	0	13	990	155
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	247	0	26	115	0	0	0	0	0	1158	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.15	1.20	1.15	1.13	1.18	1.13	1.10	1.10	1.10	1.17	1.17	1.17
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		Cl+Ex	CI+Ex					Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		CI+Ex			Cl+Ex						Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 9: N Madison St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		17.4		17.4	17.4					45.2	45.2	
Total Split (s)		24.1		24.1	24.1					45.9	45.9	
Total Split (%)		34.4%		34.4%	34.4%					65.6%	65.6%	
Maximum Green (s)		18.7		18.7	18.7					40.7	40.7	
Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)		2.2		2.2	2.2					2.0	2.0	
Lost Time Adjust (s)		0.0		0.0	0.0						0.0	
Total Lost Time (s)		5.4		5.4	5.4						5.2	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		None	None					C-Max	C-Max	
Walk Time (s)		1.0		1.0	1.0					29.0	29.0	
Flash Dont Walk (s)		11.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		13.7		13.7	13.7						45.7	
Actuated g/C Ratio		0.20		0.20	0.20						0.65	
v/c Ratio		0.72		0.18	0.36						0.58	
Control Delay		31.2		24.1	26.1						1.6	
Queue Delay		0.0		0.0	0.0						0.0	
Total Delay		31.2		24.1	26.1						1.6	
LOS		С		С	С						А	
Approach Delay		31.2			25.7						1.6	
Approach LOS		С			С						A	
90th %ile Green (s)		18.7		18.7	18.7					40.7	40.7	
90th %ile Term Code		Max		Hold	Hold					Coord	Coord	
70th %ile Green (s)		16.9		16.9	16.9					42.5	42.5	
70th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
50th %ile Green (s)		14.2		14.2	14.2					45.2	45.2	
50th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
30th %ile Green (s)		11.4		11.4	11.4					48.0	48.0	
30th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
10th %ile Green (s)		7.4		7.4	7.4					52.0	52.0	
10th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
Stops (vph)		159		21	88						48	
Fuel Used(gal)		4		0	1						8	
CO Emissions (g/hr)		254		20	89						554	
NOx Emissions (g/hr)		49		4	1/						108	
VOC Emissions (g/hr)		59		5	21						128	
Dilemma Vehicles (#)		0		0	0						0	
Queue Length 50th (ft)		74		9	43						12	
Queue Length 95th (ft)		136		27	79						15	
Internal Link Dist (ft)		625			167			320			780	
Turn Bay Length (ft)												
Base Capacity (vph)		445		202	437						2003	

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 9: N Madison St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn		0		0	0						0	
Spillback Cap Reductn		0		0	0						0	
Storage Cap Reductn		0		0	0						0	
Reduced v/c Ratio		0.56		0.13	0.26						0.58	
Intersection Summary												
Area Type:	CBD											
Cycle Length: 70												
Actuated Cycle Length: 70												
Offset: 52 (74%), Reference	ed to phase	2: and 6:	SBTL, Sta	art of Gree	en							
Natural Cycle: 65												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.72												
Intersection Signal Delay: 8	8.5			In	tersectior	n LOS: A						
Intersection Capacity Utilization	ation 65.2%			IC	U Level o	of Service	С					
Analysis Period (min) 15												

Splits and Phases: 9: N Madison St & W Market St

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	24.1s	
₩ Ø6 (R)	★ Ø8	
45.9 s	24.1 s	

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	1	LDR	HUL	<u>.</u>		
Traffic Volume (vnh)	137	0	126	1008	0	0
Future Volume (vph)	137	0	120	1000	0	0
Ideal Flow (vphpl)	1000	1900	1000	1000	1000	1000
Lane Width (ft)	1300	12	100	11	12	1000
Grade (%)	0%	12	12	_2%	2%	12
	1.00	1 00	0.05	-2 /0	2 /0	1 00
Pad Rika Factor	0.00	1.00	0.90	1.00	1.00	1.00
Ert	0.90			1.00		
Fit Protoctod	0.050			0.004		
Fil Piolecieu	0.900	0	0	0.994	0	0
Sato. Flow (prot)	1481	0	0	2/82	0	0
	0.950	^	^	0.994	^	^
Satd. Flow (perm)	1447	0	0	2782	0	0
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)						
Link Speed (mph)	30			30	30	
Link Distance (ft)	263			127	878	
Travel Time (s)	6.0			2.9	20.0	
Confl. Peds. (#/hr)	11		2			
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Parking (#/hr)	0		20	20		
Adi, Flow (vph)	146	0	134	1072	0	0
Shared Lane Traffic (%)				-		
Lane Group Flow (vph)	146	0	0	1206	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Loft	Right	Loft	Loft	Loft	Right
Modian Width(ft)	12	Tright	Leit			Night
	13			0	0	
	10			10	10	
	10			16	16	
I wo way Left I urn Lane	4.05		4.40	4.0.4	4.40	4.40
Headway Factor	1.25	1.14	1.13	1.34	1.16	1.16
Turning Speed (mph)	15	9	15			9
Number of Detectors	1		1	2		
Detector Template	Left		Left	Thru		
Leading Detector (ft)	20		20	100		
Trailing Detector (ft)	0		0	0		
Detector 1 Position(ft)	0		0	0		
Detector 1 Size(ft)	20		20	6		
Detector 1 Type	CI+Fx		Cl+Fx	CI+Fx		
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0		
	0.0		0.0	0.0		
Detector 1 Delay (a)	0.0		0.0	0.0		
Detector 1 Delay (S)	0.0		0.0	0.0		
Detector 2 Position(III)				94		
Detector 2 Size(tt)				6		
Detector 2 Type				CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)				0.0		
Turn Type	Prot		Perm	NA		

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Protected Phases	4			2		
Permitted Phases			2	2		
Detector Phase	4		2	2		
Switch Phase	7		2	2		
Minimum Initial (s)	5.0		5.0	5.0		
Minimum Split (s)	15 /		17.5	17.5		
Total Split (a)	20.0		50.0	50.0		
Total Split (8)	20.0		50.0	50.0 71 40/		
Total Split (%)	20.0%		11.4%	11.4%		
Maximum Green (s)	14.0		44.5	44.5		
Yellow Time (s)	3.2		3.2	3.Z		
All-Red Time (s)	2.2		2.3	2.3		
Lost Time Adjust (s)	0.0			0.0		
Total Lost Time (s)	5.4			5.5		
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0		3.0	3.0		
Recall Mode	None		C-Max	C-Max		
Walk Time (s)	1.0		31.0	31.0		
Flash Dont Walk (s)	9.0		11.0	11.0		
Pedestrian Calls (#/hr)	0		0	0		
Act Effct Green (s)	11.6			51.1		
Actuated g/C Ratio	0.17			0.73		
v/c Ratio	0.60			0.59		
Control Delay	36.9			8.0		
Queue Delav	0.0			0.0		
Total Delay	36.9			8.0		
LOS	D			A		
Approach Delay	36.9			8.0		
Approach LOS	00.5 D			Δ		
90th %ile Green (s)	14.6		11 5	11 5		
90th %ile Term Code	Max		Coord	Coord		
70th %ile Groop (s)	1/1 1		45.0	45.0		
70th %ile Term Code	14.1		40.0	40.0		
	Gap		47.0	47.0		
Soun %ile Green (S)	12.1		47.0	47.0		
	Gap		Coord	Coord		
30th %ile Green (s)	10.0		49.1	49.1		
30th %ile Term Code	Gap		Coord	Coord		
10th %ile Green (s)	0.0		64.5	64.5		
10th %ile Term Code	Skip		Coord	Coord		
Stops (vph)	122			559		
Fuel Used(gal)	2			6		
CO Emissions (g/hr)	139			424		
NOx Emissions (g/hr)	27			82		
VOC Emissions (g/hr)	32			98		
Dilemma Vehicles (#)	0			0		
Queue Length 50th (ft)	58			133		
Queue Length 95th (ft)	108			217		
Internal Link Dist (ff)	183			47	798	
Turn Bay Length (ft)						

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Base Capacity (vph)	308			2031			
Starvation Cap Reductn	0			0			
Spillback Cap Reductn	0			0			
Storage Cap Reductn	0			0			
Reduced v/c Ratio	0.47			0.59			
Intersection Summary							
Area Type:	CBD						
Cycle Length: 70							
Actuated Cycle Length: 70)						
Offset: 14 (20%), Reference	ced to phase 2	2:NBTL a	nd 6:, Sta	art of Gree	en		
Natural Cycle: 65							
Control Type: Actuated-Co	ordinated						
Maximum v/c Ratio: 0.60							
Intersection Signal Delay:	11.1			Int	tersectior	n LOS: B	
Intersection Capacity Utiliz	ation 52.5%			IC	U Level o	of Service A	
Analysis Period (min) 15							

Splits and Phases: 11: N East St & E Market St

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50 s		20 s	

Lanes, Volumes, Timings 12: N Madison St & W Monroe St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,			÷.						đ î þ	
Traffic Volume (vph)	0	9	0	31	25	0	0	0	0	3	1057	6
Future Volume (vph)	0	9	0	31	25	0	0	0	0	3	1057	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	11	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			-3%			6%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt											0.999	
Flt Protected					0.973							
Satd. Flow (prot)	0	1621	0	0	1631	0	0	0	0	0	3087	0
Flt Permitted					0.973							
Satd. Flow (perm)	0	1621	0	0	1631	0	0	0	0	0	3087	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		699			209			307			400	
Travel Time (s)		15.9			4.8			7.0			9.1	
Confl. Peds. (#/hr)										4		2
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Adj. Flow (vph)	0	10	0	35	28	0	0	0	0	3	1188	7
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	10	0	0	63	0	0	0	0	0	1198	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.19	1.14	1.14	1.14	1.14	1.12	1.12	1.12	1.19	1.19	1.19
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Free	
Intersection Summary												
Area Type: 0	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 49.5%			IC	CU Level	of Service	A					

Analysis Period (min) 15

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	3			At⊾		
Traffic Volume (vph)	48	0	19	1054	0	0
Future Volume (vph)	48	0	10	1054	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	11	12	12
Grade (%)	-2%	12	12	-3%	4%	12
Lane Util Factor	1 00	1 00	0.95	0.95	1 00	1 00
Ped Bike Factor	0.98	1.00	0.00	1.00	1.00	1.00
Frt	0.00			1.00		
Flt Protected	0 950			0 999		
Satd Flow (prot)	1//8	0	0	2810	٥	٥
Elt Permitted	0 050	U	U	0 000	U	U
Satd Flow (norm)	1/06	0	0	2810	0	0
Satu. Flow (perifi)	1420	Vee	U	2010	U	U Voo
Right Turn on Red		res				res
Satu. Flow (KTUK)	- 00					
Link Speed (mph)	30			30	30	
LINK Distance (ft)	221			318	266	
Travel Time (s)	5.0		_	7.2	6.0	
Contl. Peds. (#/hr)	5		7			
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Parking (#/hr)	0		20	20		
Adj. Flow (vph)	49	0	20	1087	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	49	0	0	1107	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.29	1.13	1.12	1.34	1.17	1.17
Turning Speed (mph)	15	9	15			9
Number of Detectors	1	Ŭ	1	2		v
Detector Template	l eft		l eft	Thru		
Leading Detector (ft)	20		20	100		
Trailing Detector (ft)	0		0	0		
Detector 1 Position(ft)	0		0	0		
Detector 1 Size(ft)	20		20	6		
Detector 1 Type						
Detector 1 Channel	OI+EX		UI+EX	UI+EX		
Detector 1 Channel	0.0		0.0	0.0		
Detector 1 Extend (S)	0.0		0.0	0.0		
Detector 1 Queue (s)	0.0		0.0	0.0		
Detector 1 Delay (s)	0.0		0.0	0.0		
Detector 2 Position(ft)				94		
Detector 2 Size(ft)				6		
Detector 2 Type				CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)				0.0		
Turn Type	Prot		Perm	NA		

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

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Lane Group	FBI	FBR	NBI	NBT	SBT	SBR	
Protected Phases	4			2	001	JUN	
Permitted Phases	-		2	2			
Detector Phase	1		2	2			
Switch Phase	4		2	2			
Minimum Initial (c)	5.0		50	5.0			
Minimum Initial (5)	12.0		10.2	10.2			
Total Split (s)	17.0		49.Z	49.Z			
Total Split (%)	21.3%		78.8%	78.8%			
Maximum Green (s)	21.370		57.8	57.8			
Vellow Time (s)	3.2		32	3.2			
All Ped Time (s)	2.0		2.0	2.0			
Lost Time Adjust (s)	2.0		2.0	2.0			
Total Lost Time (s)	5.2			5.2			
	J.Z			J.Z			
Leau/Lay							
Vehicle Extension (s)	3.0		30	3.0			
	Nono	(0.0 ℃_May	C_May			
Wolk Time (c)	1.0	,	22.0	22.0			
Flach Dont Walk (c)	7.0		11 0	11.0			
Podestrian Calls (#/br)	7.0		0	0			
Act Effet Green (s)	82		0	0 83			
Actuated a/C Patio	0.2			0.0			
v/c Patio	0.10			0.05			
Control Delay	38.6			1.40			
	0.0			0.0			
Total Delay	38.6			1.2			
	JU.U			1.2			
LUG Approach Dolay	38.6			1 2			
Approach LOS	J0.0			1.2			
90th %ile Green (s)	11 3		58 3	58.3			
90th %ile Term Code	Gan		Coord	Coord			
70th %ile Green (s)	0 <i>/</i>		60.2	60.2			
70th %ile Term Code	Gan		Coord	Coord			
50th %ile Green (s)	8 1		61 5	61 5			
50th %ile Term Code	Gan		Coord	Coord			
30th %ile Green (s)	0.0		74.8	74.8			
30th %ile Term Code	Skin		Coord	Coord			
10th %ile Green (s)	0.0		74 8	74.8			
10th %ile Term Code	Skin		Coord	Coord			
Stons (vnh)			COOld	62			
Fuel Lised(gal)	1			3			
CO Emissions (a/br)	10			228			
NOx Emissions (g/III)	10			11			
VOC Emissions (g/hr)	11			52			
Dilemma Vehicles (#)	0			0			
Oueue Length 50th (ft)	23			12			
Queue Length Joth (II)	53			24			
Internal Link Diet (ff)	1/1			24	186		
Turn Bay Length (ft)	1 7 1			200	100		

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

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			-		-		
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Base Capacity (vph)	213			2388			
Starvation Cap Reductn	0			0			
Spillback Cap Reductn	0			0			
Storage Cap Reductn	0			0			
Reduced v/c Ratio	0.23			0.46			
Intersection Summary							
Area Type:	CBD						
Cycle Length: 80							
Actuated Cycle Length: 80)						
Offset: 58 (73%), Referen	ced to phase 2	2:NBTL a	nd 6:, Sta	art of Gre	en		
Natural Cycle: 65							
Control Type: Actuated-C	oordinated						
Maximum v/c Ratio: 0.46							
Intersection Signal Delay:	2.8			In	tersection	LOS: A	
Intersection Capacity Utiliz	zation 45.8%			IC	U Level c	of Service A	
Analysis Period (min) 15							

Splits and Phases: 13: N East St & Monroe St

Lanes, Volumes, Timings 15: N Madison St & W Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1÷			र्स						4 P	
Traffic Volume (vph)	0	19	9	25	21	0	0	0	0	18	1046	17
Future Volume (vph)	0	19	9	25	21	0	0	0	0	18	1046	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	16	12	12	14	12	12	12	12	12	12	12
Grade (%)		4%			-3%			2%			2%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		0.99			1.00						1.00	
Frt		0.955									0.998	
Flt Protected					0.974						0.999	
Satd. Flow (prot)	0	1766	0	0	1591	0	0	0	0	0	3144	0
Flt Permitted					0.854						0.999	
Satd. Flow (perm)	0	1766	0	0	1388	0	0	0	0	0	3144	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		11									4	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		692			253			310			307	
Travel Time (s)		15.7			5.8			7.0			7.0	
Confl. Peds. (#/hr)			4	4						2		
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Parking (#/hr)					0					20		20
Adi, Flow (vph)	0	22	11	29	25	0	0	0	0	21	1231	20
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	33	0	0	54	0	0	0	0	0	1272	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0	Ŭ		0	J		12	Ū		12	Ū
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.17	1.00	1.17	1.12	1.18	1.12	1.16	1.16	1.16	1.16	1.16	1.16
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		CI+Ex	CI+Ex					Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		CI+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	
Turn Type		NA		Perm	NA					Perm	NA	

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 15: N Madison St & W Jefferson St

02/22/	2024
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		15.1		15.4	15.4					47.4	47.4	
Total Split (s)		23.0		23.0	23.0					57.0	57.0	
Total Split (%)		28.8%		28.8%	28.8%					71.3%	71.3%	
Maximum Green (s)		17.9		17.6	17.6					51.6	51.6	
Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)		1.9		2.2	2.2					2.2	2.2	
Lost Time Adjust (s)		0.0			0.0						0.0	
Total Lost Time (s)		5.1			5.4						5.4	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		Max		None	None					C-Max	C-Max	
Walk Time (s)		1.0		1.0	1.0					31.0	31.0	
Flash Dont Walk (s)		9.0		9.0	9.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0.0	0.0					0	0	
Act Effct Green (s)		17.9		Ŭ	17 6					Ŭ	51 6	
Actuated g/C Ratio		0.22			0.22						0.64	
v/c Ratio		0.08			0.18						0.63	
Control Delay		19.2			27.2						10.2	
Queue Delay		0.0			0.0						0.0	
Total Delay		19.2			27.2						10.2	
LOS		B			C						B	
Approach Delay		19.2			27.2						10.2	
Approach LOS		B			C						B	
90th %ile Green (s)		17.9		17 6	17.6					51.6	516	
90th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
70th %ile Green (s)		17.9		17.6	17.6					51.6	51.6	
70th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
50th %ile Green (s)		17.9		17.6	17.6					51.6	51.6	
50th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
30th %ile Green (s)		17.9		17.6	17.6					51.6	51.6	
30th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
10th %ile Green (s)		17.9		17.6	17.6					51.6	51.6	
10th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
Stops (vph)		17		11010	37					00014	595	
Fuel Used(gal)		0			1						8	
CO Emissions (g/hr)		25			38						567	
NOx Emissions (g/hr)		5			7						110	
VOC Emissions (g/hr)		6			9						131	
Dilemma Vehicles (#)		0			0 0						0	
Queue Length 50th (ft)		9			22						174	
Queue Length 95th (ft)		29			49						211	
Internal Link Dist (ft)		612			173			230			227	
Turn Bay Length (ft)												

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 15: N Madison St & W Jefferson St

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	100		•			20	1		1		•	200
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Base Capacity (vph)		403			305						2029	
Starvation Cap Reductn		0			0						0	
Spillback Cap Reductn		0			0						0	
Storage Cap Reductn		0			0						0	
Reduced v/c Ratio		0.08			0.18						0.63	
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80)											
Offset: 6 (8%), Referenced	d to phase 2:	and 6:SB	TL, Start	of Green								
Natural Cycle: 65												
Control Type: Actuated-Co	oordinated											
Maximum v/c Ratio: 0.63												
Intersection Signal Delay:	11.1			In	tersectior	n LOS: B						
Intersection Capacity Utiliz	zation 51.7%			IC	U Level o	of Service	A					
Analysis Period (min) 15												

Splits and Phases: 15: N Madison St & W Jefferson St

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	23 s
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57 s	23 s

Lanes, Volumes, Timings 16: N East St & E Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्स			f,			đ þ				
Traffic Volume (vph)	26	25	0	0	22	21	21	1032	6	0	0	0
Future Volume (vph)	26	25	0	0	22	21	21	1032	6	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	11	11	12	12	12
Grade (%)		-3%			3%			0%			3%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor		0.97			0.97			1.00				
Frt					0.934			0.999				
Flt Protected		0.975						0.999				
Satd. Flow (prot)	0	1493	0	0	1539	0	0	3073	0	0	0	0
Flt Permitted		0.845						0.999				
Satd. Flow (perm)	0	1257	0	0	1539	0	0	3072	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					22			2				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		268			710			308			318	
Travel Time (s)		6.1			16.1			7.0			7.2	
Confl. Peds. (#/hr)	25					25	17		4			
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Parking (#/hr)		0					20		20			
Adj. Flow (vph)	27	26	0	0	23	22	22	1075	6	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	53	0	0	45	0	0	1103	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0	J		0	Ū		0	Ū		0	Ŭ
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.12	1.28	1.12	1.17	1.12	1.17	1.14	1.19	1.19	1.17	1.17	1.17
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2			2		1	2				
Detector Template	Left	Thru			Thru		Left	Thru				
Leading Detector (ft)	20	100			100		20	100				
Trailing Detector (ft)	0	0			0		0	0				
Detector 1 Position(ft)	0	0			0		0	0				
Detector 1 Size(ft)	20	6			6		20	6				
Detector 1 Type	Cl+Ex	Cl+Ex			Cl+Ex		CI+Ex	CI+Ex				
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0				
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA		Perm	NA				

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 16: N East St & E Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases		4			8			2				
Permitted Phases	4						2					
Detector Phase	4	4			8		2	2				
Switch Phase					-							
Minimum Initial (s)	5.0	5.0			5.0		5.0	5.0				
Minimum Split (s)	15.2	15.2			15.2		47.2	47.2				
Total Split (s)	19.0	19.0			19.0		61.0	61.0				
Total Split (%)	23.8%	23.8%			23.8%		76.3%	76.3%				
Maximum Green (s)	13.8	13.8			13.8		55.8	55.8				
Yellow Time (s)	3.2	3.2			3.2		3.2	3.2				
All-Red Time (s)	2.0	2.0			2.0		2.0	2.0				
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.2			5.0			5.2				
Lead/Lag					•.=			•				
Lead-Lag Optimize?												
Vehicle Extension (s)	30	3.0			30		30	30				
Recall Mode	Max	Max			None		C-Max	C-Max				
Walk Time (s)	1.0	1.0			1.0		31.0	31.0				
Flash Dont Walk (s)	9.0	9.0			9.0		11.0	11.0				
Pedestrian Calls (#/hr)	0.0	0.0			0.0		0	0				
Act Effet Green (s)	Ū	13.8			13.8		0	55.8				
Actuated a/C Ratio		0.17			0.17			0.70				
v/c Ratio		0.17			0.16			0.70				
Control Delay		32.1			19.3			2.4				
		0.0			0.0			0.2				
Total Delay		32.1			19.3			2.7				
		C			10.0 B			Δ				
Approach Delay		32.1			19.3			27				
Approach LOS		02.1 C			13.5 B			Δ				
90th %ile Green (s)	13.8	13.8			13.8		55.8	55.8				
90th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
70th %ile Green (s)	13.8	13.8			13.8		55.8	55.8				
70th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
50th %ile Green (s)	13.8	13.8			13.8		55.8	55.8				
50th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
30th %ile Green (s)	13.8	13.8			13.8		55.8	55.8				
30th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
10th %ile Green (s)	13.8	13.8			13.8		55.8	55.8				
10th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
Stops (uph)	IVICALI	1/1			25		COOld	201				
Fuel Leed(gal)		1			25			201				
CO Emissions (a/br)		18			38			202				
NOv Emissions (g/m)		40			50			292				
VOC Emissions (g/hr)		11			0			68				
Dilemma Vehicles (#)					9			00				
Ouque Length 50th (#)					10			10				
Queue Length OUII (II)		20 E6			20			12				
Internal Link Dist (#)		100			630			22			220	
Turn Bay Length (ft)		100			030			220			200	

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 16: N East St & E Jefferson St

02/22/2024	
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Lane Group	EBL	EBT	EBR	• WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Base Capacity (vph)		216			283			2143				
Starvation Cap Reductn		0			0			366				
Spillback Cap Reductn		0			0			0				
Storage Cap Reductn		0			0			0				
Reduced v/c Ratio		0.25			0.16			0.62				
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 62 (78%), Reference	ed to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 65												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.51												
Intersection Signal Delay: 4	4.6			In	tersectior	n LOS: A						
Intersection Capacity Utiliz	ation 53.4%			IC	U Level o	of Service	А					
Analysis Period (min) 15												

Splits and Phases: 16: N East St & E Jefferson St

Ø2 (R)	▲ _{Ø4}
61s	19 s
	← Ø8
	19 s

Lanes, Volumes, Timings 19: N Madison St & W Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ħ		2	•					5	† 12	
Traffic Volume (vph)	0	126	8	59	228	0	0	0	0	58	1011	40
Future Volume (vph)	0	126	8	59	228	0	0	0	0	58	1011	40
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	13	12	12	13	12	12	12	12	12	12	12
Grade (%)		3%			-3%			2%			-2%	
Storage Length (ft)	0		0	65		0	0		0	150		0
Storage Lanes	0		0	1		0	0		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95
Ped Bike Factor		1.00		0.99						1.00	1.00	
Frt		0.992									0.994	
Flt Protected				0.950						0.950		
Satd. Flow (prot)	0	1522	0	1617	1758	0	0	0	0	1287	3195	0
Flt Permitted				0.666						0.950		
Satd. Flow (perm)	0	1522	0	1126	1758	0	0	0	0	1283	3195	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		4									7	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		690			253			324			310	
Travel Time (s)		15.7			5.8			7.4			7.0	
Confl. Peds. (#/hr)			4	4						2		3
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Parking (#/hr)	-	0				-	-	-	-	20		20
Adj. Flow (vph)	0	133	8	62	240	0	0	0	0	61	1064	42
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	141	0	62	240	0	0	0	0	61	1106	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
		0			0			0			0	
		10			10 Vac			10			10	
Two way Leπ Turn Lane	4 47	1 00	1 17	1 10	res	1 10	1 10	1 10	1 10	1 40	1 1 2	1 1 2
Headway Factor	1.17	1.20	1.17	1. IZ	1.08	1.12	1.10	1.10	01.1	1.49	1.13	1.13
Turning Speed (mpn)	15	n	9	10	0	9	15		9	10	0	9
Number of Delectors		Z		l off	Z					l off	Z	
Leading Detector (ft)		100		Leit	100					20	100	
Trailing Detector (II)		100		20	100					20	100	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		0		20	6					20	6	
Detector 1 Type		CI±Ev										
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		9 <u>1</u>		0.0	94					0.0	94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		Cl+Fx			Cl+Fx						Cl+Fx	
		J/										

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 19: N Madison St & W Washington St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		17.4		27.4	27.4					35.6	35.6	
Total Split (s)		32.0		32.0	32.0					48.0	48.0	
Total Split (%)		40.0%		40.0%	40.0%					60.0%	60.0%	
Maximum Green (s)		26.6		26.6	26.6					42.4	42.4	
Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)		2.2		2.2	2.2					2.4	2.4	
Lost Time Adjust (s)		0.0		0.0	0.0					0.0	0.0	
Total Lost Time (s)		5.4		5.4	5.4					5.6	5.6	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		None	None					C-Max	C-Max	
Walk Time (s)		1.0		11.0	11.0					19.0	19.0	
Flash Dont Walk (s)		11.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		16.3		16.3	16.3					52.7	52.7	
Actuated g/C Ratio		0.20		0.20	0.20					0.66	0.66	
v/c Ratio		0.45		0.27	0.67					0.07	0.53	
Control Delay		30.5		16.3	23.2					5.3	6.0	
Queue Delay		0.0		0.0	0.2					0.0	0.1	
Total Delay		30.5		16.3	23.4					5.3	6.1	
LOS		С		В	С					А	А	
Approach Delay		30.5			21.9						6.1	
Approach LOS		С			С						А	
90th %ile Green (s)		22.9		22.9	22.9					46.1	46.1	
90th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
70th %ile Green (s)		18.7		18.7	18.7					50.3	50.3	
70th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
50th %ile Green (s)		16.2		16.2	16.2					52.8	52.8	
50th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
30th %ile Green (s)		13.7		13.7	13.7					55.3	55.3	
30th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
10th %ile Green (s)		10.2		10.2	10.2					58.8	58.8	
10th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
Stops (vph)		107		14	56					15	266	
Fuel Used(gal)		2		0	2					0	5	
CO Emissions (g/hr)		150		27	128					20	371	
NOx Emissions (g/hr)		29		5	25					4	72	
VOC Emissions (g/hr)		35		6	30					5	86	
Dilemma Vehicles (#)		0		0	0					0	0	
Queue Length 50th (ft)		60		8	33					8	81	

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 19: N Madison St & W Washington St

02/22/2024	02	22	20)24
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)		102		17	47					m14	99	
Internal Link Dist (ft)		610			173			244			230	
Turn Bay Length (ft)				65						150		
Base Capacity (vph)		508		374	584					844	2105	
Starvation Cap Reductn		0		0	44					0	201	
Spillback Cap Reductn		0		0	0					0	0	
Storage Cap Reductn		0		0	0					0	0	
Reduced v/c Ratio		0.28		0.17	0.44					0.07	0.58	
Intersection Summary												
Area Type: Cl	BD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 3 (4%), Referenced to	phase 2:	and 6:SB	TL, Start	of Green								
Natural Cycle: 65												
Control Type: Actuated-Coord	inated											
Maximum v/c Ratio: 0.67												
Intersection Signal Delay: 11.2	2			In	tersectior	ILOS: B						
Intersection Capacity Utilization	on 58.5%			IC	U Level o	of Service	В					
Analysis Period (min) 15												
m Volume for 95th percentil	e queue is	s metered	l by upstr	eam sign	al.							
Splits and Phases: 19: N M	adison St	& W Was	shington S	St								

	→ Ø4	
	32 s	
Ø6 (R)	₩Ø8	
48 s	32 s	

Lanes, Volumes, Timings 20: N East St & E Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٢	•			*	1	5	1				
Traffic Volume (vph)	34	222	0	0	285	87	31	906	69	0	0	0
Future Volume (vph)	34	222	0	0	285	87	31	906	69	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	11	10	11	11	11	12	12	12
Grade (%)		-3%			2%			-3%			1%	
Storage Length (ft)	65		0	0		80	80		0	0		0
Storage Lanes	1		0	0		1	1		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	1.00					0.98	1.00	1.00				
Frt						0.850		0.989				
Flt Protected	0.950						0.950					
Satd. Flow (prot)	1617	1531	0	0	1604	1317	1250	3085	0	0	0	0
Flt Permitted	0.382						0.950					
Satd. Flow (perm)	648	1531	0	0	1604	1292	1245	3085	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)						87		15				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		262			724			322			308	
Travel Time (s)		6.0			16.5			7.3			7.0	
Confl. Peds. (#/hr)	4					4	4		9			
Confl. Bikes (#/hr)						1						
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Parking (#/hr)		0					20		20			
Adj. Flow (vph)	37	239	0	0	306	94	33	974	74	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	37	239	0	0	306	94	33	1048	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			11			11	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.12	1.28	1.12	1.16	1.21	1.26	1.54	1.17	1.17	1.15	1.15	1.15
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2			2	1	1	2				
Detector Template	Left	Thru			Thru	Right	Left	Thru				
Leading Detector (ft)	20	100			100	20	20	100				
Trailing Detector (ft)	0	0			0	0	0	0				
Detector 1 Position(ft)	0	0			0	0	0	0				
Detector 1 Size(ft)	20	6			6	20	20	6				
Detector 1 Type	CI+Ex	CI+Ex			CI+Ex	Cl+Ex	Cl+Ex	CI+Ex				
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 20: N East St & E Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Type		CI+Ex			Cl+Ex			CI+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA	Perm	Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4					8	2					
Detector Phase	4	4			8	8	2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0	5.0	5.0	5.0				
Minimum Split (s)	31.4	31.4			21.4	21.4	47.0	47.0				
Total Split (s)	32.0	32.0			32.0	32.0	48.0	48.0				
Total Split (%)	40.0%	40.0%			40.0%	40.0%	60.0%	60.0%				
Maximum Green (s)	26.6	26.6			26.6	26.6	42.0	42.0				
Yellow Time (s)	3.2	3.2			3.2	3.2	3.2	3.2				
All-Red Time (s)	2.2	2.2			2.2	2.2	2.8	2.8				
Lost Time Adjust (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Total Lost Time (s)	5.4	5.4			5.4	5.4	6.0	6.0				
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0	3.0	3.0	3.0				
Recall Mode	None	None			None	None	C-Max	C-Max				
Walk Time (s)	15.0	15.0			5.0	5.0	30.0	30.0				
Flash Dont Walk (s)	11.0	11.0			11.0	11.0	11.0	11.0				
Pedestrian Calls (#/hr)	0	0			0	0	0	0				
Act Effct Green (s)	20.4	20.4			20.4	20.4	48.2	48.2				
Actuated g/C Ratio	0.26	0.26			0.26	0.26	0.60	0.60				
v/c Ratio	0.22	0.61			0.75	0.24	0.04	0.56				
Control Delay	29.5	39.1			38.7	7.2	4.5	6.7				
Queue Delay	0.0	0.2			0.5	0.0	0.0	0.1				
Total Delay	29.5	39.3			39.2	7.2	4.5	6.9				
LOS	С	D			D	А	А	А				
Approach Delay		38.0			31.7			6.8				
Approach LOS		D			С			А				
90th %ile Green (s)	26.6	26.6			26.6	26.6	42.0	42.0				
90th %ile Term Code	Hold	Hold			Max	Max	Coord	Coord				
70th %ile Green (s)	24.1	24.1			24.1	24.1	44.5	44.5				
70th %ile Term Code	Hold	Hold			Gap	Gap	Coord	Coord				
50th %ile Green (s)	20.5	20.5			20.5	20.5	48.1	48.1				
50th %ile Term Code	Hold	Hold			Gap	Gap	Coord	Coord				
30th %ile Green (s)	17.5	17.5			17.5	17.5	51.1	51.1				
30th %ile Term Code	Hold	Hold			Gap	Gap	Coord	Coord				
10th %ile Green (s)	13.1	13.1			13.1	13.1	55.5	55.5				
10th %ile Term Code	Hold	Hold			Gap	Gap	Coord	Coord				
Stops (vph)	21	146			251	19	6	395				
Fuel Used(gal)	0	3			5	1	0	6				
CO Emissions (g/hr)	27	212			366	51	10	417				
NOx Emissions (g/hr)	5	41			71	10	2	81				
VOC Emissions (g/hr)	6	49			85	12	2	97				
Dilemma Vehicles (#)	0	0			0	0	0	0				

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 20: N East St & E Washington St

02/22/20	24
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Lane Group	EBL	EBT	EBR	• WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)	12	81			141	3	3	52				
Queue Length 95th (ft)	33	134			205	33	m7	108				
Internal Link Dist (ft)		182			644			242			228	
Turn Bay Length (ft)	65					80	80					
Base Capacity (vph)	215	509			533	487	750	1865				
Starvation Cap Reductn	0	30			0	0	0	161				
Spillback Cap Reductn	0	0			46	0	0	0				
Storage Cap Reductn	0	0			0	0	0	0				
Reduced v/c Ratio	0.17	0.50			0.63	0.19	0.04	0.62				
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 60 (75%), Reference	ed to phase	2:NBTL a	ind 6:, Sta	art of Gre	en							
Natural Cycle: 80												
Control Type: Actuated-Coc	ordinated											
Maximum v/c Ratio: 0.75												
Intersection Signal Delay: 1	7.4			In	tersectior	n LOS: B						
Intersection Capacity Utiliza	tion 69.0%			IC	U Level o	of Service	С					
Analysis Period (min) 15												
m Volume for 95th percen	ntile queue is	s metered	l by upstr	eam sign	al.							

Splits and Phases: 20: N East St & E Washington St

Ø2 (R)		- 22
48 s	32 s	
46.6.1.752		
	Ø8	
	32 s	

Lanes, Volumes, Timings 23: S Center St/N Madison St & W Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,		2	+						đ þ	
Traffic Volume (vph)	0	35	13	53	65	0	0	0	0	22	1026	27
Future Volume (vph)	0	35	13	53	65	0	0	0	0	22	1026	27
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	13	12	12	12	12	12	12	12	12
Grade (%)		3%			-3%			1%			-1%	
Storage Length (ft)	0		0	60		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	0		0
Taper Length (ft)	25			0			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		0.99		0.98							1.00	
Frt		0.964									0.996	
Flt Protected				0.950							0.999	
Satd. Flow (prot)	0	1421	0	1670	1531	0	0	0	0	0	3183	0
Flt Permitted				0.725							0.999	
Satd. Flow (perm)	0	1421	0	1252	1531	0	0	0	0	0	3182	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		13									5	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		684			247			660			324	
Travel Time (s)		15.5			5.6			15.0			7.4	
Confl. Peds. (#/hr)			8	8						13		4
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Parking (#/hr)		0			0					20		20
Adj. Flow (vph)	0	36	13	55	67	0	0	0	0	23	1058	28
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	49	0	55	67	0	0	0	0	0	1109	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		13			13			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.17	1.33	1.17	1.08	1.28	1.12	1.15	1.15	1.15	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		Cl+Ex	CI+Ex					Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		Cl+Ex			Cl+Ex						Cl+Ex	

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 23: S Center St/N Madison St & W Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		15.5		25.5	25.5					37.7	37.7	
Total Split (s)		29.0		29.0	29.0					51.0	51.0	
Total Split (%)		36.3%		36.3%	36.3%					63.8%	63.8%	
Maximum Green (s)		23.5		23.5	23.5					45.3	45.3	
Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)		2.3		2.3	2.3					2.5	2.5	
Lost Time Adjust (s)		0.0		0.0	0.0						0.0	
Total Lost Time (s)		5.5		5.5	5.5						5.7	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		None	None					C-Max	C-Max	
Walk Time (s)		1.0		9.0	9.0					21.0	21.0	
Flash Dont Walk (s)		9.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		9.0		9.0	9.0						63.1	
Actuated g/C Ratio		0.11		0.11	0.11						0.79	
v/c Ratio		0.28		0.39	0.39						0.44	
Control Delay		29.0		40.0	38.5						0.9	
Queue Delay		0.0		0.0	0.0						0.0	
Total Delay		29.0		40.0	38.5						0.9	
LOS		С		D	D						А	
Approach Delay		29.0			39.2						0.9	
Approach LOS		С			D						А	
90th %ile Green (s)		12.9		12.9	12.9					55.9	55.9	
90th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
70th %ile Green (s)		10.5		10.5	10.5					58.3	58.3	
70th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
50th %ile Green (s)		8.9		8.9	8.9					59.9	59.9	
50th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
30th %ile Green (s)		7.4		7.4	7.4					61.4	61.4	
30th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
10th %ile Green (s)		0.0		0.0	0.0					74.3	74.3	
10th %ile Term Code		Skip		Skip	Skip					Coord	Coord	
Stops (vph)		35		48	58						45	
Fuel Used(gal)		1		1	1						3	
CO Emissions (g/hr)		51		56	67						221	
NOx Emissions (g/hr)		10		11	13						43	
VOC Emissions (g/hr)		12		13	15						51	
Dilemma Vehicles (#)		0		0	0						0	
Queue Length 50th (ft)		17		26	32						11	

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 23: S Center St/N Madison St & W Front St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)		46		58	66						14	
Internal Link Dist (ft)		604			167			580			244	
Turn Bay Length (ft)				60								
Base Capacity (vph)		426		367	449						2510	
Starvation Cap Reductn		0		0	0						99	
Spillback Cap Reductn		0		0	0						0	
Storage Cap Reductn		0		0	0						0	
Reduced v/c Ratio		0.12		0.15	0.15						0.46	
Intersection Summary												
Area Type: Cł	BD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 8 (10%), Referenced to	o phase 2	and 6:S	BTL, Star	t of Gree	n							
Natural Cycle: 65												
Control Type: Actuated-Coord	linated											
Maximum v/c Ratio: 0.44												
Intersection Signal Delay: 5.7				In	tersectior	n LOS: A						
Intersection Capacity Utilization	on 52.4%			IC	U Level o	of Service	А					
Analysis Period (min) 15												

Splits and Phases: 23: S Center St/N Madison St & W Front St

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02/22/2024

Lanes, Volumes, Timings 24: S East St/N East St & E Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	1			1	1		đ þ				
Traffic Volume (vph)	82	115	0	0	48	22	43	910	29	0	0	0
Future Volume (vph)	82	115	0	0	48	22	43	910	29	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	16	12	12	10	10	13	11	11	12	12	12
Grade (%)		-1%			0%			-1%			2%	
Storage Length (ft)	90		0	0		100	0		0	0		0
Storage Lanes	1		0	0		1	0		0	0		0
Taper Length (ft)	0			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.97					0.95		1.00				
Frt						0.850		0.996				
Flt Protected	0.950							0.998				
Satd. Flow (prot)	1547	1910	0	0	1565	1330	0	3074	0	0	0	0
Flt Permitted	0.721							0.998				
Satd. Flow (perm)	1135	1910	0	0	1565	1267	0	3071	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)						35		6				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		256			710			633			322	
Travel Time (s)		5.8			16.1			14.4			7.3	
Confl. Peds. (#/hr)	18					18	28		1			
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Parking (#/hr)							5					
Adj. Flow (vph)	94	132	0	0	55	25	49	1046	33	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	94	132	0	0	55	25	0	1128	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		11			11			11			11	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.19	0.97	1.14	1.14	1.25	1.25	1.09	1.19	1.19	1.16	1.16	1.16
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2			2	1	1	2				
Detector Template	Left	Thru			Thru	Right	Left	Thru				
Leading Detector (ft)	20	100			100	20	20	100				
Trailing Detector (ft)	0	0			0	0	0	0				
Detector 1 Position(ft)	0	0			0	0	0	0				
Detector 1 Size(ft)	20	6			6	20	20	6				
Detector 1 Type	CI+Ex	Cl+Ex			CI+Ex	CI+Ex	CI+Ex	CI+Ex				
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			Cl+Ex			CI+Ex				

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 24: S East St/N East St & E Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA	Perm	Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4					8	2					
Detector Phase	4	4			8	8	2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0	5.0	5.0	5.0				
Minimum Split (s)	27.2	27.2			17.2	17.2	50.6	50.6				
Total Split (s)	29.2	29.2			29.2	29.2	50.8	50.8				
Total Split (%)	36.5%	36.5%			36.5%	36.5%	63.5%	63.5%				
Maximum Green (s)	24.0	24.0			24.0	24.0	45.2	45.2				
Yellow Time (s)	3.2	3.2			3.2	3.2	3.2	3.2				
All-Red Time (s)	2.0	2.0			2.0	2.0	2.4	2.4				
Lost Time Adjust (s)	0.0	0.0			0.0	0.0		0.0				
Total Lost Time (s)	5.2	5.2			5.2	5.2		5.6				
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0	3.0	3.0	3.0				
Recall Mode	None	None			None	None	C-Max	C-Max				
Walk Time (s)	11.0	11.0			1.0	1.0	34.0	34.0				
Flash Dont Walk (s)	11.0	11.0			11.0	11.0	11.0	11.0				
Pedestrian Calls (#/hr)	0	0			0	0	0	0				
Act Effct Green (s)	12.1	12.1			12.1	12.1		57.1				
Actuated g/C Ratio	0.15	0.15			0.15	0.15		0.71				
v/c Ratio	0.55	0.46			0.23	0.11		0.51				
Control Delay	42.6	35.0			30.5	8.3		5.0				
Queue Delay	0.0	0.0			0.0	0.0		0.0				
Total Delay	42.6	35.0			30.5	8.3		5.0				
LOS	D	D			С	А		А				
Approach Delay		38.2			23.5			5.0				
Approach LOS		D			С			А				
90th %ile Green (s)	17.7	17.7			17.7	17.7	51.5	51.5				
90th %ile Term Code	Gap	Gap			Hold	Hold	Coord	Coord				
70th %ile Green (s)	14.3	14.3			14.3	14.3	54.9	54.9				
70th %ile Term Code	Gap	Gap			Hold	Hold	Coord	Coord				
50th %ile Green (s)	11.9	11.9			11.9	11.9	57.3	57.3				
50th %ile Term Code	Gap	Gap			Hold	Hold	Coord	Coord				
30th %ile Green (s)	9.6	9.6			9.6	9.6	59.6	59.6				
30th %ile Term Code	Gap	Gap			Hold	Hold	Coord	Coord				
10th %ile Green (s)	6.8	6.8			6.8	6.8	62.4	62.4				
10th %ile Term Code	Gap	Gap			Hold	Hold	Coord	Coord				
Stops (vph)	74	99			40	5		284				
Fuel Used(gal)	1	2			1	0		7				
CO Emissions (g/hr)	90	112			55	13		518				
NOx Emissions (g/hr)	17	22			11	3		101				
VOC Emissions (g/hr)	21	26			13	3		120				
Dilemma Vehicles (#)	0	0			0	0		0				
Queue Length 50th (ft)	44	61			25	0		71				

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 24: S East St/N East St & E Front St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)	81	100			50	14		104				
Internal Link Dist (ft)		176			630			553			242	
Turn Bay Length (ft)	90					100						
Base Capacity (vph)	340	573			469	404		2195				
Starvation Cap Reductn	0	0			0	0		0				
Spillback Cap Reductn	0	0			0	0		17				
Storage Cap Reductn	0	0			0	0		0				
Reduced v/c Ratio	0.28	0.23			0.12	0.06		0.52				
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 58 (73%), Reference	ed to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 80												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.55												
Intersection Signal Delay: 1	11.3			In	tersectior	n LOS: B						
Intersection Capacity Utilization	ation 62.7%			IC	U Level o	of Service	В					
Analysis Period (min) 15												

Splits and Phases: 24: S East St/N East St & E Front St

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50.8 s	29.2 s	
	4 [⊕] Ø8	
	29.2 s	

Lanes, Volumes, Timings 27: N Center St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,			र्स						đ þ	
Traffic Volume (vph)	0	134	28	33	108	0	0	0	0	33	121	15
Future Volume (vph)	0	134	28	33	108	0	0	0	0	33	121	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt		0.977									0.986	
Flt Protected					0.988						0.990	
Satd. Flow (prot)	0	1606	0	0	1462	0	0	0	0	0	2897	0
Flt Permitted					0.988						0.990	
Satd. Flow (perm)	0	1606	0	0	1462	0	0	0	0	0	2897	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		247			266			264			413	
Travel Time (s)		5.6			6.0			6.0			9.4	
Confl. Peds. (#/hr)			30	30						6		5
Confl. Bikes (#/hr)			1									
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)					0						0	
Adj. Flow (vph)	0	149	31	37	120	0	0	0	0	37	134	17
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	180	0	0	157	0	0	0	0	0	188	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.14	1.14	1.14	1.30	1.14	1.14	1.14	1.14	1.14	1.22	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: 0	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 37.1%			IC	CU Level	of Service	А					
Analysis Period (min) 15												

Lanes, Volumes, Timings 29: N Center St & W Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		Þ			र्स						4 P	
Traffic Volume (vph)	0	35	4	9	39	0	0	0	0	35	96	10
Future Volume (vph)	0	35	4	9	39	0	0	0	0	35	96	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt		0.985									0.989	
Flt Protected					0.991						0.988	
Satd. Flow (prot)	0	1458	0	0	1466	0	0	0	0	0	2900	0
Flt Permitted					0.991						0.988	
Satd. Flow (perm)	0	1458	0	0	1466	0	0	0	0	0	2900	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		253			255			306			278	
Travel Time (s)		5.8			5.8			7.0			6.3	
Confl. Peds. (#/hr)			5	5						20		8
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)		0			0						0	
Adj. Flow (vph)	0	41	5	10	45	0	0	0	0	41	112	12
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	46	0	0	55	0	0	0	0	0	165	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.14	1.14	1.14	1.22	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: 0	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 22.7%			IC	CU Level	of Service	А					

Analysis Period (min) 15

Lanes, Volumes, Timings 30: N Center St & W Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,		5	+						đ î þ	
Traffic Volume (vph)	0	193	5	8	286	0	0	0	0	62	42	7
Future Volume (vph)	0	193	5	8	286	0	0	0	0	62	42	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	65		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		1.00		0.99							0.98	
Frt		0.997									0.991	
Flt Protected				0.950							0.973	
Satd. Flow (prot)	0	1474	0	1562	1480	0	0	0	0	0	2854	0
Flt Permitted				0.563							0.973	
Satd. Flow (perm)	0	1474	0	916	1480	0	0	0	0	0	2804	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		3									7	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		253			254			326			306	
Travel Time (s)		5.8			5.8			7.4			7.0	
Confl. Peds. (#/hr)			10	10						14		12
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)		0			0						0	
Adj. Flow (vph)	0	201	5	8	298	0	0	0	0	65	44	7
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	206	0	8	298	0	0	0	0	0	116	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.14	1.14	1.14	1.22	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		Cl+Ex	CI+Ex					CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		CI+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE
Lanes, Volumes, Timings 30: N Center St & W Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		23.0		23.0	23.0					23.0	23.0	
Total Split (s)		52.0		52.0	52.0					28.0	28.0	
Total Split (%)		65.0%		65.0%	65.0%					35.0%	35.0%	
Maximum Green (s)		47.0		47.0	47.0					23.0	23.0	
Yellow Time (s)		4.0		4.0	4.0					4.0	4.0	
All-Red Time (s)		1.0		1.0	1.0					1.0	1.0	
Lost Time Adjust (s)		0.0		0.0	0.0						0.0	
Total Lost Time (s)		5.0		5.0	5.0						5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		None	None					C-Max	C-Max	
Walk Time (s)		7.0		7.0	7.0					7.0	7.0	
Flash Dont Walk (s)		11.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		22.9		22.9	22.9						47.1	
Actuated g/C Ratio		0.29		0.29	0.29						0.59	
v/c Ratio		0.49		0.03	0.70						0.07	
Control Delay		13.6		3.1	16.6						8.6	
Queue Delay		0.0		0.0	0.1						0.0	
Total Delay		13.7		3.1	16.7						8.6	
LOS		В		А	В						А	
Approach Delay		13.7			16.3						8.6	
Approach LOS		В			В						А	
90th %ile Green (s)		32.6		32.6	32.6					37.4	37.4	
90th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
70th %ile Green (s)		26.5		26.5	26.5					43.5	43.5	
70th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
50th %ile Green (s)		22.6		22.6	22.6					47.4	47.4	
50th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
30th %ile Green (s)		18.9		18.9	18.9					51.1	51.1	
30th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
10th %ile Green (s)		13.7		13.7	13.7					56.3	56.3	
10th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
Stops (vph)		68		3	250						47	
Fuel Used(gal)		1		0	3						1	
CO Emissions (g/hr)		92		3	204						50	
NOx Emissions (g/hr)		18		1	40						10	
VOC Emissions (g/hr)		21		1	47						12	
Dilemma Vehicles (#)		0		0	0						0	
Queue Length 50th (ft)		38		0	78						11	
Queue Length 95th (ft)		59		m1	94						29	

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 30: N Center St & W Washington St

02/22/2024	ŀ
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		173			174			246			226	
Turn Bay Length (ft)				65								
Base Capacity (vph)		867		538	869						1655	
Starvation Cap Reductn		62		0	94						0	
Spillback Cap Reductn		0		0	0						0	
Storage Cap Reductn		0		0	0						0	
Reduced v/c Ratio		0.26		0.01	0.38						0.07	
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80)											
Offset: 21 (26%), Reference	ced to phase	2: and 6:	SBTL, Sta	art of Gre	en							
Natural Cycle: 50												
Control Type: Actuated-Co	oordinated											
Maximum v/c Ratio: 0.70												
Intersection Signal Delay:	14.0			In	tersectior	n LOS: B						
Intersection Capacity Utiliz	zation 51.3%			IC	U Level o	of Service	A					
Analysis Period (min) 15												
m Valuma for OFth pares	natila autorra i	a matara	المبري بالم	aam alan	a l							

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 30: N Center St & W Washington St

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	52 s	
Ø6 (R)	₩ Ø8	
28 s	52 s	

Lanes, Volumes, Timings 31: W Front St & N Center St

02/22/202	24
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,		7	1			\$		7	ħ	
Traffic Volume (vph)	0	54	5	5	110	0	0	0	1	46	3	9
Future Volume (vph)	0	54	5	5	110	0	0	0	1	46	3	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	8	12	12	12	12
Storage Length (ft)	0		0	70		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.989						0.865			0.890	
Flt Protected				0.950						0.950		
Satd. Flow (prot)	0	1396	0	1490	1412	0	0	1176	0	1341	1257	0
Flt Permitted				0.950						0.950		
Satd. Flow (perm)	0	1396	0	1490	1412	0	0	1176	0	1341	1257	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		247			262			221			326	
Travel Time (s)		5.6			6.0			5.0			7.4	
Confl. Peds. (#/hr)			48	48			25		13	13		25
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	9%	9%	9%	9%	9%	9%	9%	9%	9%	9%	9%	9%
Parking (#/hr)		0			0					0	0	
Adj. Flow (vph)	0	66	6	6	134	0	0	0	1	56	4	11
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	72	0	6	134	0	0	1	0	56	15	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		13			13			0			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane					Yes							
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.37	1.14	1.30	1.30	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: C	BD											
Control Type: Unsignalized												
Intersection Capacity Utilization	on 29.7%			IC	U Level o	of Service	А					
Analysis Period (min) 15												

Lanes, Volumes, Timings 34: N Main St & W Market St/E Market St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		÷.			Þ			đ î de				
Traffic Volume (vph)	32	130	0	0	115	8	43	58	25	0	0	0
Future Volume (vph)	32	130	0	0	115	8	43	58	25	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.992			0.970				
Flt Protected		0.990						0.983				
Satd. Flow (prot)	0	1479	0	0	1482	0	0	2857	0	0	0	0
Flt Permitted		0.990						0.983				
Satd. Flow (perm)	0	1479	0	0	1482	0	0	2857	0	0	0	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		266			263			249			404	
Travel Time (s)		6.0			6.0			5.7			9.2	
Confl. Peds. (#/hr)	19					19	20		18			
Confl. Bikes (#/hr)						1			1			
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Parking (#/hr)		0			0			0				
Adj. Flow (vph)	37	151	0	0	134	9	50	67	29	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	188	0	0	143	0	0	146	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.22	1.14	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: 0	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 38.2%			IC	CU Level of	of Service	А					
Analysis Period (min) 15												

Lanes, Volumes, Timings 36: N Main St & W Jefferson St/E Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ŧ			ħ			đ þ				
Traffic Volume (vph)	33	35	0	0	28	15	14	75	8	0	0	0
Future Volume (vph)	33	35	0	0	28	15	14	75	8	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.953			0.987				
Flt Protected		0.976						0.993				
Satd. Flow (prot)	0	1404	0	0	1371	0	0	2827	0	0	0	0
Flt Permitted		0.976						0.993				
Satd. Flow (perm)	0	1404	0	0	1371	0	0	2827	0	0	0	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		255			268			306			272	
Travel Time (s)		5.8			6.1			7.0			6.2	
Confl. Peds. (#/hr)	11					11	3		22			
Confl. Bikes (#/hr)									2			
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83
Heavy Vehicles (%)	7%	7%	7%	7%	7%	7%	7%	7%	7%	7%	7%	7%
Parking (#/hr)		0			0			0				
Adj. Flow (vph)	40	42	0	0	34	18	17	90	10	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	82	0	0	52	0	0	117	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.22	1.14	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: 0	BD											
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 25.9%			IC	CU Level o	of Service	А					
Analysis Period (min) 15												

Lanes, Volumes, Timings	
37: N Main St & W Washington St/E Washington St	t

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲	1			f)			é.	1			
Traffic Volume (vph)	17	240	0	0	291	37	11	26	15	0	0	0
Future Volume (vph)	17	240	0	0	291	37	11	26	15	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	65		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99				1.00			0.99	0.93			
Frt					0.985				0.850			
Flt Protected	0.950							0.986				
Satd. Flow (prot)	1562	1480	0	0	1454	0	0	1459	1258	0	0	0
Flt Permitted	0.387							0.986				
Satd. Flow (perm)	632	1480	0	0	1454	0	0	1451	1168	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					14				27			
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		254			262			326			306	
Travel Time (s)		5.8			6.0			7.4			7.0	
Confl. Peds. (#/hr)	8					8	9		23			
Confl. Bikes (#/hr)						1			1			
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)		0			0			0	0			
Adj. Flow (vph)	17	245	0	0	297	38	11	27	15	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	17	245	0	0	335	0	0	38	15	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.30	1.30	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2			2		1	2	1			
Detector Template	Left	Thru			Thru		Left	Thru	Right			
Leading Detector (ft)	20	100			100		20	100	20			
Trailing Detector (ft)	0	0			0		0	0	0			
Detector 1 Position(ft)	0	0			0		0	0	0			
Detector 1 Size(ft)	20	6			6		20	6	20			
Detector 1 Type	Cl+Ex	Cl+Ex			Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex			
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings	
37: N Main St & W Washington St/E Washington	St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA		Perm	NA	Perm			
Protected Phases		4			8			2				
Permitted Phases	4						2		2			
Detector Phase	4	4			8		2	2	2			
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0		5.0	5.0	5.0			
Minimum Split (s)	23.0	23.0			23.0		23.0	23.0	23.0			
Total Split (s)	51.0	51.0			51.0		29.0	29.0	29.0			
Total Split (%)	63.8%	63.8%			63.8%		36.3%	36.3%	36.3%			
Maximum Green (s)	46.0	46.0			46.0		24.0	24.0	24.0			
Yellow Time (s)	4.0	4.0			4.0		4.0	4.0	4.0			
All-Red Time (s)	1.0	1.0			1.0		1.0	1.0	1.0			
Lost Time Adjust (s)	0.0	0.0			0.0			0.0	0.0			
Total Lost Time (s)	5.0	5.0			5.0			5.0	5.0			
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0	3.0			
Recall Mode	None	None			None		C-Max	C-Max	C-Max			
Walk Time (s)	7.0	7.0			7.0		7.0	7.0	7.0			
Flash Dont Walk (s)	11.0	11.0			11.0		11.0	11.0	11.0			
Pedestrian Calls (#/hr)	0	0			0		0	0	0			
Act Effct Green (s)	24.8	24.8			24.8			45.2	45.2			
Actuated g/C Ratio	0.31	0.31			0.31			0.56	0.56			
v/c Ratio	0.09	0.53			0.73			0.05	0.02			
Control Delay	17.9	29.5			14.1			13.0	5.3			
Queue Delay	0.0	0.1			0.3			0.0	0.0			
Total Delay	17.9	29.6			14.5			13.0	5.3			
LOS	В	С			В			В	А			
Approach Delay		28.9			14.5			10.8				
Approach LOS		С			В			В				
90th %ile Green (s)	35.2	35.2			35.2		34.8	34.8	34.8			
90th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
70th %ile Green (s)	28.7	28.7			28.7		41.3	41.3	41.3			
70th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
50th %ile Green (s)	24.6	24.6			24.6		45.4	45.4	45.4			
50th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
30th %ile Green (s)	20.5	20.5			20.5		49.5	49.5	49.5			
30th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
10th %ile Green (s)	14.8	14.8			14.8		55.2	55.2	55.2			
10th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
Stops (vph)	8	141			261			21	4			
Fuel Used(gal)	0	3			3			0	0			
CO Emissions (g/hr)	10	188			214			22	5			
NOx Emissions (g/hr)	2	37			42			4	1			
VOC Emissions (g/hr)	2	44			50			5	1			
Dilemma Vehicles (#)	0	0			0			0	0			
Queue Length 50th (ft)	6	95			9			9	0			

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 37: N Main St & W Washington St/E Washington St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)	10	69			13			m29	m7			
Internal Link Dist (ft)		174			182			246			226	
Turn Bay Length (ft)	65											
Base Capacity (vph)	363	851			842			820	672			
Starvation Cap Reductn	0	112			154			0	0			
Spillback Cap Reductn	0	0			88			0	0			
Storage Cap Reductn	0	0			0			0	0			
Reduced v/c Ratio	0.05	0.33			0.49			0.05	0.02			
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 8 (10%), Reference	ed to phase 2	NBTL ar	id 6:, Stai	rt of Gree	n							
Natural Cycle: 50												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.73												
Intersection Signal Delay:	20.0			In	tersectior	n LOS: B						
Intersection Capacity Utiliz	ation 51.3%			IC	CU Level o	of Service	А					
Analysis Period (min) 15												
m Volume for 95th percentile queue is metered by upstream signal.												
Splits and Phases: 37. N	Solite and Phases: 37: N Main St & W Washington St/E Washington St											

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Ø2 (R)			
29 s		51s	
		← Ø8	
		51 s	

02/22/2024

Lanes, Volumes, Timings 38: W Front St/E Front St & N Main St

02/22/	2024
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲	ţ,			4			\$				
Traffic Volume (vph)	21	79	0	0	108	21	0	0	0	0	0	0
Future Volume (vph)	21	79	0	0	108	21	0	0	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	8	12	12	12	12
Storage Length (ft)	70		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.978							
Flt Protected	0.950											
Satd. Flow (prot)	1504	1425	0	0	1394	0	0	1308	0	0	0	0
Flt Permitted	0.950											
Satd. Flow (perm)	1504	1425	0	0	1394	0	0	1308	0	0	0	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		262			256			210			326	
Travel Time (s)		6.0			5.8			4.8			7.4	
Confl. Peds. (#/hr)	13		7	7		13	4		32			
Peak Hour Factor	0.82	0.82	0.92	0.92	0.82	0.82	0.92	0.92	0.92	0.82	0.92	0.82
Heavy Vehicles (%)	8%	8%	2%	2%	8%	8%	2%	2%	2%	8%	2%	8%
Parking (#/hr)		0			0			0				
Adj. Flow (vph)	26	96	0	0	132	26	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	26	96	0	0	158	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.55	1.14	1.14	1.14	1.14
Turning Speed (mph)	15		60	60		9	60		60	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type: C	BD											
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 35.9%			IC	U Level	of Service	А					
Analysis Period (min) 15												

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		1	≜ ↑₽			
Traffic Volume (vph)	0	26	1108	6	0	0
Future Volume (vph)	0	26	1108	6	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	11	11	12	12
Grade (%)	0%		-2%			2%
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Frt		0.865	0.999			
Flt Protected						
Satd. Flow (prot)	0	1305	3107	0	0	0
Flt Permitted						
Satd. Flow (perm)	0	1305	3107	0	0	0
Link Speed (mph)	30		30			30
Link Distance (ft)	558		266			127
Travel Time (s)	12.7		6.0			2.9
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Parking (#/hr)		0				
Adj. Flow (vph)	0	28	1179	6	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	28	1185	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		0			0
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.14	1.30	1.18	1.18	1.16	1.16
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Stop
Intersection Summary						
Area Type:	CBD					
Control Type: Unsignalized						
Intersection Capacity Utilizat	ion 44.2%			IC	U Level o	of Service
Analysis Period (min) 15						

Lanes, Volumes, Timings 47: S Center St & W Olive St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1	1	7	•						4 ₽	1
Traffic Volume (vph)	0	81	43	9	29	0	0	0	0	48	1037	46
Future Volume (vph)	0	81	43	9	29	0	0	0	0	48	1037	46
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	11	10	12	12	12	12	12	12	12	12	12
Grade (%)		1%			-3%			3%			-1%	
Storage Length (ft)	0		80	0		0	0		0	0		50
Storage Lanes	0		1	1		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00
Frt			0.850									0.850
Flt Protected				0.950							0.998	
Satd. Flow (prot)	0	1792	1470	1796	1891	0	0	0	0	0	3550	1591
Flt Permitted				0.700							0.998	
Satd. Flow (perm)	0	1792	1470	1323	1891	0	0	0	0	0	3550	1591
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			47									34
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		777			330			770			660	
Travel Time (s)		17.7			7.5			17.5			15.0	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	88	47	10	32	0	0	0	0	52	1127	50
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	88	47	10	32	0	0	0	0	0	1179	50
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12	Ū		12	Ū		0	Ŭ		0	•
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.01	1.05	1.10	0.98	0.98	0.98	1.02	1.02	1.02	0.99	0.99	0.99
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors		2	1	1	2					1	2	1
Detector Template		Thru	Right	Left	Thru					Left	Thru	Right
Leading Detector (ft)		100	20	20	100					20	100	20
Trailing Detector (ft)		0	0	0	0					0	0	0
Detector 1 Position(ft)		0	0	0	0					0	0	0
Detector 1 Size(ft)		6	20	20	6					20	6	20
Detector 1 Type		CI+Ex	Cl+Ex	Cl+Ex	CI+Ex					Cl+Ex	CI+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 1 Queue (s)		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 1 Delay (s)		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		Cl+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA	Perm	Perm	NA					Perm	NA	Perm

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Lanes, Volumes, Timings 47: S Center St & W Olive St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases		4			8						6	
Permitted Phases			4	8						6		6
Detector Phase		4	4	8	8					6	6	6
Switch Phase												
Minimum Initial (s)		5.0	5.0	5.0	5.0					5.0	5.0	5.0
Minimum Split (s)		13.5	13.5	13.5	13.5					37.9	37.9	37.9
Total Split (s)		21.0	21.0	21.0	21.0					59.0	59.0	59.0
Total Split (%)		26.3%	26.3%	26.3%	26.3%					73.8%	73.8%	73.8%
Maximum Green (s)		15.5	15.5	15.5	15.5					53.1	53.1	53.1
Yellow Time (s)		3.2	3.2	3.2	3.2					3.2	3.2	3.2
All-Red Time (s)		2.3	2.3	2.3	2.3					2.7	2.7	2.7
Lost Time Adjust (s)		0.0	0.0	0.0	0.0						0.0	0.0
Total Lost Time (s)		5.5	5.5	5.5	5.5						5.9	5.9
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0	3.0	3.0	3.0					3.0	3.0	3.0
Recall Mode		None	None	None	None					C-Max	C-Max	C-Max
Walk Time (s)		1.0	1.0	1.0	1.0					21.0	21.0	21.0
Flash Dont Walk (s)		7.0	7.0	7.0	7.0					11.0	11.0	11.0
Pedestrian Calls (#/hr)		0	0	0	0					0	0	0
Act Effct Green (s)		9.3	9.3	9.2	9.2						62.8	62.8
Actuated g/C Ratio		0.12	0.12	0.12	0.12						0.78	0.78
v/c Ratio		0.43	0.22	0.07	0.15						0.42	0.04
Control Delay		38.5	12.4	36.8	36.9						2.1	0.4
Queue Delay		0.0	0.0	0.0	0.0						0.0	0.0
Total Delay		38.5	12.4	36.8	36.9						2.1	0.4
LOS		D	В	D	D						А	A
Approach Delay		29.4			36.9						2.1	
Approach LOS		С			D						А	
90th %ile Green (s)		12.7	12.7	12.7	12.7					55.9	55.9	55.9
90th %ile Term Code		Gap	Gap	Hold	Hold					Coord	Coord	Coord
70th %ile Green (s)		10.7	10.7	10.7	10.7					57.9	57.9	57.9
70th %ile Term Code		Gap	Gap	Hold	Hold					Coord	Coord	Coord
50th %ile Green (s)		9.3	9.3	9.3	9.3					59.3	59.3	59.3
50th %ile Term Code		Gap	Gap	Hold	Hold					Coord	Coord	Coord
30th %ile Green (s)		7.8	7.8	7.8	7.8					60.8	60.8	60.8
30th %ile Term Code		Gap	Gap	Hold	Hold					Coord	Coord	Coord
10th %ile Green (s)		0.0	0.0	0.0	0.0					74.1	74.1	74.1
10th %ile Term Code		Skip	Skip	Skip	Skip					Coord	Coord	Coord
Stops (vph)		73	13	12	30						229	0
Fuel Used(gal)		2	0	0	0						7	0
CO Emissions (g/hr)		107	31	11	32						512	17
NOx Emissions (g/hr)		21	6	2	6						100	3
VOC Emissions (g/hr)		25	7	3	7						119	4
Dilemma Vehicles (#)		0	0	0	0						0	0
Queue Length 50th (ft)		42	0	5	16						24	1
Queue Length 95th (ft)		82	28	m18	m42						38	m1
Internal Link Dist (ft)		697			250			690			580	= 6
Turn Bay Length (ft)			80									50

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Lanes, Volumes, Timings 47: S Center St & W Olive St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Base Capacity (vph)		347	322	256	366						2786	1255
Starvation Cap Reductn		0	0	0	0						0	0
Spillback Cap Reductn		0	0	0	0						0	0
Storage Cap Reductn		0	0	0	0						0	0
Reduced v/c Ratio		0.25	0.15	0.04	0.09						0.42	0.04
Intersection Summary												
Area Type:	Other											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 25 (31%), Reference	ed to phase	2: and 6:	SBTL, Sta	art of Gre	en							
Natural Cycle: 55												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.43												
Intersection Signal Delay:	5.7			In	tersectior	n LOS: A						
Intersection Capacity Utiliz	ation 52.5%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
m Volume for 95th perce	ntile queue i	s metered	l by upstr	eam sign	al.							
Splits and Phases: 47: S	Center St 8	W Olive	St									
5%								100	1020			25



Lanes, Volumes, Timings 48: S East St & E Olive St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ŧ			≜ ↑			đ þ				
Traffic Volume (vph)	79	60	0	0	35	27	16	830	14	0	0	0
Future Volume (vph)	79	60	0	0	35	27	16	830	14	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	12	12	12	12	12	12	12	12	12
Grade (%)		2%			-4%			4%			-4%	
Storage Length (ft)	0		0	0		0	300		0	0		0
Storage Lanes	0		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00
Frt					0.935			0.998				
Flt Protected		0.972						0.999				
Satd. Flow (prot)	0	1672	0	0	3279	0	0	3359	0	0	0	0
Flt Permitted		0.784						0.999				
Satd. Flow (perm)	0	1348	0	0	3279	0	0	3359	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					29			4				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		330			647			682			633	
Travel Time (s)		7.5			14.7			15.5			14.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%
Parking (#/hr)		0										
Adj. Flow (vph)	86	65	0	0	38	29	17	902	15	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	151	0	0	67	0	0	934	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.01	1.07	1.01	0.97	0.97	0.97	1.03	1.03	1.03	0.97	0.97	0.97
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors	1	2			2		1	2				
Detector Template	Left	Thru			Thru		Left	Thru				
Leading Detector (ft)	20	100			100		20	100				
Trailing Detector (ft)	0	0			0		0	0				
Detector 1 Position(ft)	0	0			0		0	0				
Detector 1 Size(ft)	20	6			6		20	6				
Detector 1 Type	CI+Ex	Cl+Ex			CI+Ex		CI+Ex	CI+Ex				
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0				
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex				
Detector 2 Channel												

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 48: S East St & E Olive St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA		Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4				-		2					
Detector Phase	4	4			8		2	2				
Switch Phase	•						_	_				
Minimum Initial (s)	50	50			50		50	50				
Minimum Split (s)	19.5	19.5			19.5		58.6	58.6				
Total Split (s)	21.0	21.0			21.0		59.0	59.0				
Total Split (%)	26.3%	26.3%			26.3%		73.8%	73.8%				
Maximum Green (s)	15.5	15.5			15.5		53.4	53.4				
Yellow Time (s)	32	3.2			3.2		3.2	3.2				
All-Red Time (s)	2.3	2.3			2.3		2.4	2.4				
Lost Time Adjust (s)	2.0	0.0			0.0		2.1	0.0				
Total Lost Time (s)		5.5			5.5			5.6				
Lead/Lag		0.0			0.0			0.0				
Lead-Lag Ontimize?												
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0				
Recall Mode	Max	Max			None		C-Max	C-Max				
Walk Time (s)	3.0	3.0			3.0		/2 0	12 0				
Flash Dont Walk (s)	11.0	11.0			11.0		42.0	42.0				
Pedestrian Calls (#/br)	0.11	0			0		0	0				
Act Effet Green (s)	0	15.5			15.5		0	53 /				
Actuated a/C Patio		0.10			0.10			0.67				
v/c Patio		0.19			0.19			0.07				
Control Dolov		25.0			17.9			6.8				
		0.0			17.0			0.0				
Queue Delay		25.0			17.0			0.0 6.9				
		55.9 D			17.0 D			0.0				
LUS Approach Dolov		25 O			17 0			A 6 9				
Approach LOS		JD.9			I/.0			0.0				
Approach LOS	1E E	15 5			15 5		E2 4	E2 4				
90th %ile Green (S)	10.0 MayD	ID.D			ID.D		00.4	DO.4				
30th %ile Croop (a)							C0010	C0010				
70th %ile Green (S)	ID.D	ID.D			0.CI		53.4 Caard	53.4 Coord				
70th %ile Term Code	MaxR	MaxR			HOIO							
50th %ile Green (S)	ID.D	ID.D			0.CI		53.4 Caard	53.4 Coord				
30th %ile Green (s)	15.5 Max D	15.5 Max D			15.5		53.4	53.4				
30th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
10th %ile Green (s)	15.5	15.5			15.5		53.4	53.4				
10th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
Stops (vph)		135			32			358				
Fuel Used(gal)		2			1			8				
CO Emissions (g/hr)		148			50			540				
NUX Emissions (g/hr)		29			10			105				
VUC Emissions (g/hr)		34			12			125				
Dilemma Vehicles (#)		0			0			0				
Queue Length 50th (ft)		80			8			96				
Queue Length 95th (ft)		138			24			130				

2024 PM Downtown Bloomington 3:35 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 48: S East St & E Olive St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		250			567			602			553	
Turn Bay Length (ft)												
Base Capacity (vph)		261			658			2243				
Starvation Cap Reductn		0			0			0				
Spillback Cap Reductn		0			0			0				
Storage Cap Reductn		0			0			0				
Reduced v/c Ratio		0.58			0.10			0.42				
Intersection Summary												
Area Type: 0	Other											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 52 (65%), Referenced	d to phase 2	2:NBTL a	ind 6:, Sta	art of Gre	en							
Natural Cycle: 80												
Control Type: Actuated-Coor	dinated											
Maximum v/c Ratio: 0.58												
Intersection Signal Delay: 11	.2			In	tersectior	n LOS: B						
Intersection Capacity Utilizat	ion 47.3%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
Splits and Phases: 48: S E	East St & E	Olive St										

Ø2 (R)	▲ _{Ø4}
59 s	21 s
	← Ø8
	21 s

02/22/2024

Lanes, Volumes, Timings 3: N Madison St/N Center St & W Locust St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		† 1 ₂			ŧ						41	1
Traffic Volume (vph)	0	459	57	0	0	0	0	0	0	164	1046	0
Future Volume (vph)	0	459	57	0	0	0	0	0	0	164	1046	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	0		0	0		150
Storage Lanes	0		0	0		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00
Ped Bike Factor		1.00									1.00	
Frt		0.983										
Flt Protected											0.993	
Satd. Flow (prot)	0	3407	0	0	1827	0	0	0	0	0	3447	1827
Flt Permitted											0.993	
Satd. Flow (perm)	0	3407	0	0	1827	0	0	0	0	0	3445	1827
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		19										
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		659			244			860			526	
Travel Time (s)		15.0			5.5			19.5			12.0	
Confl. Peds. (#/hr)			2							4		
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	560	70	0	0	0	0	0	0	200	1276	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	630	0	0	0	0	0	0	0	0	1476	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	1
Detector Template		Thru		Left	Thru					Left	Thru	Right
Leading Detector (ft)		100		20	100					20	100	20
Trailing Detector (ft)		0		0	0					0	0	0
Detector 1 Position(ft)		0		0	0					0	0	0
Detector 1 Size(ft)		6		20	6					20	6	20
Detector 1 Type		Cl+Ex		Cl+Ex	Cl+Ex					Cl+Ex	Cl+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		Cl+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 3: N Madison St/N Center St & W Locust St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type		NA								Perm	NA	Perm
Protected Phases		4			8						6	
Permitted Phases				8						6		6
Detector Phase		4		8	8					6	6	6
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	5.0
Minimum Split (s)		23.4		22.5	22.5					23.4	23.4	23.4
Total Split (s)		23.7		22.5	22.5					46.3	46.3	46.3
Total Split (%)		33.9%		32.1%	32.1%					66.1%	66.1%	66.1%
Maximum Green (s)		18.3		18.0	18.0					40.9	40.9	40.9
Yellow Time (s)		3.2		3.5	3.5					3.2	3.2	3.2
All-Red Time (s)		2.2		1.0	1.0					2.2	2.2	2.2
Lost Time Adjust (s)		0.0			0.0						0.0	0.0
Total Lost Time (s)		5.4			4.5						5.4	5.4
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	3.0
Recall Mode		None		None	None					C-Max	C-Max	C-Max
Walk Time (s)		7.0		7.0	7.0					7.0	7.0	7.0
Flash Dont Walk (s)		11.0		11.0	11.0					11.0	11.0	11.0
Pedestrian Calls (#/hr)		0		0	0					0	0	0
Act Effct Green (s)		16.9		-	-					-	42.3	-
Actuated g/C Ratio		0.24									0.60	
v/c Ratio		0.75									0.71	
Control Delay		30.0									12.4	
Queue Delav		52.8									0.0	
Total Delay		82.8									12.4	
LOS		F									В	
Approach Delay		82.8									12.4	
Approach LOS		F									В	
90th %ile Green (s)		18.3		19.2	19.2					40.9	40.9	40.9
90th %ile Term Code		Max		Hold	Hold					Coord	Coord	Coord
70th %ile Green (s)		18.3		19.2	19.2					40.9	40.9	40.9
70th %ile Term Code		Max		Hold	Hold					Coord	Coord	Coord
50th %ile Green (s)		18.3		19.2	19.2					40.9	40.9	40.9
50th %ile Term Code		Max		Hold	Hold					Coord	Coord	Coord
30th %ile Green (s)		16.6		17.5	17.5					42.6	42.6	42.6
30th %ile Term Code		Gap		Hold	Hold					Coord	Coord	Coord
10th %ile Green (s)		13.1		14.0	14.0					46.1	46.1	46.1
10th %ile Term Code		Gap		Hold	Hold					Coord	Coord	Coord
Stops (vph)		445									794	
Fuel Used(gal)		8									12	
CO Emissions (a/hr)		578									867	
NOx Emissions (g/hr)		112									169	
VOC Emissions (a/hr)		134									201	
Dilemma Vehicles (#)		0									0	
Queue Length 50th (ft)		124									218	
Queue Length 95th (ft)		158									247	
Internal Link Dist (ft)		579			164			780			446	

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 3: N Madison St/N Center St & W Locust St

3: N Madison St/N	Center	St & V	/ Locu	st St							02/2	2/2024
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)		904									2080	
Starvation Cap Reductn		0									0	
Spillback Cap Reductn		367									0	
Storage Cap Reductn		0									0	
Reduced v/c Ratio		1.17									0.71	
Intersection Summary												
Area Type:	Other											
Cycle Length: 70												
Actuated Cycle Length: 70												
Offset: 10 (14%), Reference	ed to phase	2: and 6:	SBTL, Sta	art of Gre	en							
Natural Cycle: 60												
Control Type: Actuated-Coo	ordinated											
Maximum v/c Ratio: 0.75												
Intersection Signal Delay: 3	3.4			In	tersectior	n LOS: C						
Intersection Capacity Utiliza	ation 57.2%			IC	U Level o	of Service	В					
Analysis Period (min) 15												

Splits and Phases: 3: N Madison St/N Center St & W Locust St



Lanes, Volumes, Timings 6: N East St/N Main St & W Locust St/E Locust St

02/22/2024

Lane Group EBL EBT EBR WBL WBT WBR NBT NBT NBR SBL SBT SBR Lane Configurations 44		۶	-	7	1	•	*	1	Ť	1	1	ŧ	~
Lane Configurations 4↑	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph) 92 528 0 0 0 0 331 103 0 0 0 Future Volume (vph) 92 528 0	Lane Configurations		≜ 12						41.				
Fiture (vph) 92 528 0 0 0 0 931 103 0 0 0 ideal Flow (vph) 1900 100 100 100	Traffic Volume (vph)	92	528	0	0	0	0	0	931	103	0	0	0
Ideal Flow (prip) 1900 <td>Future Volume (vph)</td> <td>92</td> <td>528</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>931</td> <td>103</td> <td>0</td> <td>0</td> <td>0</td>	Future Volume (vph)	92	528	0	0	0	0	0	931	103	0	0	0
Lane Ulti-Factor 0.95 0.95 1.00 1.00 1.00 1.00 0.95 0.95 0.95 0.95 1.00 1.00 Ped Bike Factor 1.00 0 0.993 0 0.955 0.955 0.95 0 0 0	Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ped Bike Factor 1.00 0.985 Frt 0.985 El Protected 0.993 0 Satd. Flow (prot) 0 3447 0 0 0 0 3419 0 <td>Lane Util. Factor</td> <td>0.95</td> <td>0.95</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>0.95</td> <td>0.95</td> <td>0.95</td> <td>1.00</td> <td>1.00</td> <td>1.00</td>	Lane Util. Factor	0.95	0.95	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Frt 0.993 0 0.995 Filt Protected 0.993 0 0 0.419 0 0 0 0 Satd. Flow (prot) 0 3447 0 0 0 0 3419 0 0 0 0 Satd. Flow (prot) 0 3446 0 0 0 0 3419 0 0 0 0 Satd. Flow (prot) 0 3446 0 0 0 0 3419 0 0 0 0 Satd. Flow (prot) 0 3446 0<	Ped Bike Factor		1.00										
Fit Protected 0.993 Satd. Flow (prot) 0 3447 0 0 0 0 3419 0 </td <td>Frt</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>0.985</td> <td></td> <td></td> <td></td> <td></td>	Frt								0.985				
Satid. Flow (prot) 0 3447 0 0 0 0 3419 0 0 0 0 FIt Permitted 0.993 0<	Flt Protected		0.993										
Fit Permitted 0.993 Satd. Flow (perm) 0.3446 0 0 0 3419 0	Satd. Flow (prot)	0	3447	0	0	0	0	0	3419	0	0	0	0
Satd. Flow (perm) 0 3446 0 0 0 0 3419 0 0 0 0 Right Tum on Red Yes	Flt Permitted		0.993										-
Right Turn on Red Yes	Satd, Flow (perm)	0	3446	0	0	0	0	0	3419	0	0	0	0
Satil. Flow (RTOR) 27 Link Speed (mph) 30 30 30 30 Link Distance (ft) 244 514 878 525 Travel Time (s) 5.5 11.7 20.0 11.9 Confl. Peds. (#hr) 2 2 2 2 2 Paek Hour Factor 0.87	Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph) 30 30 30 30 30 Link Distance (tt) 244 514 878 525 Travel Time (s) 5.5 11.7 20.0 11.9 Confl. Peds. (#hr) 2	Satd, Flow (RTOR)								27				
Link Distance (t) 244 514 878 525 Travel Time (s) 5.5 11.7 20.0 11.9 Confl. Peds. (#hr) 2 Peak Hour Factor 0.87 <td>Link Speed (mph)</td> <td></td> <td>30</td> <td></td> <td></td> <td>30</td> <td></td> <td></td> <td>30</td> <td></td> <td></td> <td>30</td> <td></td>	Link Speed (mph)		30			30			30			30	
Travel Time (s) 5.5 11.7 20.0 11.9 Confl. Peds. (#hr) 2	Link Distance (ft)		244			514			878			525	
Interview Interview <t< td=""><td>Travel Time (s)</td><td></td><td>5.5</td><td></td><td></td><td>11 7</td><td></td><td></td><td>20.0</td><td></td><td></td><td>11.9</td><td></td></t<>	Travel Time (s)		5.5			11 7			20.0			11.9	
Deak Hour Factor 0.87	Confl Peds (#/hr)	2	0.0						20.0				
Deterviol Date	Peak Hour Factor	0.87	0 87	0.87	0 87	0.87	0 87	0.87	0.87	0 87	0.87	0 87	0 87
Adj. Flow (vph) 106 607 0 0 0 0 107 118 0 0 0 0 Lane Group Flow (vph) 0 713 0 0 0 0 0 1188 0 0 0 0 Lane Group Flow (vph) 0 713 0 0 0 0 1188 0 0 0 0 Enter Blocked Intersection No	Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Name (Line) 100	Adi Flow (vph)	106	607	0	0	0	0	0	1070	118	0	0	0
Construction Construction 0 <td>Shared Lane Traffic (%)</td> <td>100</td> <td>001</td> <td>v</td> <td>Ŭ</td> <td>Ū</td> <td>Ū</td> <td>v</td> <td>1010</td> <td>110</td> <td>v</td> <td>Ŭ</td> <td>v</td>	Shared Lane Traffic (%)	100	001	v	Ŭ	Ū	Ū	v	1010	110	v	Ŭ	v
Lans olsop Flore Disopression Disopresi	Lane Group Flow (vph)	0	713	0	0	0	0	0	1188	0	0	0	0
Link Diotection Ho	Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Low low low low low low low low low low l	Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Inix Offset(ft) 0 0 0 0 0 Crosswalk Width(ft) 16 16 16 16 16 Two way Left Turn Lane	Median Width(ft)	2011	0	rugitt	2011	0	rugitt	2011	0	i dgirt	Lon	0	i agin
Line Out(y) C C C C C Crosswalk Width(ft) 16 16 16 16 Two way Left Turn Lane 1 1 1 16 16 16 Headway Factor 1.00 </td <td>Link Offset(ft)</td> <td></td> <td>0</td> <td></td> <td></td> <td>0</td> <td></td> <td></td> <td>0</td> <td></td> <td></td> <td>0</td> <td></td>	Link Offset(ft)		0			0			0			0	
Two way Left Turn Lane To hto< th=""> To <t< td=""><td>Crosswalk Width(ft)</td><td></td><td>16</td><td></td><td></td><td>16</td><td></td><td></td><td>16</td><td></td><td></td><td>16</td><td></td></t<></thto<>	Crosswalk Width(ft)		16			16			16			16	
Headway Factor 1.00	Two way Left Turn Lane												
Turning Speed (mph) 15 9 15 16 16 17 10 <th10< th=""> 10 <th10< th=""></th10<></th10<>	Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Number of Detectors 1 2 1 2 Detector Template Left Thru Left Thru Leading Detector (ft) 20 100 20 100 Trailing Detector (ft) 0 0 0 0 Detector 1 Position(ft) 0 0 0 0 Detector 1 Size(ft) 20 6 20 6 Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 1 Channel	Turning Speed (mph)	15		9	15		9	15		9	15		9
Detector Template Left Thru Left Thru Leading Detector (ft) 20 100 20 100 Trailing Detector (ft) 0 0 0 0 Detector 1 Position(ft) 0 0 0 0 Detector 1 Size(ft) 20 6 20 6 Detector 1 Channel	Number of Detectors	1	2	· ·			, in the second s	1	2	•			·
Leading Detector (ft) 20 100 20 100 Trailing Detector (ft) 0 0 0 0 0 Detector 1 Position(ft) 0 0 0 0 0 0 Detector 1 Position(ft) 0	Detector Template	Left	Thru					Left	Thru				
Trailing Detector (ft) 0 0 0 Detector 1 Position(ft) 0 0 0 Detector 1 Size(ft) 20 6 20 6 Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 1 Channel	Leading Detector (ft)	20	100					20	100				
Detector 1 Position(ft) 0 0 0 0 Detector 1 Size(ft) 20 6 20 6 Detector 1 Size(ft) 20 6 20 6 Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 1 Channel	Trailing Detector (ft)	0	0					0	0				
Detector 1 Size(ft) 20 6 20 6 Detector 1 Type CI+Ex CI+Ex CI+Ex Detector 1 Channel Detector 1 Extend (s) 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0	Detector 1 Position(ft)	0	0					0	0				
Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 1 Channel	Detector 1 Size(ft)	20	6					20	6				
Detector 1 Channel 0.0 0.0 0.0 0.0 Detector 1 Extend (s) 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 94 Detector 2 Size(ft) 6 6 6 Detector 2 Type CI+Ex CI+Ex 0.0 Detector 2 Channel 0.0 0.0 0.0 Detector 2 Extend (s) 0.0 0.0 0.0 Turn Type Perm NA NA Protected Phases 4 2 2	Detector 1 Type	CI+Ex	CI+Ex					Cl+Ex	CI+Ex				
Detector 1 Extend (s) 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 Detector 2 Size(ft) 6 6 Detector 2 Type CI+Ex CI+Ex Detector 2 Channel 0.0 0.0 Detector 2 Extend (s) 0.0 0.0 Turn Type Perm NA Protected Phases 4 2	Detector 1 Channel	••• ••	•. =					0/	• =				
Detector 1 Queue (s) 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 94 Detector 2 Size(ft) 6 6 6 Detector 2 Type CI+Ex CI+Ex Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 Turn Type Perm NA NA Protected Phases 4 2	Detector 1 Extend (s)	0.0	0.0					0.0	0.0				
Detector 1 Delay (s) 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 94 Detector 2 Size(ft) 6 6 6 Detector 2 Type CI+Ex CI+Ex 0.0 Detector 2 Channel 0.0 0.0 0.0 Detector 2 Extend (s) 0.0 0.0 0.0 Turn Type Perm NA NA Protected Phases 4 2 2	Detector 1 Queue (s)	0.0	0.0					0.0	0.0				
Detector 2 Position(ft)9494Detector 2 Size(ft)66Detector 2 TypeCI+ExDetector 2 Channel0.0Detector 2 Extend (s)0.0Turn TypePermProtected Phases42	Detector 1 Delay (s)	0.0	0.0					0.0	0.0				
Detector 2 Size(ft) 6 6 Detector 2 Type CI+Ex CI+Ex Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 Turn Type Perm NA NA Protected Phases 4 2 Permitted Phases 4 2	Detector 2 Position(ft)	0.0	94					0.0	94				
Detector 2 Type CI+Ex Detector 2 Channel Detector 2 Extend (s) 0.0 Turn Type Perm NA Protected Phases 4 2	Detector 2 Size(ft)		6						6				
Detector 2 Channel 0.0 0.0 Detector 2 Extend (s) 0.0 0.0 Turn Type Perm NA Protected Phases 4 2 Permitted Phases 4 2	Detector 2 Type		CI+Ex						CI+Ex				
Detector 2 Extend (s)0.00.0Turn TypePermNANAProtected Phases42Permitted Phases42	Detector 2 Channel								. . .				
Turn Type Perm NA NA Protected Phases 4 2 Permitted Phases 4 2	Detector 2 Extend (s)		0.0						0.0				
Protected Phases 4 2 Permitted Phases 4 2	Turn Type	Perm	NA						NA				
Permitted Phases 4 2	Protected Phases		4						2				
-	Permitted Phases	4						2	_				

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings
6: N East St/N Main St & W Locust St/E Locust St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4					2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0					5.0	5.0				
Minimum Split (s)	23.5	23.5					23.5	23.5				
Total Split (s)	26.0	26.0					44.0	44.0				
Total Split (%)	37.1%	37.1%					62.9%	62.9%				
Maximum Green (s)	20.5	20.5					38.5	38.5				
Yellow Time (s)	3.2	3.2					3.2	3.2				
All-Red Time (s)	2.3	2.3					2.3	2.3				
Lost Time Adjust (s)		0.0						0.0				
Total Lost Time (s)		5.5						5.5				
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0				
Recall Mode	None	None					C-Max	C-Max				
Walk Time (s)	7.0	7.0					7.0	7.0				
Flash Dont Walk (s)	11.0	11.0					11.0	11.0				
Pedestrian Calls (#/hr)	0	0					0	0				
Act Effct Green (s)		18.9						40.1				
Actuated g/C Ratio		0.27						0.57				
v/c Ratio		0.77						0.60				
Control Delay		19.3						7.7				
Queue Delay		46.9						0.0				
Total Delay		66.3						7.7				
LOS		Е						А				
Approach Delay		66.3						7.7				
Approach LOS		Е						А				
90th %ile Green (s)	20.5	20.5					38.5	38.5				
90th %ile Term Code	Max	Max					Coord	Coord				
70th %ile Green (s)	20.5	20.5					38.5	38.5				
70th %ile Term Code	Max	Max					Coord	Coord				
50th %ile Green (s)	20.5	20.5					38.5	38.5				
50th %ile Term Code	Max	Max					Coord	Coord				
30th %ile Green (s)	18.3	18.3					40.7	40.7				
30th %ile Term Code	Gap	Gap					Coord	Coord				
10th %ile Green (s)	14.7	14.7					44.3	44.3				
10th %ile Term Code	Gap	Gap					Coord	Coord				
Stops (vph)		584						480				
Fuel Used(gal)		7						11				
CO Emissions (g/hr)		479						793				
NOx Emissions (g/hr)		93						154				
VOC Emissions (g/hr)		111						184				
Dilemma Vehicles (#)		0						0				
Queue Length 50th (ft)		167						57				
Queue Length 95th (ft)		217						67				
Internal Link Dist (ft)		164			434			798			445	
Turn Bay Length (ft)												
Base Capacity (vph)		1009						1970				
Starvation Cap Reductn		354						0				

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings
6: N East St/N Main St & W Locust St/E Locust St

02/22/2024	
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Spillback Cap Reductn		0						0				
Storage Cap Reductn		0						0				
Reduced v/c Ratio		1.09						0.60				
Intersection Summary												
Area Type:	Other											
Cycle Length: 70												
Actuated Cycle Length: 70)											
Offset: 23 (33%), Reference	ced to phase 2	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 55												
Control Type: Actuated-Co	oordinated											
Maximum v/c Ratio: 0.77												
Intersection Signal Delay:	29.6			In	tersectior	LOS: C						
Intersection Capacity Utiliz	zation 55.4%			IC	U Level o	of Service	В					
Analysis Period (min) 15												
Solits and Phases: 6: N	East St/N Ma	in St & V	/ Locust \$	St/E Locu	ist St							

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Lanes, Volumes, Timings 9: N Madison St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,		7	1						đ þ	
Traffic Volume (vph)	0	75	80	20	80	0	0	0	0	10	780	105
Future Volume (vph)	0	75	80	20	80	0	0	0	0	10	780	105
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	11	12	12	11	12	12	12	12	12	12	12
Grade (%)		1%			-2%			-6%			4%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		0.99		1.00								
Frt		0.930									0.982	
Flt Protected				0.950							0.999	
Satd. Flow (prot)	0	1456	0	1578	1605	0	0	0	0	0	3003	0
Flt Permitted				0.508							0.999	
Satd. Flow (perm)	0	1456	0	840	1605	0	0	0	0	0	3003	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		72									38	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		705			247			400			860	
Travel Time (s)		16.0			5.6			9.1			19.5	
Confl. Peds. (#/hr)			3	3								
Confl. Bikes (#/hr)			1									
Peak Hour Factor	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	101	108	27	108	0	0	0	0	14	1054	142
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	209	0	27	108	0	0	0	0	0	1210	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.15	1.20	1.15	1.13	1.18	1.13	1.10	1.10	1.10	1.17	1.17	1.17
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		Cl+Ex	Cl+Ex					Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		Cl+Ex			Cl+Ex						Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 9: N Madison St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		15.4		15.4	15.4					47.2	47.2	
Total Split (s)		22.0		22.0	22.0					48.0	48.0	
Total Split (%)		31.4%		31.4%	31.4%					68.6%	68.6%	
Maximum Green (s)		16.6		16.6	16.6					42.8	42.8	
Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)		2.2		2.2	2.2					2.0	2.0	
Lost Time Adjust (s)		0.0		0.0	0.0						0.0	
Total Lost Time (s)		5.4		5.4	5.4						5.2	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		None	None					C-Max	C-Max	
Walk Time (s)		1.0		1.0	1.0					31.0	31.0	
Flash Dont Walk (s)		9.0		9.0	9.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		11.8		11.8	11.8					· ·	47.6	
Actuated g/C Ratio		0.17		0.17	0.17						0.68	
v/c Ratio		0.69		0.19	0.40						0.59	
Control Delay		28.8		26.1	29.0						1.4	
Queue Delav		0.0		0.0	0.0						0.0	
Total Delay		28.8		26.1	29.0						1.4	
LOS		С		С	С						А	
Approach Delay		28.8			28.4						1.4	
Approach LOS		С			С						А	
90th %ile Green (s)		16.6		16.6	16.6					42.8	42.8	
90th %ile Term Code		Max		Hold	Hold					Coord	Coord	
70th %ile Green (s)		14.8		14.8	14.8					44.6	44.6	
70th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
50th %ile Green (s)		12.1		12.1	12.1					47.3	47.3	
50th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
30th %ile Green (s)		9.5		9.5	9.5					49.9	49.9	
30th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
10th %ile Green (s)		6.0		6.0	6.0					53.4	53.4	
10th %ile Term Code		Hold		Hold	Hold					Coord	Coord	
Stops (vph)		93		18	67						38	
Fuel Used(gal)		2		0	1						6	
CO Emissions (g/hr)		159		17	70						452	
NOx Emissions (g/hr)		31		3	14						88	
VOC Emissions (g/hr)		37		4	16						105	
Dilemma Vehicles (#)		0		0	0						0	
Queue Length 50th (ft)		54		10	42						12	
Queue Length 95th (ft)		82		23	63						10	
Internal Link Dist (ft)		625			167			320			780	

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 9: N Madison St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)		400		199	380						2054	
Starvation Cap Reductn		0		0	0						0	
Spillback Cap Reductn		0		0	0						0	
Storage Cap Reductn		0		0	0						0	
Reduced v/c Ratio		0.52		0.14	0.28						0.59	
Intersection Summary												
Area Type: Cl	3D											
Cycle Length: 70												
Actuated Cycle Length: 70												
Offset: 28 (40%), Referenced	to phase 2	2: and 6:	SBTL, Sta	art of Gre	en							
Natural Cycle: 65												
Control Type: Actuated-Coord	inated											
Maximum v/c Ratio: 0.69												
Intersection Signal Delay: 7.4				In	tersectior	n LOS: A						
Intersection Capacity Utilization	n 55.3%			IC	U Level o	of Service	В					
Analysis Period (min) 15												
Splits and Phases: 9: N Ma	dison St 8	W Mark	et St									

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	3			A ∿		
Traffic Volume (vph)	50	0	100	1010	0	0
Future Volume (vph)	50	0	100	1010	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	13	12	12	11	12	12
Grade (%)	0%	12	12	-2%	2%	12
Lano I til Easter	1 00	1 00	0.05	-2 /0	2 /0	1 00
Pad Rika Factor	1.00	1.00	0.90	1.00	1.00	1.00
Ert				1.00		
FIL FIL Drotootod	0.050			0.006		
Fil Protected	0.950	0	0	0.990	0	0
Satd. Flow (prot)	1467	0	0	2/61	0	0
Fit Permitted	0.950	_		0.996		
Satd. Flow (perm)	1467	0	0	2760	0	0
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)						
Link Speed (mph)	30			30	30	
Link Distance (ft)	263			127	878	
Travel Time (s)	6.0			2.9	20.0	
Confl. Peds. (#/hr)			1			
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%
Parking (#/hr)	0	070	20	20	0,0	070
Adi Flow (vnh)	61	0	122	1232	0	0
Shared Lane Traffic (%)	UI	U	122	1202	U	U
Lane Group Flow (uph)	61	0	0	125/	0	0
Enter Blocked Intersection	U I	U No	U No	1304 No	V	U No
Long Alignment	INU Latt	Dia ht	INU Lett	INU Left	INU Latt	Dialet
Lane Alignment	Lett	Right	Lett	Lett	Len	Right
iviedian width(ft)	13			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.25	1.14	1.13	1.34	1.16	1.16
Turning Speed (mph)	15	9	15			9
Number of Detectors	1		1	2		
Detector Template	Left		Left	Thru		
Leading Detector (ft)	20		20	100		
Trailing Detector (ft)	0		0	0		
Detector 1 Position(ft)	0		0	0		
Detector 1 Size(ft)	20		20	6		
Detector 1 Type						
Detector 1 Channel			UI+EX			
Detector I Unannel				<u> </u>		
Detector 1 Extend (s)	0.0		0.0	0.0		
Detector 1 Queue (s)	0.0		0.0	0.0		
Detector 1 Delay (s)	0.0		0.0	0.0		
Detector 2 Position(ft)				94		
Detector 2 Size(ft)				6		
Detector 2 Type				CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)				0.0		

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Turn Type	Prot		Perm	NA			
Protected Phases	4			2			
Permitted Phases	-		2				
Detector Phase	4		2	2			
Switch Phase				_			
Minimum Initial (s)	5.0		5.0	5.0			
Minimum Split (s)	13.4		49.5	49.5			
Total Split (s)	16.0		54.0	54.0			
Total Split (%)	22.9%		77.1%	77.1%			
Maximum Green (s)	10.6		48.5	48.5			
Yellow Time (s)	3.2		3.2	3.2			
All-Red Time (s)	2.2		2.3	2.3			
Lost Time Adjust (s)	0.0			0.0			
Total Lost Time (s)	5.4			5.5			
Lead/Lag	0.11			0.0			
Lead-Lag							
Vehicle Extension (s)	30		30	30			
Recall Mode	None		C-Max	C-Max			
Walk Time (s)	1.0		33.0	33.0			
Flash Dont Walk (s)	7.0		11.0	11.0			
Pedestrian Calls (#/hr)	0		0	0			
Act Effct Green (s)	82		Ū	57.8			
Actuated g/C Ratio	0.12			0.83			
v/c Ratio	0.36			0.59			
Control Delay	33.8			5.7			
Queue Delay	0.0			0.0			
Total Delay	33.8			5.7			
LOS	C			A			
Approach Delay	33.8			5.7			
Approach LOS	C			A			
90th %ile Green (s)	10.6		48.5	48.5			
90th %ile Term Code	Max		Coord	Coord			
70th %ile Green (s)	9.6		49.5	49.5			
70th %ile Term Code	Gap		Coord	Coord			
50th %ile Green (s)	8.2		50.9	50.9			
50th %ile Term Code	Gap		Coord	Coord			
30th %ile Green (s)	0.0		64.5	64.5			
30th %ile Term Code	Skip		Coord	Coord			
10th %ile Green (s)	0.0		64.5	64.5			
10th %ile Term Code	Skip		Coord	Coord			
Stops (vph)	47			425			
Fuel Used(gal)	1			5			
CO Emissions (a/hr)	49			332			
NOx Emissions (a/hr)	10			65			
VOC Emissions (g/hr)	11			77			
Dilemma Vehicles (#)	0			0			
Queue Length 50th (ff)	25			127			
Queue Length 95th (ff)	51			177			
Internal Link Dist (ft)	183			47	798		

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Turn Bay Length (ft)							
Base Capacity (vph)	222			2278			
Starvation Cap Reductn	0			0			
Spillback Cap Reductn	0			0			
Storage Cap Reductn	0			0			
Reduced v/c Ratio	0.27			0.59			
Intersection Summary							
Area Type:	CBD						
Cycle Length: 70							
Actuated Cycle Length: 70)						
Offset: 4 (6%), Referenced	d to phase 2:N	IBTL and	6:, Start	of Green			
Natural Cycle: 65							
Control Type: Actuated-Co	oordinated						
Maximum v/c Ratio: 0.59							
Intersection Signal Delay:	6.9			Int	ersection	LOS: A	
Intersection Capacity Utiliz	ation 47.5%			ICI	J Level o	f Service A	
Analysis Period (min) 15							

Splits and Phases: 11: N East St & E Market St

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Lanes, Volumes, Timings 12: N Madison St & W Monroe St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,			र्स						đ þ	
Traffic Volume (vph)	0	10	0	7	4	0	0	0	0	19	904	10
Future Volume (vph)	0	10	0	7	4	0	0	0	0	19	904	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	11	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			-3%			6%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt											0.998	
Flt Protected					0.969						0.999	
Satd. Flow (prot)	0	1605	0	0	1609	0	0	0	0	0	3051	0
Flt Permitted					0.969						0.999	
Satd. Flow (perm)	0	1605	0	0	1609	0	0	0	0	0	3051	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		699			209			307			400	
Travel Time (s)		15.9			4.8			7.0			9.1	
Confl. Peds. (#/hr)			3	3						4		1
Confl. Bikes (#/hr)			1									
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	0	13	0	9	5	0	0	0	0	25	1174	13
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	13	0	0	14	0	0	0	0	0	1212	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.19	1.14	1.14	1.14	1.14	1.12	1.12	1.12	1.19	1.19	1.19
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Free	
Intersection Summary												
Area Type: C	BD											
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 42.5%			IC	CU Level	of Service	A					
Analysis Period (min) 15												

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	3			≜ ta		
Traffic Volume (voh)	15	0	45	1020	0	0
Future Volume (vph)	15	0	45	1020	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	11	12	12
Grade (%)	-2%	12	12	_3%	/%	12
	- <u>2</u> /0	1 00	0.05	-5 /6	4 /0	1 00
Pad Rike Factor	1.00	1.00	0.90	1.00	1.00	1.00
Feu DIKE Facilli Ert				1.00		
FIL Fit Droto stod	0.050			0 000		
Fil Protected	0.950	0	0	0.990	0	0
Sato. Flow (prot)	1434	0	0	2780	0	0
Fit Permitted	0.950	_		0.998		
Satd. Flow (perm)	1434	0	0	2780	0	0
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)						
Link Speed (mph)	30			30	30	
Link Distance (ft)	221			318	266	
Travel Time (s)	5.0			7.2	6.0	
Confl. Peds. (#/hr)			1			
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%
Parking (#/hr)	0.0	070	20	20	570	0 /0
Adi Flow (vnh)	18	0	55	1244	Ο	0
Shared Lane Traffic (%)	10	U	55	1244	U	U
Lano Group Flow (upb)	10	0	0	1200	0	0
Larie Group Flow (Vpri)	IÖ	U	U	1299	U	U
Enter Blocked Intersection	N0	NO	NO	NO	NO	NO Di Lu
	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.29	1.13	1.12	1.34	1.17	1.17
Turning Speed (mph)	15	9	15			9
Number of Detectors	1		1	2		
Detector Template	Left		Left	Thru		
Leading Detector (ft)	20		20	100		
Trailing Detector (ft)	0		0	0		
Detector 1 Position(ft)	0		0	0		
Detector 1 Size(#)	20		20	6		
Detector 1 Size(II)						
Detector 1 Type	CI+EX		CI+EX	UI+EX		
Detector 1 Channel	~ ~		~ ~	~ ~		
Detector 1 Extend (s)	0.0		0.0	0.0		
Detector 1 Queue (s)	0.0		0.0	0.0		
Detector 1 Delay (s)	0.0		0.0	0.0		
Detector 2 Position(ft)				94		
Detector 2 Size(ft)				6		
Detector 2 Type				CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)				0.0		

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Turn Type	Prot		Perm	NA			
Protected Phases	4			2			
Permitted Phases			2	_			
Detector Phase	4		2	2			
Switch Phase				_			
Minimum Initial (s)	50		50	50			
Minimum Split (s)	13.2		49.2	49.2			
Total Split (s)	15.0		65.0	65.0			
Total Split (%)	18.8%		81.3%	81.3%			
Maximum Green (s)	9.8		59.8	59.8			
Yellow Time (s)	3.2		3.2	3.2			
All-Red Time (s)	2.0		2.0	2.0			
Lost Time Adjust (s)	0.0		2.0	0.0			
Total Lost Time (s)	5.0			5.2			
Lead/Lag	0.2			0.2			
Lead-Lag Ontimize?							
Vehicle Extension (s)	3.0		3.0	3.0			
Recall Mode	None		C-Max	C-Max			
Walk Time (s)	10		33.0	33.0			
Flash Dont Walk (s)	7.0		11.0	11.0			
Pedestrian Calls (#/hr)	1.0		0	0			
Act Effet Green (s)	66		U	76.2			
Actuated a/C Ratio	0.0			0.95			
v/c Ratio	0.00			0.00			
Control Delay	36.4			0.45			
Queue Delay	0.0			0.0			
Total Delay	36.4			0.0			
LOS	00.1 D			0.0 A			
Approach Delay	36.4			0.6			
Approach LOS	00.1 D			0.0 A			
90th %ile Green (s)	84		61.2	61.2			
90th %ile Term Code	Gan		Coord	Coord			
70th %ile Green (s)	0.0		74.8	74.8			
70th %ile Term Code	Skin		Coord	Coord			
50th %ile Green (s)	0.0		74.8	74.8			
50th %ile Term Code	Skin		Coord	Coord			
30th %ile Green (s)	0.0		74.8	74.8			
30th %ile Term Code	Skin		Coord	Coord			
10th %ile Green (s)	0.0		74.8	74.8			
10th %ile Term Code	Skin		Coord	Coord			
Stops (vph)	17		COOR	5			
Evel Used(gal)	0			3			
CO Emissions (a/hr)	16			195			
NOx Emissions (g/hr)	2			38			
VOC Emissions (g/hr)	4			45			
Dilemma Vehicles (#)							
Oueue I enath 50th (ft)	Q			0			
	25			2			
Internal Link Dist (ff)	141			238	186		

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Turn Bay Length (ft)							
Base Capacity (vph)	175			2649			
Starvation Cap Reductn	0			9			
Spillback Cap Reductn	0			0			
Storage Cap Reductn	0			0			
Reduced v/c Ratio	0.10			0.49			
Intersection Summary							
Area Type:	CBD						
Cycle Length: 80							
Actuated Cycle Length: 80)						
Offset: 61 (76%), Reference	ced to phase 2	2:NBTL a	nd 6:, Sta	art of Gree	en		
Natural Cycle: 65							
Control Type: Actuated-Co	oordinated						
Maximum v/c Ratio: 0.49							
Intersection Signal Delay:	1.1			In	tersection	LOS: A	
Intersection Capacity Utiliz	zation 45.6%			IC	U Level o	f Service A	
Analysis Period (min) 15							
Splits and Phases: 13: 1	N East St & M	onroe St					

Ø2 (R)		28 21-22-20
65 s 👘 👘	15 s	

Lanes, Volumes, Timings 15: N Madison St & W Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		T.			÷.						412	
Traffic Volume (vph)	0	10	5	5	20	0	0	0	0	5	840	10
Future Volume (vph)	0	10	5	5	20	0	0	0	0	5	840	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	16	12	12	14	12	12	12	12	12	12	12
Grade (%)		4%			-3%			2%			2%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		0.99			1.00							
Frt		0.953									0.998	
Flt Protected					0.990							
Satd. Flow (prot)	0	1746	0	0	1602	0	0	0	0	0	3117	0
Flt Permitted					0.948							
Satd. Flow (perm)	0	1746	0	0	1531	0	0	0	0	0	3117	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		7									3	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		692			253			310			307	
Travel Time (s)		15.7			5.8			7.0			7.0	
Confl. Peds. (#/hr)			3	3								
Peak Hour Factor	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Parking (#/hr)					0					20		20
Adj. Flow (vph)	0	13	7	7	26	0	0	0	0	7	1105	13
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	20	0	0	33	0	0	0	0	0	1125	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.17	1.00	1.17	1.12	1.18	1.12	1.16	1.16	1.16	1.16	1.16	1.16
Turning Speed (mph)	15		9	15		9	15		9	15	-	9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	l hru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		CI+Ex		CI+Ex	CI+Ex					CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		CI+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 15: N Madison St & W Jefferson St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		15.1		15.4	15.4					47.4	47.4	
Total Split (s)		18.0		18.0	18.0					62.0	62.0	
Total Split (%)		22.5%		22.5%	22.5%					77.5%	77.5%	
Maximum Green (s)		12.9		12.6	12.6					56.6	56.6	
Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)		1.9		2.2	2.2					2.2	2.2	
Lost Time Adjust (s)		0.0			0.0						0.0	
Total Lost Time (s)		5.1			5.4						5.4	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		Max	Max					C-Max	C-Max	
Walk Time (s)		1.0		1.0	1.0					31.0	31.0	
Flash Dont Walk (s)		9.0		9.0	9.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		12.9			12.6						56.6	
Actuated g/C Ratio		0.16			0.16						0.71	
v/c Ratio		0.07			0.14						0.51	
Control Delay		23.1			30.7						6.3	
Queue Delav		0.0			0.0						0.0	
Total Delay		23.1			30.7						6.3	
LOS		С			С						A	
Approach Delay		23.1			30.7						6.3	
Approach LOS		С			С						A	
90th %ile Green (s)		12.9		12.6	12.6					56.6	56.6	
90th %ile Term Code		Hold		MaxR	MaxR					Coord	Coord	
70th %ile Green (s)		12.9		12.6	12.6					56.6	56.6	
70th %ile Term Code		Hold		MaxR	MaxR					Coord	Coord	
50th %ile Green (s)		12.9		12.6	12.6					56.6	56.6	
50th %ile Term Code		Hold		MaxR	MaxR					Coord	Coord	
30th %ile Green (s)		12.9		12.6	12.6					56.6	56.6	
30th %ile Term Code		Hold		MaxR	MaxR					Coord	Coord	
10th %ile Green (s)		12.9		12.6	12.6					56.6	56.6	
10th %ile Term Code		Hold		MaxR	MaxR					Coord	Coord	
Stops (vph)		11			23						352	
Fuel Used(gal)		0			0						5	
CO Emissions (g/hr)		15			23						356	
NOx Emissions (a/hr)		3			5						69	
VOC Emissions (a/hr)		3			5						82	
Dilemma Vehicles (#)		0			0						0	
Queue Length 50th (ft)		6			14						111	
Queue Length 95th (ft)		20			33						113	
Internal Link Dist (ft)		612			173			230			227	

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 15: N Madison St & W Jefferson St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)		287			241						2206	
Starvation Cap Reductn		0			0						0	
Spillback Cap Reductn		0			0						0	
Storage Cap Reductn		0			0						0	
Reduced v/c Ratio		0.07			0.14						0.51	
Intersection Summary												
Area Type: C	BD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 11 (14%), Referenced	to phase	2: and 6:8	SBTL, Sta	art of Gre	en							
Natural Cycle: 65												
Control Type: Actuated-Coord	dinated											
Maximum v/c Ratio: 0.51												
Intersection Signal Delay: 7.3				In	tersectior	LOS: A						
Intersection Capacity Utilization	on 41.3%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
Splits and Phases: 15: N M	ladison St	& W Jeffe	erson St									

	→ Ø4
	18 s
▼ Ø6 (R)	₹ Ø8
62 s	18 s

Lanes, Volumes, Timings 16: N East St & E Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		÷.			Þ			đ þ				
Traffic Volume (vph)	5	25	0	0	5	5	20	1040	25	0	0	0
Future Volume (vph)	5	25	0	0	5	5	20	1040	25	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	11	11	12	12	12
Grade (%)		-3%			3%			0%			3%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor		0.99			0.97			1.00				
Frt					0.932			0.997				
Flt Protected		0.992						0.999				
Satd. Flow (prot)	0	1504	0	0	1534	0	0	3035	0	0	0	0
Flt Permitted		0.960						0.999				
Satd. Flow (perm)	0	1446	0	0	1534	0	0	3035	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					6			7				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		268			710			308			318	
Travel Time (s)		6.1			16.1			7.0			7.2	
Confl. Peds. (#/hr)	17					17	8		8			
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Parking (#/hr)		0					20		20			
Adj. Flow (vph)	6	30	0	0	6	6	24	1253	30	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	36	0	0	12	0	0	1307	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.12	1.28	1.12	1.17	1.12	1.17	1.14	1.19	1.19	1.17	1.17	1.17
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2			2		1	2				
Detector Template	Left	Thru			Thru		Left	Thru				
Leading Detector (ft)	20	100			100		20	100				
Trailing Detector (ft)	0	0			0		0	0				
Detector 1 Position(ft)	0	0			0		0	0				
Detector 1 Size(ft)	20	6			6		20	6				
Detector 1 Type	Cl+Ex	Cl+Ex			CI+Ex		Cl+Ex	CI+Ex				
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0				
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE
Lanes, Volumes, Timings 16: N East St & E Jefferson St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA			NA		Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4						2					
Detector Phase	4	4			8		2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0		5.0	5.0				
Minimum Split (s)	15.2	15.2			15.2		47.2	47.2				
Total Split (s)	17.0	17.0			17.0		63.0	63.0				
Total Split (%)	21.3%	21.3%			21.3%		78.8%	78.8%				
Maximum Green (s)	11.8	11.8			11.8		57.8	57.8				
Yellow Time (s)	32	3.2			32		32	3.2				
All-Red Time (s)	2.0	2.0			2.0		2.0	2.0				
Lost Time Adjust (s)	2.0	0.0			0.0		2.0	0.0				
Total Lost Time (s)		5.2			5.2			5.2				
Lead/Lag		0.2			0.2			0.2				
Lead-Lag Ontimize?												
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0				
Recall Mode	None	None			Max		C-Max	C-Max				
Walk Time (s)	10	1 0			10		31.0	31.0				
Flash Dont Walk (s)	0.0	0.0			0.0		11.0	11.0				
Pedestrian Calls (#/br)	9.0	9.0			9.0		0	0				
Act Effet Groop (c)	U	11.0			11 0		0	57.8				
Actuated a/C Patio		0.15			0.15			0.72				
Actualed y/C Natio		0.15			0.15			0.72				
V/C Rallo		20.17			0.00			0.00				
Ouque Delay		0.0			23.2			2.0				
Queue Delay		20.0			0.0			0.1				
		JZ.1			23.2			۷.۱				
LUS Approach Dolov		20.4			12.0			A 2.7				
Approach LOS		JZ.1			23.2			Z.1 A				
Approach LOS	11 0	11 0			11.0		E7 0	E7 0				
90th %ile Green (S)	11.0 Hold	.0 ∐old			II.0 MovD		0.1C	0.1C				
							57.0	57.0				
70th %ile Green (S)	0.11	0.11			II.0 MaxD		0.1C	0.1C				
South %ile Green (S)	0.11	0.11			II.0 MaxD		0.1C	0.1C				
30th %ile Green (s)	11.8	11.8			11.8 MauD		57.8	57.8				
	HOID	HOID			MaxR			Coord				
10th %ile Green (s)	11.8	11.8			11.8		57.8	57.8				
10th %ile Term Code	Hold	Hold			MaxR		Coord	Coord				
Stops (vph)		28			8			117				
Fuel Used(gal)		0			0			4				
CO Emissions (g/hr)		29			10			267				
NOx Emissions (g/hr)		6			2			52				
VOC Emissions (g/hr)		7			2			62				
Dilemma Vehicles (#)		0			0			0				
Queue Length 50th (ft)		16			3			40				
Queue Length 95th (ft)		39			16			47				
Internal Link Dist (ft)		188			630			228			238	

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 16: N East St & E Jefferson St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)		213			231			2194				
Starvation Cap Reductn		0			0			190				
Spillback Cap Reductn		0			0			0				
Storage Cap Reductn		0			0			0				
Reduced v/c Ratio		0.17			0.05			0.65				
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 55 (69%), Reference	ed to phase	2:NBTL a	ind 6:, Sta	art of Gre	en							
Natural Cycle: 65												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.60												
Intersection Signal Delay: 3	3.7			In	tersectior	n LOS: A						
Intersection Capacity Utilization	ation 52.0%			IC	CU Level o	of Service	А					
Analysis Period (min) 15												

Splits and Phases: 16: N East St & E Jefferson St

Ø2 (R)	A ₀₄
63 s	17 s
	← Ø8
	17 s

Lanes, Volumes, Timings 19: N Madison St & W Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f,		5	1					5	† 12	
Traffic Volume (vph)	0	150	20	40	215	0	0	0	0	35	760	35
Future Volume (vph)	0	150	20	40	215	0	0	0	0	35	760	35
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	13	12	12	13	12	12	12	12	12	12	12
Grade (%)		3%			-3%			2%			-2%	
Storage Length (ft)	0		0	65		0	0		0	150		0
Storage Lanes	0		0	1		0	0		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95
Frt		0.984									0.993	
Flt Protected				0.950						0.950		
Satd. Flow (prot)	0	1482	0	1585	1552	0	0	0	0	1262	3133	0
Flt Permitted				0.538						0.950		
Satd. Flow (perm)	0	1482	0	898	1552	0	0	0	0	1262	3133	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		8									10	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		690			253			324			310	
Travel Time (s)		15.7			5.8			7.4			7.0	
Peak Hour Factor	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)		0			0					20		20
Adj. Flow (vph)	0	192	26	51	276	0	0	0	0	45	974	45
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	218	0	51	276	0	0	0	0	45	1019	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12	Ŭ		12	J		12	Ū		12	Ū
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane					Yes							
Headway Factor	1.17	1.28	1.17	1.12	1.23	1.12	1.16	1.16	1.16	1.49	1.13	1.13
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		Cl+Ex	CI+Ex					CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		CI+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 19: N Madison St & W Washington St

02/22/	2024
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Lane Group	EBL EI	BT EBF	R WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	(.0		0.0						0.0	
Turn Type	1	IA	Perm	NA					Perm	NA	
Protected Phases		4		8						6	
Permitted Phases			8						6		
Detector Phase		4	8	8					6	6	
Switch Phase											
Minimum Initial (s)	Ę	.0	5.0	5.0					5.0	5.0	
Minimum Split (s)	15	.4	25.4	25.4					37.6	37.6	
Total Split (s)	26	.0	26.0	26.0					54.0	54.0	
Total Split (%)	32.5	%	32.5%	32.5%					67.5%	67.5%	
Maximum Green (s)	20	.6	20.6	20.6					48.4	48.4	
Yellow Time (s)	3	.2	3.2	3.2					3.2	3.2	
All-Red Time (s)	2	.2	2.2	2.2					2.4	2.4	
Lost Time Adjust (s)	(.0	0.0	0.0					0.0	0.0	
Total Lost Time (s)	Ę	.4	5.4	5.4					5.6	5.6	
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)	3	.0	3.0	3.0					3.0	3.0	
Recall Mode	No	ne	Max	Max					C-Max	C-Max	
Walk Time (s)	,	.0	9.0	9.0					21.0	21.0	
Flash Dont Walk (s)	Ç	.0	11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0	0	0					0	0	
Act Effct Green (s)	20	.6	20.6	20.6					48.4	48.4	
Actuated g/C Ratio	0.	26	0.26	0.26					0.60	0.60	
v/c Ratio	0.	56	0.22	0.69					0.06	0.54	
Control Delay	31	.4	13.2	19.9					6.7	8.7	
Queue Delay	(.0	0.0	1.0					0.0	0.1	
Total Delay	31	.4	13.2	20.9					6.7	8.8	
LOS		С	В	С					А	А	
Approach Delay	31	.4		19.7						8.7	
Approach LOS		С		В						А	
90th %ile Green (s)	20	.6	20.6	20.6					48.4	48.4	
90th %ile Term Code	М	ах	MaxR	MaxR					Coord	Coord	
70th %ile Green (s)	20	.6	20.6	20.6					48.4	48.4	
70th %ile Term Code	Ho	ld	MaxR	MaxR					Coord	Coord	
50th %ile Green (s)	20	.6	20.6	20.6					48.4	48.4	
50th %ile Term Code	Ho	ld	MaxR	MaxR					Coord	Coord	
30th %ile Green (s)	20	.6	20.6	20.6					48.4	48.4	
30th %ile Term Code	Ho	ld	MaxR	MaxR					Coord	Coord	
10th %ile Green (s)	20	.6	20.6	20.6					48.4	48.4	
10th %ile Term Code	Ho	ld	MaxR	MaxR					Coord	Coord	
Stops (vph)	1	40	10	70					12	281	
Fuel Used(gal)		3	0	2					0	5	
CO Emissions (g/hr)	1	94	17	118					14	342	
NOx Emissions (g/hr)		38	3	23					3	66	
VOC Emissions (g/hr)		45	4	27					3	79	
Dilemma Vehicles (#)		0	0	0					0	0	
Queue Length 50th (ft)		91	4	21					9	104	
Queue Length 95th (ft)	1	34	19	70					m17	109	

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 19: N Madison St & W Washington St

02/22/2024	ŀ
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Lane Group	EBL	ERI	EBK	WBL	WRI	WBR	NBL	NRI	NBK	SBL	SBT	SBR
Internal Link Dist (ft)		610			173			244			230	
Turn Bay Length (ft)				65						150		
Base Capacity (vph)		387		231	399					763	1899	
Starvation Cap Reductn		0		0	25					0	111	
Spillback Cap Reductn		0		0	0					0	0	
Storage Cap Reductn		0		0	0					0	0	
Reduced v/c Ratio		0.56		0.22	0.74					0.06	0.57	
Intersection Summary												
Area Type: C	BD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 78 (98%), Referenced	to phase	2: and 6:	SBTL, Sta	art of Gre	en							
Natural Cycle: 65												
Control Type: Actuated-Coord	dinated											
Maximum v/c Ratio: 0.69												
Intersection Signal Delay: 14.	.0			In	tersectior	n LOS: B						
Intersection Capacity Utilizati	on 52.5%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
m Volume for 95th percenti	le queue is	s metered	l by upstr	eam sign	al.							

Splits and Phases: 19: N Madison St & W Washington St

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54 s	26 s	

Lanes, Volumes, Timings 20: N East St & E Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	1			1	1	٢	≜ ↑₽				
Traffic Volume (vph)	10	155	0	0	235	100	30	1070	55	0	0	0
Future Volume (vph)	10	155	0	0	235	100	30	1070	55	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	11	10	11	11	11	12	12	12
Grade (%)		-3%			2%			-3%			1%	
Storage Length (ft)	65		0	0		80	80		0	0		0
Storage Lanes	1		0	0		1	1		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	1.00					0.98	1.00	1.00				
Frt						0.850		0.993				
Flt Protected	0.950						0.950					
Satd. Flow (prot)	1585	1502	0	0	1574	1291	1226	3040	0	0	0	0
Flt Permitted	0.375						0.950					
Satd. Flow (perm)	623	1502	0	0	1574	1266	1220	3040	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)						62		12				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		262			724			322			308	
Travel Time (s)		6.0			16.5			7.3			7.0	
Confl. Peds. (#/hr)	5					5	5		2			
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)		0					20		20			
Adj. Flow (vph)	13	194	0	0	294	125	38	1338	69	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	13	194	0	0	294	125	38	1407	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			11			11	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.12	1.28	1.12	1.16	1.21	1.26	1.54	1.17	1.17	1.15	1.15	1.15
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2			2	1	1	2				
Detector Template	Left	Thru			Thru	Right	Left	Thru				
Leading Detector (ft)	20	100			100	20	20	100				
Trailing Detector (ft)	0	0			0	0	0	0				
Detector 1 Position(ft)	0	0			0	0	0	0				
Detector 1 Size(ft)	20	6			6	20	20	6				
Detector 1 Type	Cl+Ex	Cl+Ex			Cl+Ex	Cl+Ex	CI+Ex	CI+Ex				
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 20: N East St & E Washington St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA	Perm	Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4				-	8	2					
Detector Phase	4	4			8	8	2	2				
Switch Phase					-	-						
Minimum Initial (s)	50	50			50	50	50	50				
Minimum Split (s)	31.4	31.4			21.4	21.4	47.0	47.0				
Total Solit (s)	24.0	24.0			24.0	24.0	56.0	56.0				
Total Split (%)	30.0%	30.0%			30.0%	30.0%	70.0%	70.0%				
Maximum Green (s)	18.6	18.6			18.6	18.6	50.0	50.0				
Vellow Time (s)	3.2	3.2			3.2	3.2	3.2	3.2				
All Ped Time (s)	J.Z 2.2	0.Z			2.2	3.Z	2.2	2.2				
All-Reu Time (S)	2.2	2.2			2.2	2.2	2.0	2.0				
Lost Time Aujust (s)	0.0	0.0 5.4			0.0	0.0	0.0	0.0				
	5.4	5.4			5.4	0.4	0.0	0.0				
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0	3.0	3.0	3.0				
Recall Mode	Max	Max			None	None	C-Max	C-Max				
Walk Time (s)	15.0	15.0			5.0	5.0	30.0	30.0				
Flash Dont Walk (s)	11.0	11.0			11.0	11.0	11.0	11.0				
Pedestrian Calls (#/hr)	0	0			0	0	0	0				
Act Effct Green (s)	18.6	18.6			18.6	18.6	50.0	50.0				
Actuated g/C Ratio	0.23	0.23			0.23	0.23	0.62	0.62				
v/c Ratio	0.09	0.56			0.81	0.37	0.05	0.74				
Control Delay	20.7	23.3			47.9	17.6	3.5	6.1				
Queue Delay	0.0	0.2			0.0	0.0	0.0	0.1				
Total Delay	20.7	23.5			47.9	17.6	3.5	6.1				
LOS	20.1 C	<u>20.0</u>			П.0 П	R 8	Δ	Δ				
Approach Delay	U	23.3			38.9	D	1	61				
Approach LOS		20.0 C			00.5 D			Δ				
90th %ile Green (s)	18.6	18.6			18.6	18.6	50.0	50.0				
90th %ile Term Code	Ped	Ped			Max	Max	Coord	Coord				
70th %ile Green (s)	18.6	18.6			18.6	18.6	50.0	50.0				
70th %ile Term Code	Pod	Dod			Max	Max	Coord	Coord				
50th %ile Green (s)	18.6	18.6			18.6	18.6	50.0	50.0				
50th %ile Term Code	Dod	Dod			Mox	Max	Coord	Coord				
20th %ile Croon (a)	10 G	10 G			10 6	19 6	50.0	50 0				
Solir %ile Green (S)	IO.0	Dod			10.0 Hold	10.0	Coord	Coord				
10th %ile Croon (a)	19.6	19.6					50.0	50.0				
10th %ile Green (S)	10.0 Ded	10.0 Ded			10.0	10.0	0.00	0.00				
Tuth %ile Term Code	Pea	Pea			Hold	HOID	Coord	Coord				
	5	69			205	47	5	224				
Fuel Used(gal)	0	1			5	1	0	5				
CO Emissions (g/hr)	6	100			332	83	9	382				
NOx Emissions (g/hr)	1	19			65	16	2	74				
VOC Emissions (g/hr)	1	23			77	19	2	89				
Dilemma Vehicles (#)	0	0			0	0	0	0				

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 20: N East St & E Washington St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)	0	27			139	25	3	63				
Queue Length 95th (ft)	12	77			#214	59	m5	68				
Internal Link Dist (ft)		182			644			242			228	
Turn Bay Length (ft)	65					80	80					
Base Capacity (vph)	144	349			365	341	762	1904				
Starvation Cap Reductn	0	11			0	0	0	32				
Spillback Cap Reductn	0	0			0	0	0	0				
Storage Cap Reductn	0	0			0	0	0	0				
Reduced v/c Ratio	0.09	0.57			0.81	0.37	0.05	0.75				
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 61 (76%), Reference	ed to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 80												
Control Type: Actuated-Coc	ordinated											
Maximum v/c Ratio: 0.81												
Intersection Signal Delay: 1-	4.5			In	tersectior	n LOS: B						
Intersection Capacity Utiliza	tion 61.3%			IC	U Level o	of Service	В					
Analysis Period (min) 15												
# 95th percentile volume e	exceeds cap	oacity, qu	eue may	be longer								
Queue shown is maximu	ım after two	cycles.										
m Volume for 95th percen	itile queue i	s meterec	l by upstr	eam sign	al.							
Splits and Phases: 20: N	East St & E	Washing	ton St									
A								· · · ·				38

	<u></u> Ø4	28
56 s	24 s	
	4	
	Ø8	
	24 s	

Lanes, Volumes, Timings 23: S Center St/N Madison St & W Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ĥ		7	•						đ î þ	
Traffic Volume (vph)	0	70	10	20	30	0	0	0	0	50	800	30
Future Volume (vph)	0	70	10	20	30	0	0	0	0	50	800	30
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	13	12	12	12	12	12	12	12	12
Grade (%)		3%			-3%			1%			-1%	
Storage Length (ft)	0		0	60		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	0		0
Taper Length (ft)	25			0			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		1.00		0.98							1.00	
Frt		0.983									0.995	
Flt Protected				0.950							0.997	
Satd. Flow (prot)	0	1413	0	1623	1488	0	0	0	0	0	3082	0
Flt Permitted				0.689							0.997	
Satd. Flow (perm)	0	1413	0	1150	1488	0	0	0	0	0	3082	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		8									10	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		684			247			660			324	
Travel Time (s)		15.5			5.6			15.0			7.4	
Confl. Peds. (#/hr)			11	11						3		5
Peak Hour Factor	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%
Parking (#/hr)		0			0					20		20
Adj. Flow (vph)	0	93	13	27	40	0	0	0	0	67	1067	40
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	106	0	27	40	0	0	0	0	0	1174	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		13			13			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.17	1.33	1.17	1.08	1.28	1.12	1.15	1.15	1.15	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		Cl+Ex	Cl+Ex					Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 23: S Center St/N Madison St & W Front St

02/22/2024

Lane Group EBL EBT EBR WBL WBT WBR NBT NBR SBL SBT SBR Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex Detector 2 Channel 0.0 </th <th></th> <th>٠</th> <th>-</th> <th>7</th> <th>1</th> <th>+</th> <th>*</th> <th>1</th> <th>Ť</th> <th>1</th> <th>1</th> <th>ŧ</th> <th>~</th>		٠	-	7	1	+	*	1	Ť	1	1	ŧ	~
Detector 2 Type CI+Ex CI+Ex CI+Ex Detector 2 Channel 0.0 0.0 0.0 Detector 2 Extend (s) 0.0 0.0 0.0 Turn Type NA Perm NA Protected Phases 4 8 6 Permitted Phases 8 6 6 Detector Phase 4 8 6 6 Switch Phase 5.0 5.0 5.0 5.0 5.0 Minimum Initial (s) 5.0 5.0 5.0 5.0 5.0 Minimum Split (s) 15.5 25.5 25.5 37.7 37.7 Total Split (s) 25.0% 25.0% 25.0% 75.0% 75.0% Maximum Green (s) 14.5 14.5 14.5 54.3 54.3 Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 All-Red Time (s) 2.3 2.3 2.3 2.5 2.5 Lost Time (s) 5.5 5.5 5.5	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s) 0.0 0.0 0.0 Turn Type NA Perm NA Perm NA Protected Phases 4 8 6 6 Permitted Phases 8 6 6 Detector Phase 4 8 6 6 Detector Phase 4 8 6 6 Switch Phase 6 6 6 6 Minimum Initial (s) 5.0 5.0 5.0 5.0 5.0 Minimum Split (s) 15.5 25.5 25.5 37.7 37.7 Total Split (s) 25.0% 25.0% 25.0% 75.0% 75.0% Maximum Green (s) 14.5 14.5 14.5 54.3 54.3 Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 All-Red Time (s) 2.3 2.3 2.3 2.5 2.5 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 L	Detector 2 Type		Cl+Ex			CI+Ex						CI+Ex	
Detector 2 Extend (s) 0.0 0.0 0.0 Turn Type NA Perm NA Perm NA Protected Phases 4 8 6 6 Permitted Phases 8 6 6 6 Detector Phase 4 8 8 6 6 Switch Phase 7 8 6 6 6 Minimum Initial (s) 5.0 5.7 5.0 5.7 5.7 5.7 5.7 5.7 5.7 5.7 5.7 5.7	Detector 2 Channel												
Turn Type NA Perm NA Perm NA Protected Phases 4 8 6 6 Permitted Phases 8 6 6 Detector Phase 4 8 8 6 Switch Phase 7 5.0 5.0 5.0 Minimum Initial (s) 5.0 5.0 5.0 5.0 Minimum Split (s) 15.5 25.5 25.5 37.7 37.7 Total Split (s) 20.0 20.0 20.0 60.0 60.0 Total Split (s) 25.0% 25.0% 75.0% 75.0% 75.0% Maximum Green (s) 14.5 14.5 14.5 54.3 54.3 Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 3.2 All-Red Time (s) 2.5 5.5 5.5 5.7 Lead/Lag Lead/Lag Lead/Lag Lead-Lag Optimize? Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 3.0<	Detector 2 Extend (s)		0.0			0.0						0.0	
Protected Phases 4 8 6 Permitted Phases 8 6 6 Detector Phase 4 8 8 6 6 Switch Phase 5.0 5.0 5.0 5.0 5.0 Minimum Initial (s) 5.0 5.0 5.0 5.0 5.0 5.0 Minimum Split (s) 15.5 25.5 25.5 37.7 37.7 Total Split (s) 20.0 20.0 20.0 60.0 60.0 Total Split (%) 25.0% 25.0% 25.0% 75.0% 75.0% Maximum Green (s) 14.5 14.5 14.5 54.3 54.3 Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 All-Red Time (s) 2.3 2.3 2.3 2.5 2.5 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 3.0 3.0 3.0 3.0 3.0 3.0	Turn Type		NA		Perm	NA					Perm	NA	
Permitted Phases 8 6 Detector Phase 4 8 8 6 6 Switch Phase	Protected Phases		4			8						6	
Detector Phase 4 8 8 6 6 Switch Phase	Permitted Phases				8						6		
Switch Phase Minimum Initial (s) 5.0 5.0 5.0 5.0 Minimum Split (s) 15.5 25.5 25.5 37.7 37.7 Total Split (s) 20.0 20.0 20.0 60.0 60.0 Total Split (%) 25.0% 25.0% 25.0% 75.0% 75.0% Maximum Green (s) 14.5 14.5 14.5 54.3 54.3 Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 All-Red Time (s) 2.3 2.3 2.3 2.5 2.5 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.5 5.5 5.7 5.7 Lead/Lag Lead-Lag Optimize? Vehicle Extension (s) 3.0	Detector Phase		4		8	8					6	6	
Minimum Initial (s) 5.0 5.0 5.0 5.0 Minimum Split (s) 15.5 25.5 25.5 37.7 37.7 Total Split (s) 20.0 20.0 20.0 60.0 60.0 Total Split (%) 25.0% 25.0% 25.0% 75.0% 75.0% Maximum Green (s) 14.5 14.5 14.5 54.3 54.3 Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 All-Red Time (s) 2.3 2.3 2.3 2.5 2.5 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.5 5.5 5.5 5.7 5.7 Lead-Lag U U U U U 0.0 3.0 </td <td>Switch Phase</td> <td></td>	Switch Phase												
Minimum Split (s) 15.5 25.5 25.5 37.7 37.7 Total Split (s) 20.0 20.0 20.0 60.0 60.0 Total Split (s) 25.0% 25.0% 25.0% 75.0% 75.0% Maximum Green (s) 14.5 14.5 14.5 54.3 54.3 Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 All-Red Time (s) 2.3 2.3 2.3 2.5 2.5 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.5 5.5 5.5 5.7 5.7 Lead-Lag Used-Lag Used-Lag 0.0 3.0 3.0 3.0 3.0 Recall Mode Max None None C-Max C-Max C-Max	Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Total Split (s) 20.0 20.0 20.0 20.0 60.0 60.0 75.0% 75.0%	Minimum Split (s)		15.5		25.5	25.5					37.7	37.7	
Total Split (%) 25.0% 25.0% 25.0% 75.0% 75.0% Maximum Green (s) 14.5 14.5 14.5 54.3 54.3 54.3 Yellow Time (s) 3.2 5.5 5.5 5.5 5.7 Lead/Lag Lead/Lag June 4.2 June 4.2 June 4.2	Total Split (s)		20.0		20.0	20.0					60.0	60.0	
Maximum Green (s) 14.5 14.5 14.5 54.3 54.3 54.3 Yellow Time (s) 3.2 3.1 3.1	Total Split (%)		25.0%		25.0%	25.0%					75.0%	75.0%	
Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 All-Red Time (s) 2.3 2.3 2.3 2.5 2.5 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.5 5.5 5.7 Lead/Lag	Maximum Green (s)		14.5		14.5	14.5					54.3	54.3	
All-Red Time (s) 2.3 2.3 2.3 2.5 2.5 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.5 5.5 5.7 Lead/Lag	Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
Lost Time Adjust (s) 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.5 5.5 5.5 5.7 Lead/Lag	All-Red Time (s)		2.3		2.3	2.3					2.5	2.5	
Total Lost Time (s) 5.5 5.5 5.7 Lead/Lag	Lost Time Adjust (s)		0.0		0.0	0.0						0.0	
Lead/Lag Lead-Lag Optimize? Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Recall Mode Max None None C-Max C-Max	Total Lost Time (s)		5.5		5.5	5.5						5.7	
Lead-Lag Optimize?Vehicle Extension (s)3.03.03.03.0Recall ModeMaxNoneC-MaxC-Max	Lead/Lag												
Vehicle Extension (s)3.03.03.03.0Recall ModeMaxNoneC-MaxC-Max	Lead-Lag Optimize?												
Recall Mode Max None None C-Max C-Max	Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
	Recall Mode		Max		None	None					C-Max	C-Max	
Walk Time (s) 1.0 9.0 9.0 21.0 21.0	Walk Time (s)		1.0		9.0	9.0					21.0	21.0	
Flash Dont Walk (s) 9.0 11.0 11.0 11.0 11.0	Flash Dont Walk (s)		9.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr) 0 0 0 0 0	Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s) 14.5 14.5 54.3	Act Effct Green (s)		14.5		14.5	14.5						54.3	
Actuated g/C Ratio 0.18 0.18 0.18 0.68	Actuated g/C Ratio		0.18		0.18	0.18						0.68	
v/c Ratio 0.40 0.13 0.15 0.56	v/c Ratio		0.40		0.13	0.15						0.56	
Control Delay 32.1 29.4 29.3 3.1	Control Delay		32.1		29.4	29.3						3.1	
Queue Delay 0.0 0.0 0.0 0.1	Queue Delav		0.0		0.0	0.0						0.1	
Total Delay 32.1 29.4 29.3 3.2	Total Delay		32.1		29.4	29.3						3.2	
LOS C C C A	LOS		С		С	С						A	
Approach Delay 32.1 29.3 3.2	Approach Delay		32.1			29.3						3.2	
Approach LOS C C A	Approach LOS		С			С						А	
90th %ile Green (s) 14.5 14.5 14.5 54.3 54.3	90th %ile Green (s)		14.5		14.5	14.5					54.3	54.3	
90th %ile Term Code MaxR Hold Hold Coord Coord	90th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
70th %ile Green (s) 14.5 14.5 14.5 54.3 54.3	70th %ile Green (s)		14.5		14.5	14.5					54.3	54.3	
70th %ile Term Code MaxR Hold Hold Coord Coord	70th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
50th %ile Green (s) 14.5 14.5 14.5 54.3 54.3	50th %ile Green (s)		14.5		14.5	14.5					54.3	54.3	
50th %ile Term Code MaxR Hold Hold Coord Coord	50th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
30th %ile Green (s) 14.5 14.5 14.5 54.3 54.3	30th %ile Green (s)		14.5		14.5	14.5					54.3	54.3	
30th %ile Term Code MaxR Hold Hold Coord Coord	30th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
10th %ile Green (s) 14.5 14.5 14.5 54.3 54.3	10th %ile Green (s)		14.5		14.5	14.5					54.3	54.3	
10th %ile Term Code MaxR Hold Hold Coord Coord	10th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
Stops (vph) 64 18 26 159	Stops (vph)		64		18	26						159	
Fuel Used(gal) $1 \qquad 0 \qquad 0 \qquad 4$	Fuel Used(gal)		1		0	0						4	
$CO = \frac{1}{1000} + \frac{1}{1000} $	CO Emissions (g/hr)		91		18	27						256	
NOx Emissions (q/hr) 18 4 5 50	NOx Emissions (g/hr)		18		4	5						50	
VOC Emissions (g/hr) 21 4 6 59	VOC Emissions (g/hr)		21		4	6						59	
Dilemma Vehicles (#) 0 0 0 0	Dilemma Vehicles (#)		0		0	Ũ						0	

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 23: S Center St/N Madison St & W Front St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)		44		11	17						32	
Queue Length 95th (ft)		73		28	36						32	
Internal Link Dist (ft)		604			167			580			244	
Turn Bay Length (ft)				60								
Base Capacity (vph)		262		208	269						2095	
Starvation Cap Reductn		0		0	0						165	
Spillback Cap Reductn		0		0	0						0	
Storage Cap Reductn		0		0	0						0	
Reduced v/c Ratio		0.40		0.13	0.15						0.61	
Intersection Summary												
Area Type: C	BD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 4 (5%), Referenced to	phase 2:	and 6:SB	TL, Start	of Green								
Natural Cycle: 65												
Control Type: Actuated-Coord	dinated											
Maximum v/c Ratio: 0.56												
Intersection Signal Delay: 6.8	}			In	tersectior	LOS: A						
Intersection Capacity Utilizati	on 44.9%			IC	U Level o	of Service	Α					
Analysis Period (min) 15												

Splits and Phases: 23: S Center St/N Madison St & W Front St

	→ Ø4	; (21112-0)
	20 s	
Ø6 (R)	Ø8	
60 s	20 s	

02/22/2024

Lanes, Volumes, Timings 24: S East St/N East St & E Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	1			1	1		đ þ				
Traffic Volume (vph)	25	50	0	0	110	30	40	1045	15	0	0	0
Future Volume (vph)	25	50	0	0	110	30	40	1045	15	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	16	12	12	10	10	13	11	11	12	12	12
Grade (%)		-1%			0%			-1%			2%	
Storage Length (ft)	90		0	0		100	0		0	0		0
Storage Lanes	1		0	0		1	0		0	0		0
Taper Length (ft)	0			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.97					0.96		1.00				
Frt						0.850		0.998				
Flt Protected	0.950							0.998				
Satd. Flow (prot)	1518	1873	0	0	1535	1304	0	3022	0	0	0	0
Flt Permitted	0.674							0.998				
Satd. Flow (perm)	1050	1873	0	0	1535	1250	0	3020	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)						35		4				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		256			710			633			322	
Travel Time (s)		5.8			16.1			14.4			7.3	
Confl. Peds. (#/hr)	15					15	19		1			
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	29	59	0	0	129	35	47	1229	18	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	29	59	0	0	129	35	0	1294	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		11			11			11			11	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.19	0.97	1.14	1.14	1.25	1.25	1.09	1.19	1.19	1.16	1.16	1.16
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2			2	1	1	2				
Detector Template	Left	Thru			Thru	Right	Left	Thru				
Leading Detector (ft)	20	100			100	20	20	100				
Trailing Detector (ft)	0	0			0	0	0	0				
Detector 1 Position(ft)	0	0			0	0	0	0				
Detector 1 Size(ft)	20	6			6	20	20	6				
Detector 1 Type	CI+Ex	Cl+Ex			Cl+Ex	Cl+Ex	Cl+Ex	CI+Ex				
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			Cl+Ex			CI+Ex				

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 24: S East St/N East St & E Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA	Perm	Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4					8	2					
Detector Phase	4	4			8	8	2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0	5.0	5.0	5.0				
Minimum Split (s)	27.2	27.2			17.2	17.2	50.6	50.6				
Total Split (s)	22.0	22.0			22.0	22.0	58.0	58.0				
Total Split (%)	27.5%	27.5%			27.5%	27.5%	72.5%	72.5%				
Maximum Green (s)	16.8	16.8			16.8	16.8	52.4	52.4				
Yellow Time (s)	3.2	3.2			3.2	3.2	3.2	3.2				
All-Red Time (s)	2.0	2.0			2.0	2.0	2.4	2.4				
Lost Time Adjust (s)	0.0	0.0			0.0	0.0		0.0				
Total Lost Time (s)	5.2	5.2			5.2	5.2		5.6				
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0	3.0	3.0	3.0				
Recall Mode	Мах	Max			None	None	C-Max	C-Max				
Walk Time (s)	11.0	11.0			1.0	1.0	34.0	34.0				
Flash Dont Walk (s)	11.0	11.0			11.0	11.0	11.0	11.0				
Pedestrian Calls (#/hr)	0	0			0	0	0	0				
Act Effct Green (s)	16.8	16.8			16.8	16.8	-	52.4				
Actuated g/C Ratio	0.21	0.21			0.21	0.21		0.66				
v/c Ratio	0.13	0.15			0.40	0.12		0.65				
Control Delay	27.6	27.0			31.7	10.6		6.4				
Queue Delay	0.0	0.0			0.0	0.0		0.0				
Total Delay	27.6	27.0			31.7	10.6		6.4				
LOS	С	С			С	В		А				
Approach Delay		27.2			27.2			6.4				
Approach LOS		С			С			А				
90th %ile Green (s)	16.8	16.8			16.8	16.8	52.4	52.4				
90th %ile Term Code	Ped	Ped			Max	Max	Coord	Coord				
70th %ile Green (s)	16.8	16.8			16.8	16.8	52.4	52.4				
70th %ile Term Code	Ped	Ped			Hold	Hold	Coord	Coord				
50th %ile Green (s)	16.8	16.8			16.8	16.8	52.4	52.4				
50th %ile Term Code	Ped	Ped			Hold	Hold	Coord	Coord				
30th %ile Green (s)	16.8	16.8			16.8	16.8	52.4	52.4				
30th %ile Term Code	Ped	Ped			Hold	Hold	Coord	Coord				
10th %ile Green (s)	16.8	16.8			16.8	16.8	52.4	52.4				
10th %ile Term Code	Ped	Ped			Hold	Hold	Coord	Coord				
Stops (vph)	22	41			93	9		303				
Fuel Used(gal)	0	1			2	0		9				
CO Emissions (g/hr)	22	42			128	20		596				
NOx Emissions (g/hr)	4	8			25	4		116				
VOC Emissions (g/hr)	5	10			30	5		138				
Dilemma Vehicles (#)	0	0			0	0		0				
Queue Length 50th (ft)	12	24			56	0		84				

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 24: S East St/N East St & E Front St

02/22/2024	ŀ
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)	32	52			100	21		93				
Internal Link Dist (ft)		176			630			553			242	
Turn Bay Length (ft)	90					100						
Base Capacity (vph)	220	393			322	290		1979				
Starvation Cap Reductn	0	0			0	0		0				
Spillback Cap Reductn	0	0			0	0		15				
Storage Cap Reductn	0	0			0	0		0				
Reduced v/c Ratio	0.13	0.15			0.40	0.12		0.66				
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 60 (75%), Reference	ed to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 80												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.65												
Intersection Signal Delay: 9	9.8			In	tersectior	n LOS: A						
Intersection Capacity Utiliz	ation 61.5%			IC	U Level o	of Service	В					
Analysis Period (min) 15												
Splits and Phases: 24: S	East St/N E	ast St & E	E Front St	t								

Ø2 (R)	<u>→</u> _{Ø4}
58 s	22 s
	 Ø8
	22 s

Lanes, Volumes, Timings 27: N Center St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,			र्स						đ þ	
Traffic Volume (vph)	0	42	23	13	112	0	0	0	0	4	139	13
Future Volume (vph)	0	42	23	13	112	0	0	0	0	4	139	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt		0.952									0.988	
Flt Protected					0.995						0.999	
Satd. Flow (prot)	0	1536	0	0	1445	0	0	0	0	0	2874	0
Flt Permitted					0.995						0.999	
Satd. Flow (perm)	0	1536	0	0	1445	0	0	0	0	0	2874	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		247			266			264			413	
Travel Time (s)		5.6			6.0			6.0			9.4	
Confl. Peds. (#/hr)			23	23						6		6
Confl. Bikes (#/hr)			1									
Peak Hour Factor	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Heavy Vehicles (%)	6%	6%	6%	6%	6%	6%	6%	6%	6%	6%	6%	6%
Parking (#/hr)					0						0	
Adj. Flow (vph)	0	53	29	16	142	0	0	0	0	5	176	16
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	82	0	0	158	0	0	0	0	0	197	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.14	1.14	1.14	1.30	1.14	1.14	1.14	1.14	1.14	1.22	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: 0	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 27.1%			IC	CU Level of	of Service	A					
Analysis Period (min) 15												

Lanes, Volumes, Timings 29: N Center St & W Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f,			ŧ						đ þ	
Traffic Volume (vph)	0	33	1	12	18	0	0	0	0	20	104	3
Future Volume (vph)	0	33	1	12	18	0	0	0	0	20	104	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt		0.997									0.996	
Flt Protected					0.981						0.992	
Satd. Flow (prot)	0	1408	0	0	1385	0	0	0	0	0	2798	0
Flt Permitted					0.981						0.992	
Satd. Flow (perm)	0	1408	0	0	1385	0	0	0	0	0	2798	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		253			255			306			278	
Travel Time (s)		5.8			5.8			7.0			6.3	
Confl. Peds. (#/hr)			7	7						13		9
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80
Heavy Vehicles (%)	9%	9%	9%	9%	9%	9%	9%	9%	9%	9%	9%	9%
Parking (#/hr)		0			0						0	
Adj. Flow (vph)	0	41	1	15	23	0	0	0	0	25	130	4
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	42	0	0	38	0	0	0	0	0	159	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.14	1.14	1.14	1.22	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: 0	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 21.5%			IC	CU Level	of Service	А					

Analysis Period (min) 15

Lanes, Volumes, Timings 30: N Center St & W Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ĥ		5	+						đ î þ	
Traffic Volume (vph)	0	165	7	23	214	0	0	0	0	50	60	7
Future Volume (vph)	0	165	7	23	214	0	0	0	0	50	60	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	65		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		1.00		1.00							0.99	
Frt		0.994									0.991	
Flt Protected				0.950							0.979	
Satd. Flow (prot)	0	1416	0	1504	1425	0	0	0	0	0	2766	0
Flt Permitted				0.538							0.979	
Satd. Flow (perm)	0	1416	0	849	1425	0	0	0	0	0	2755	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		4									9	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		253			254			326			306	
Travel Time (s)		5.8			5.8			7.4			7.0	
Confl. Peds. (#/hr)			3	3						4		10
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80
Heavy Vehicles (%)	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%
Parking (#/hr)		0			0						0	
Adj. Flow (vph)	0	206	9	29	268	0	0	0	0	63	75	9
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	215	0	29	268	0	0	0	0	0	147	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.14	1.14	1.14	1.22	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		Cl+Ex	CI+Ex					CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		Cl+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 30: N Center St & W Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		23.0		23.0	23.0					23.0	23.0	
Total Split (s)		50.0		50.0	50.0					30.0	30.0	
Total Split (%)		62.5%		62.5%	62.5%					37.5%	37.5%	
Maximum Green (s)		45.0		45.0	45.0					25.0	25.0	
Yellow Time (s)		4.0		4.0	4.0					4.0	4.0	
All-Red Time (s)		1.0		1.0	1.0					1.0	1.0	
Lost Time Adjust (s)		0.0		0.0	0.0						0.0	
Total Lost Time (s)		5.0		5.0	5.0						5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		None	None					C-Max	C-Max	
Walk Time (s)		7.0		7.0	7.0					7.0	7.0	
Flash Dont Walk (s)		11.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		21.5		21.5	21.5						48.5	
Actuated g/C Ratio		0.27		0.27	0.27						0.61	
v/c Ratio		0.56		0.13	0.70						0.09	
Control Delay		13.8		3.4	13.3						7.9	
Queue Delay		0.0		0.0	0.1						0.0	
Total Delay		13.8		3.4	13.4						7.9	
LOS		В		А	В						А	
Approach Delay		13.8			12.4						7.9	
Approach LOS		В			В						А	
90th %ile Green (s)		31.0		31.0	31.0					39.0	39.0	
90th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
70th %ile Green (s)		25.0		25.0	25.0					45.0	45.0	
70th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
50th %ile Green (s)		21.3		21.3	21.3					48.7	48.7	
50th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
30th %ile Green (s)		17.7		17.7	17.7					52.3	52.3	
30th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
10th %ile Green (s)		12.6		12.6	12.6					57.4	57.4	
10th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
Stops (vph)		43		4	75						47	
Fuel Used(gal)		1		0	1						1	
CO Emissions (g/hr)		74		6	99						51	
NOx Emissions (a/hr)		14		1	19						10	
VOC Emissions (g/hr)		17		1	23						12	
Dilemma Vehicles (#)		0		0	0						0	
Queue Length 50th (ft)		31		3	26						13	
Queue Length 95th (ft)		30		m5	30						30	

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 30: N Center St & W Washington St

02/22/2024	ŀ
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		173			174			246			226	
Turn Bay Length (ft)				65								
Base Capacity (vph)		798		477	801						1673	
Starvation Cap Reductn		35		0	70						0	
Spillback Cap Reductn		0		0	24						0	
Storage Cap Reductn		0		0	0						0	
Reduced v/c Ratio		0.28		0.06	0.37						0.09	
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80)											
Offset: 19 (24%), Reference	ced to phase	2: and 6:	SBTL, Sta	art of Gre	en							
Natural Cycle: 50												
Control Type: Actuated-Co	oordinated											
Maximum v/c Ratio: 0.70												
Intersection Signal Delay:	11.9			In	tersectior	n LOS: B						
Intersection Capacity Utiliz	zation 48.5%			IC	U Level o	of Service	A					
Analysis Period (min) 15												
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m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 30: N Center St & W Washington St

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	50 s
	+
🕈 Ø6 (R)	▼ Ø8
30 s	50 s

Lanes, Volumes, Timings 31: W Front St & N Center St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		¢Î,		٢	1			\$		٢	f,	
Traffic Volume (vph)	0	127	21	22	61	0	0	0	0	57	15	6
Future Volume (vph)	0	127	21	22	61	0	0	0	0	57	15	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	8	12	12	12	12
Storage Length (ft)	0		0	70		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.981									0.954	
Flt Protected				0.950						0.950		
Satd. Flow (prot)	0	1398	0	1504	1425	0	0	1235	0	1354	1359	0
Flt Permitted				0.950						0.950		
Satd. Flow (perm)	0	1398	0	1504	1425	0	0	1235	0	1354	1359	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		247			262			221			326	
Travel Time (s)		5.6			6.0			5.0			7.4	
Confl. Peds. (#/hr)			35	35			19		8	8		19
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%
Parking (#/hr)		0			0			0		0	0	
Adj. Flow (vph)	0	138	23	24	66	0	0	0	0	62	16	7
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	161	0	24	66	0	0	0	0	62	23	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		13			13			0			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane					Yes							
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.55	1.14	1.30	1.30	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: C	BD											
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 34.9%			IC	U Level o	of Service	А					
Analysis Period (min) 15												

Lanes, Volumes, Timings 34: N Main St & W Market St/E Market St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		÷.			ţ.			4î b				
Traffic Volume (vph)	8	41	0	0	96	6	32	29	20	0	0	0
Future Volume (vph)	8	41	0	0	96	6	32	29	20	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.992			0.963				
Flt Protected		0.992						0.981				
Satd. Flow (prot)	0	1414	0	0	1414	0	0	2700	0	0	0	0
Flt Permitted		0.992						0.981				
Satd. Flow (perm)	0	1414	0	0	1414	0	0	2700	0	0	0	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		266			263			249			404	
Travel Time (s)		6.0			6.0			5.7			9.2	
Confl. Peds. (#/hr)	13					13	1		7			
Confl. Bikes (#/hr)						1						
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Heavy Vehicles (%)	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%
Parking (#/hr)		0			0			0				
Adj. Flow (vph)	10	53	0	0	125	8	42	38	26	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	63	0	0	133	0	0	106	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.22	1.14	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: 0	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 21.6%			IC	CU Level of	of Service	А					
Analysis Period (min) 15												

Lanes, Volumes, Timings <u>36: N Main St & W Jefferson St/E Jefferson St</u>

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ŧ			f,			đ þ				
Traffic Volume (vph)	16	16	0	0	17	11	26	67	11	0	0	0
Future Volume (vph)	16	16	0	0	17	11	26	67	11	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.946			0.984				
Flt Protected		0.976						0.988				
Satd. Flow (prot)	0	1366	0	0	1324	0	0	2728	0	0	0	0
Flt Permitted		0.976						0.988				
Satd. Flow (perm)	0	1366	0	0	1324	0	0	2728	0	0	0	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		255			268			306			272	
Travel Time (s)		5.8			6.1			7.0			6.2	
Confl. Peds. (#/hr)	11					11	2		24			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	10%	10%	10%	10%	10%	10%	10%	10%	10%	10%	10%	10%
Parking (#/hr)		0			0			0				
Adj. Flow (vph)	17	17	0	0	18	12	28	73	12	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	34	0	0	30	0	0	113	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.22	1.14	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type:	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	tion 24.2%			IC	CU Level	of Service	Α					

Analysis Period (min) 15

Lanes, Volumes, Timings	
37: N Main St & W Washington St/E Washington St	t

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	•			ef.			ŧ	1			
Traffic Volume (vph)	1	194	0	0	253	30	6	25	10	0	0	0
Future Volume (vph)	1	194	0	0	253	30	6	25	10	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	65		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00				1.00			1.00	0.97			
Frt					0.986				0.850			
Flt Protected	0.950							0.990				
Satd. Flow (prot)	1504	1425	0	0	1403	0	0	1411	1211	0	0	0
Flt Permitted	0.386							0.990				
Satd. Flow (perm)	610	1425	0	0	1403	0	0	1406	1171	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					13				27			
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		254			262			326			306	
Travel Time (s)		5.8			6.0			7.4			7.0	
Confl. Peds. (#/hr)	3					3	7		6			
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80
Heavy Vehicles (%)	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%
Parking (#/hr)		0			0			0	0			
Adj. Flow (vph)	1	243	0	0	316	38	8	31	13	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	1	243	0	0	354	0	0	39	13	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.30	1.30	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2			2		1	2	1			
Detector Template	Left	Thru			Thru		Left	Thru	Right			
Leading Detector (ft)	20	100			100		20	100	20			
Trailing Detector (ft)	0	0			0		0	0	0			
Detector 1 Position(ft)	0	0			0		0	0	0			
Detector 1 Size(ft)	20	6			6		20	6	20			
Detector 1 Type	Cl+Ex	Cl+Ex			CI+Ex		CI+Ex	Cl+Ex	Cl+Ex			
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			Cl+Ex			Cl+Ex				
Detector 2 Channel												

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings	
37: N Main St & W Washington St/E Washington	St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA		Perm	NA	Perm			
Protected Phases		4			8			2				
Permitted Phases	4						2		2			
Detector Phase	4	4			8		2	2	2			
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0		5.0	5.0	5.0			
Minimum Split (s)	23.0	23.0			23.0		23.0	23.0	23.0			
Total Split (s)	52.0	52.0			52.0		28.0	28.0	28.0			
Total Split (%)	65.0%	65.0%			65.0%		35.0%	35.0%	35.0%			
Maximum Green (s)	47.0	47.0			47.0		23.0	23.0	23.0			
Yellow Time (s)	4.0	4.0			4.0		4.0	4.0	4.0			
All-Red Time (s)	1.0	1.0			1.0		1.0	1.0	1.0			
Lost Time Adjust (s)	0.0	0.0			0.0			0.0	0.0			
Total Lost Time (s)	5.0	5.0			5.0			5.0	5.0			
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0	3.0			
Recall Mode	None	None			None		C-Max	C-Max	C-Max			
Walk Time (s)	7.0	7.0			7.0		7.0	7.0	7.0			
Flash Dont Walk (s)	11.0	11.0			11.0		11.0	11.0	11.0			
Pedestrian Calls (#/hr)	0	0			0		0	0	0			
Act Effct Green (s)	26.9	26.9			26.9			43.1	43.1			
Actuated g/C Ratio	0.34	0.34			0.34			0.54	0.54			
v/c Ratio	0.00	0.51			0.74			0.05	0.02			
Control Delay	23.0	32.4			13.0			13.8	5.3			
Queue Delay	0.0	0.1			0.1			0.0	0.0			
Total Delay	23.0	32.6			13.1			13.8	5.3			
LOS	С	С			В			В	А			
Approach Delay		32.5			13.1			11.7				
Approach LOS		С			В			В				
90th %ile Green (s)	38.1	38.1			38.1		31.9	31.9	31.9			
90th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
70th %ile Green (s)	31.3	31.3			31.3		38.7	38.7	38.7			
70th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
50th %ile Green (s)	26.7	26.7			26.7		43.3	43.3	43.3			
50th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
30th %ile Green (s)	22.3	22.3			22.3		47.7	47.7	47.7			
30th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
10th %ile Green (s)	16.1	16.1			16.1		53.9	53.9	53.9			
10th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
Stops (vph)	2	134			75			22	4			
Fuel Used(gal)	0	2			2			0	0			
CO Emissions (g/hr)	1	168			122			20	4			
NOx Emissions (g/hr)	0	33			24			4	1			
VOC Emissions (g/hr)	0	39			28			5	1			
Dilemma Vehicles (#)	0	0			0			0	0			
Queue Length 50th (ft)	0	102			23			12	0			
Queue Length 95th (ft)	m1	92			m24			m31	m6			

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, T	imings	
37: N Main St & W	Washington St/E	Washington St

02/22/2024

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Lane Group	FBI	FBT	FBR	WBI	WBT	WBR	NBI	NBT	NBR	SBI	SBT	SBR
Internal Link Dist (ft)		174	2011		182			246		002	226	0.0.1
Turn Bay Length (ft)	65											
Base Capacity (vph)	358	837			829			757	643			
Starvation Cap Reductn	0	129			65			0	0			
Spillback Cap Reductn	0	0			0			0	0			
Storage Cap Reductn	0	0			0			0	0			
Reduced v/c Ratio	0.00	0.34			0.46			0.05	0.02			
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 7 (9%), Referenced	to phase 2:1	VBTL and	6:, Start	of Green								
Natural Cycle: 50												
Control Type: Actuated-Coc	ordinated											
Maximum v/c Ratio: 0.74												
Intersection Signal Delay: 2	0.3			In	tersectior	n LOS: C						
Intersection Capacity Utiliza	ation 48.5%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
m Volume for 95th percent	ntile queue is	s metered	l by upstr	eam sign	al.							

Splits and Phases: 37: N Main St & W Washington St/E Washington St

[™] Ø2 (R)		
28 s	52 s	-
	← Ø8	
	52 s	

Lanes, Volumes, Timings 38: W Front St/E Front St & N Main St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	ţ,			\$			\$				
Traffic Volume (vph)	33	150	0	0	89	29	0	0	0	0	0	0
Future Volume (vph)	33	150	0	0	89	29	0	0	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	8	12	12	12	12
Storage Length (ft)	70		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.967							
Flt Protected	0.950											
Satd. Flow (prot)	1504	1425	0	0	1378	0	0	1308	0	0	0	0
Flt Permitted	0.950											
Satd. Flow (perm)	1504	1425	0	0	1378	0	0	1308	0	0	0	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		262			256			210			326	
Travel Time (s)		6.0			5.8			4.8			7.4	
Confl. Peds. (#/hr)	17		2	2		17	7		36			
Peak Hour Factor	0.83	0.83	0.92	0.92	0.83	0.83	0.92	0.92	0.92	0.83	0.92	0.83
Heavy Vehicles (%)	8%	8%	2%	2%	8%	8%	2%	2%	2%	8%	2%	8%
Parking (#/hr)		0			0			0				
Adj. Flow (vph)	40	181	0	0	107	35	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	40	181	0	0	142	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.55	1.14	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type: C	BD											
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 27.1%			IC	U Level o	of Service	A					
Analysis Period (min) 15												

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		1	≜ t}			
Traffic Volume (vph)	0	17	1022	7	0	0
Future Volume (vph)	0	17	1022	7	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	11	11	12	12
Grade (%)	0%		-2%			2%
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Frt		0.865	0.999			
Flt Protected						
Satd. Flow (prot)	0	1292	3077	0	0	0
Flt Permitted						
Satd. Flow (perm)	0	1292	3077	0	0	0
Link Speed (mph)	30		30			30
Link Distance (ft)	558		266			127
Travel Time (s)	12.7		6.0			2.9
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%
Parking (#/hr)		0				
Adj. Flow (vph)	0	21	1246	9	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	21	1255	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		0			0
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.14	1.30	1.18	1.18	1.16	1.16
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Stop
Intersection Summary						
Area Type: 0	CBD					
Control Type: Unsignalized						
Intersection Capacity Utilizat	ion 41.6%			IC	U Level o	of Service A

Analysis Period (min) 15

Lanes, Volumes, Timings 47: S Center St & W Olive St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1	1	7	•						41	1
Traffic Volume (vph)	0	60	35	5	35	0	0	0	0	45	745	55
Future Volume (vph)	0	60	35	5	35	0	0	0	0	45	745	55
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	11	10	12	12	12	12	12	12	12	12	12
Grade (%)		1%			-3%			3%			-1%	
Storage Length (ft)	0		80	0		0	0		0	0		50
Storage Lanes	0		1	1		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00
Frt			0.850									0.850
Flt Protected				0.950							0.997	
Satd, Flow (prot)	0	1792	1470	1796	1891	0	0	0	0	0	3546	1591
Flt Permitted				0.715							0.997	
Satd, Flow (perm)	0	1792	1470	1352	1891	0	0	0	0	0	3546	1591
Right Turn on Red	-		Yes			Yes	-	-	Yes	-		Yes
Satd. Flow (RTOR)			40									53
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		777			330			770			660	
Travel Time (s)		17.7			7.5			17.5			15.0	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adi Flow (vph)	0.02	65	38	5	38	0.02	0.02	0.02	0.02	49	810	60
Shared Lane Traffic (%)	•			•		•	, The second sec	•	, in the second s	.•	••••	
Lane Group Flow (vph)	0	65	38	5	38	0	0	0	0	0	859	60
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12	J -		12	J -		0	J -		0	J -
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.01	1.05	1.10	0.98	0.98	0.98	1.02	1.02	1.02	0.99	0.99	0.99
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2	1	1	2					1	2	1
Detector Template		Thru	Right	Left	Thru					Left	Thru	Right
Leading Detector (ft)		100	20	20	100					20	100	20
Trailing Detector (ft)		0	0	0	0					0	0	0
Detector 1 Position(ft)		0	0	0	0					0	0	0
Detector 1 Size(ft)		6	20	20	6					20	6	20
Detector 1 Type		CI+Ex	CI+Ex	CI+Ex	Cl+Ex					Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 1 Queue (s)		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 1 Delay (s)		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		CI+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
т. т.			-	-						-		Dorm

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 47: S Center St & W Olive St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases		4			8						6	
Permitted Phases			4	8						6		6
Detector Phase		4	4	8	8					6	6	6
Switch Phase												
Minimum Initial (s)		5.0	5.0	5.0	5.0					5.0	5.0	5.0
Minimum Split (s)		13.5	13.5	13.5	13.5					37.9	37.9	37.9
Total Split (s)		21.0	21.0	21.0	21.0					59.0	59.0	59.0
Total Split (%)		26.3%	26.3%	26.3%	26.3%					73.8%	73.8%	73.8%
Maximum Green (s)		15.5	15.5	15.5	15.5					53.1	53.1	53.1
Yellow Time (s)		3.2	3.2	3.2	3.2					3.2	3.2	3.2
All-Red Time (s)		2.3	2.3	2.3	2.3					2.7	2.7	2.7
Lost Time Adjust (s)		0.0	0.0	0.0	0.0						0.0	0.0
Total Lost Time (s)		5.5	5.5	5.5	5.5						5.9	5.9
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0	3.0	3.0	3.0					3.0	3.0	3.0
Recall Mode		Max	Max	None	None					C-Max	C-Max	C-Max
Walk Time (s)		1.0	1.0	1.0	1.0					21.0	21.0	21.0
Flash Dont Walk (s)		7.0	7.0	7.0	7.0					11.0	11.0	11.0
Pedestrian Calls (#/hr)		0	0	0	0					0	0	0
Act Effct Green (s)		15.5	15.5	15.5	15.5					-	53.1	53.1
Actuated g/C Ratio		0.19	0.19	0.19	0.19						0.66	0.66
v/c Ratio		0.19	0.12	0.02	0.10						0.37	0.06
Control Delay		28.7	10.0	25.2	25.3						1.4	0.1
Queue Delay		0.0	0.0	0.0	0.0						0.0	0.0
Total Delay		28.7	10.0	25.2	25.3						1.4	0.1
LOS		С	В	С	С						А	А
Approach Delay		21.8			25.3						1.3	
Approach LOS		С			С						А	
90th %ile Green (s)		15.5	15.5	15.5	15.5					53.1	53.1	53.1
90th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
70th %ile Green (s)		15.5	15.5	15.5	15.5					53.1	53.1	53.1
70th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
50th %ile Green (s)		15.5	15.5	15.5	15.5					53.1	53.1	53.1
50th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
30th %ile Green (s)		15.5	15.5	15.5	15.5					53.1	53.1	53.1
30th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
10th %ile Green (s)		15.5	15.5	15.5	15.5					53.1	53.1	53.1
10th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
Stops (vph)		49	10	7	30						76	0
Fuel Used(gal)		1	0	0	0						5	0
CO Emissions (g/hr)		69	24	5	30						329	20
NOx Emissions (g/hr)		13	5	1	6						64	4
VOC Emissions (g/hr)		16	5	1	7						76	5
Dilemma Vehicles (#)		0	0	0	0						0	0
Queue Length 50th (ft)		27	0	2	16						10	0
Queue Length 95th (ft)		61	23	m6	m35						12	m0
Internal Link Dist (ft)		697			250			690			580	
Turn Bay Length (ft)			80									50

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 47: S Center St & W Olive St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Base Capacity (vph)		347	317	261	366					-	2353	1073
Starvation Cap Reductn		0	0	0	0						0	0
Spillback Cap Reductn		0	0	0	0						0	0
Storage Cap Reductn		0	0	0	0						0	0
Reduced v/c Ratio		0.19	0.12	0.02	0.10						0.37	0.06
Intersection Summary												
Area Type: Ot	her											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 21 (26%), Referenced	to phase	2: and 6:	SBTL, Sta	art of Gree	en							
Natural Cycle: 55												
Control Type: Actuated-Coord	inated											
Maximum v/c Ratio: 0.37												
Intersection Signal Delay: 4.2				In	tersectior	LOS: A						
Intersection Capacity Utilizatio	n 44.3%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
m Volume for 95th percentile	e queue is	s metereo	l by upstr	eam sign	al.							
Splits and Phases: 47: S Ce	enter St &	W Olive	St					-				<i>2</i> 5



Lanes, Volumes, Timings 48: S East St & E Olive St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		÷.			≜ î≽			đ î de				
Traffic Volume (vph)	60	55	0	0	15	25	30	1075	10	0	0	0
Future Volume (vph)	60	55	0	0	15	25	30	1075	10	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	12	12	12	12	12	12	12	12	12
Grade (%)		2%			-4%			4%			-4%	
Storage Length (ft)	0		0	0		0	300		0	0		0
Storage Lanes	0		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00
Frt					0.906			0.999				
Flt Protected		0.975						0.999				
Satd, Flow (prot)	0	1677	0	0	3177	0	0	3363	0	0	0	0
Flt Permitted		0.813						0.999				
Satd, Flow (perm)	0	1398	0	0	3177	0	0	3363	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					27			3				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		330			647			682			633	
Travel Time (s)		7.5			14.7			15.5			14.4	
Peak Hour Factor	0 92	0.92	0 92	0 92	0.92	0 92	0.92	0.92	0 92	0 92	0.92	0 92
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%
Parking (#/hr)	0,0	0	0,0	0,0	0,0	070	0,0	0,0	0,0	070	0,0	070
Adi Flow (vph)	65	60	0	0	16	27	33	1168	11	0	0	0
Shared Lane Traffic (%)		00	Ū	v	10	21	00	1100		v	Ū	Ū
Lane Group Flow (vph)	0	125	0	0	43	0	0	1212	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)	Lon	0	rugite	Lon	0	rugitt	Lon	0	rugite	Lon	0	ragin
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		10			10			10			10	
Headway Eactor	1 01	1 07	1 01	0 97	0 97	0 97	1.03	1.03	1.03	0 97	0 97	0 97
Turning Speed (mph)	1.01	1.07	9	15	0.07	0.07 Q	15	1.00	9	15	0.01	0.07 Q
Number of Detectors	1	2	Ū	10	2	Ū	1	2	Ŭ	10		Ū
Detector Template	Left	Thru			Thru		l eft	Thru				
Leading Detector (ft)	20	100			100		20	100				
Trailing Detector (ft)	0	0			0		20	0				
Detector 1 Position(ft)	0	0			0		0	0				
Detector 1 Size(ft)	20	6			6		20	6				
Detector 1 Type	CI+Ex	CI+Ex			CI+Ex		CI+Ex	CI+Ex				
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0				
Detector 2 Decition(#)	0.0	0.0			0.0		0.0	0.0				
Detector 2 Position(II)		94			94			94				
Detector 2 Size(II)												
Detector 2 Type		UI+EX			UI+EX			UI+EX				
Delector 2 Channel												

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 48: S East St & E Olive St

02/22/	2024
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA		Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4						2	_				
Detector Phase	4	4			8		2	2				
Switch Phase	•	•			U		-	-				
Minimum Initial (s)	50	50			50		50	50				
Minimum Split (s)	17.5	17.5			17.5		60.6	60.6				
Total Split (s)	18.0	18.0			18.0		62.0	62.0				
Total Split (%)	22.5%	22.5%			22.5%		77.5%	77.5%				
Maximum Green (s)	12.5 /0	12.5			12.5 /0		56 /	56 /				
Vellow Time (s)	3.2	3.0			3.2		3.2	3.0				
All Pod Time (s)	J.Z 2 2	J.Z 2 2			0.2		J.Z 2 /	J.Z 2.4				
All-Reu Tille (S)	2.0	2.3			2.3		2.4	2.4				
Lost Time Aujust (S)		0.0			0.0			0.0				
Total Lost Time (S)		0.0			0.0			0.C				
Leau/Lag												
Leau-Lag Optimize?	2.0	2.0			2.0		2.0	2.0				
Venicle Extension (s)	3.U	3.U			3.U		3.U	3.U				
	Max	Max			None		C-Max	C-Max				
Walk Time (s)	1.0	1.0			1.0		44.0	44.0				
Flash Dont Walk (s)	11.0	11.0			11.0		11.0	11.0				
Pedestrian Calls (#/hr)	0	0			0		0	0				
Act Effet Green (s)		12.5			12.5			56.4				
Actuated g/C Ratio		0.16			0.16			0.70				
v/c Ratio		0.57			0.08			0.51				
Control Delay		43.2			16.4			6.3				
Queue Delay		0.0			0.0			0.0				
Total Delay		43.2			16.4			6.3				
LOS		D			В			A				
Approach Delay		43.2			16.4			6.3				
Approach LOS		D			В			А				
90th %ile Green (s)	12.5	12.5			12.5		56.4	56.4				
90th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
70th %ile Green (s)	12.5	12.5			12.5		56.4	56.4				
70th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
50th %ile Green (s)	12.5	12.5			12.5		56.4	56.4				
50th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
30th %ile Green (s)	12.5	12.5			12.5		56.4	56.4				
30th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
10th %ile Green (s)	12.5	12.5			12.5		56.4	56.4				
10th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
Stops (vph)		112			20			463				
Fuel Used(gal)		2			0			10				
CO Emissions (g/hr)		135			31			694				
NOx Emissions (a/hr)		26			6			135				
VOC Emissions (a/hr)		31			7			161				
Dilemma Vehicles (#)		0			0			0				
Queue Lenath 50th (ff)		66			3			121				
Queue Length 95th (ft)		#125			17			160				

2041 AM-IDOT Downtown Bloomington 3:54 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 48: S East St & E Olive St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		250			567			602			553	
Turn Bay Length (ft)												
Base Capacity (vph)		218			519			2371				
Starvation Cap Reductn		0			0			0				
Spillback Cap Reductn		0			0			0				
Storage Cap Reductn		0			0			0				
Reduced v/c Ratio		0.57			0.08			0.51				
Intersection Summary												
Area Type: C	Other											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 55 (69%), Referenced	l to phase	2:NBTL a	nd 6:, Sta	art of Gree	en							
Natural Cycle: 80												
Control Type: Actuated-Coor	dinated											
Maximum v/c Ratio: 0.57												
Intersection Signal Delay: 10	.0			Int	tersectior	n LOS: B						
Intersection Capacity Utilizati	on 53.0%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
# 95th percentile volume exceeds capacity, queue may be longer.												
Queue shown is maximun	n after two	cycles.										

Splits and Phases: 48: S East St & E Olive St

Ø2 (R)	 A ₀₄
62 s	18 s
	← Ø8
	18 s

02/22/2024

Lanes, Volumes, Timings 3: N Madison St/N Center St & W Locust St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑ 1₀			ŧ						t ↑ 1 •	1
Traffic Volume (vph)	0	491	69	0	Ō	0	0	0	0	216	1310	0
Future Volume (vph)	0	491	69	0	0	0	0	0	0	216	1310	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	0		0	0		150
Storage Lanes	0		0	0		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00
Ped Bike Factor		1.00									1.00	
Frt		0.982										
Flt Protected											0.993	
Satd. Flow (prot)	0	3468	0	0	1863	0	0	0	0	0	3514	1863
Flt Permitted											0.993	
Satd. Flow (perm)	0	3468	0	0	1863	0	0	0	0	0	3507	1863
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		21										
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		659			244			860			526	
Travel Time (s)		15.0			5.5			19.5			12.0	
Confl. Peds. (#/hr)			4							12		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	0	522	73	0	0	0	0	0	0	230	1394	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	595	0	0	0	0	0	0	0	0	1624	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	1
Detector Template		Thru		Left	Thru					Left	Thru	Right
Leading Detector (ft)		100		20	100					20	100	20
Trailing Detector (ft)		0		0	0					0	0	0
Detector 1 Position(ft)		0		0	0					0	0	0
Detector 1 Size(ft)		6		20	6					20	6	20
Detector 1 Type		Cl+Ex		Cl+Ex	CI+Ex					Cl+Ex	CI+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		Cl+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA								Perm	NA	Perm

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 3: N Madison St/N Center St & W Locust St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases		4			8						6	
Permitted Phases				8						6		6
Detector Phase		4		8	8					6	6	6
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	5.0
Minimum Split (s)		23.4		22.5	22.5					23.4	23.4	23.4
Total Split (s)		23.6		22.5	22.5					46.4	46.4	46.4
Total Split (%)		33.7%		32.1%	32.1%					66.3%	66.3%	66.3%
Maximum Green (s)		18.2		18.0	18.0					41.0	41.0	41.0
Yellow Time (s)		3.2		3.5	3.5					3.2	3.2	3.2
All-Red Time (s)		2.2		1.0	1.0					2.2	2.2	2.2
Lost Time Adjust (s)		0.0			0.0						0.0	0.0
Total Lost Time (s)		5.4			4.5						5.4	5.4
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	3.0
Recall Mode		None		None	None					C-Max	C-Max	C-Max
Walk Time (s)		7.0		7.0	7.0					7.0	7.0	7.0
Flash Dont Walk (s)		11.0		11.0	11.0					11.0	11.0	11.0
Pedestrian Calls (#/hr)		0		0	0					0	0	0
Act Effct Green (s)		16.4									42.8	
Actuated g/C Ratio		0.23									0.61	
v/c Ratio		0.72									0.76	
Control Delay		29.0									13.2	
Queue Delav		0.8									2.4	
Total Delay		29.8									15.6	
LOS		С									В	
Approach Delay		29.8									15.6	
Approach LOS		С									В	
90th %ile Green (s)		18.2		19.1	19.1					41.0	41.0	41.0
90th %ile Term Code		Max		Hold	Hold					Coord	Coord	Coord
70th %ile Green (s)		18.2		19.1	19.1					41.0	41.0	41.0
70th %ile Term Code		Max		Hold	Hold					Coord	Coord	Coord
50th %ile Green (s)		17.5		18.4	18.4					41.7	41.7	41.7
50th %ile Term Code		Gap		Hold	Hold					Coord	Coord	Coord
30th %ile Green (s)		15.7		16.6	16.6					43.5	43.5	43.5
30th %ile Term Code		Gap		Hold	Hold					Coord	Coord	Coord
10th %ile Green (s)		12.3		13.2	13.2					46.9	46.9	46.9
10th %ile Term Code		Gap		Hold	Hold					Coord	Coord	Coord
Stops (vph)		475									1053	
Fuel Used(gal)		9									16	
CO Emissions (g/hr)		615									1132	
NOx Emissions (g/hr)		120									220	
VOC Emissions (g/hr)		142									262	
Dilemma Vehicles (#)		0									0	
Queue Length 50th (ft)		117									247	
Queue Length 95th (ft)		167									344	
Internal Link Dist (ft)		579			164			780			446	
Turn Bay Length (ft)												

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 3: N Madison St/N Center St & W Locust St

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		0.00	•	•			,	20.05		2136	•	2000
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Base Capacity (vph)		917									2145	
Starvation Cap Reductn		0									0	
Spillback Cap Reductn		117									377	
Storage Cap Reductn		0									0	
Reduced v/c Ratio		0.74									0.92	
Intersection Summary												
Area Type:	Other											
Cycle Length: 70												
Actuated Cycle Length: 70)											
Offset: 45 (64%), Referen	ced to phase	2: and 6:	SBTL, Sta	art of Gre	en							
Natural Cycle: 60												
Control Type: Actuated-Co	oordinated											
Maximum v/c Ratio: 0.76												
Intersection Signal Delay:	19.4			In	tersectior	n LOS: B						
Intersection Capacity Utiliz	zation 67.3%			IC	CU Level o	of Service	С					
Analysis Period (min) 15												

Splits and Phases: 3: N Madison St/N Center St & W Locust St

		→ Ø4
		23.6 s
Ø6 (R)	★ Ø8	
46.4 s	22.5 s	

02/22/2024
Lanes, Volumes, Timings <u>6: N East St/N Main St & W Locust St/E Locust St</u>

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		.at≜						41.				
Traffic Volume (vph)	113	599	0	0	0	0	0	1109	231	0	0	0
Future Volume (vph)	113	599	0	0	0	0	0	1109	231	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util, Factor	0.95	0.95	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor		1.00						1.00				
Frt								0.974				
Flt Protected		0.992										
Satd, Flow (prot)	0	3511	0	0	0	0	0	3438	0	0	0	0
Flt Permitted		0.992										-
Satd. Flow (perm)	0	3508	0	0	0	0	0	3438	0	0	0	0
Right Turn on Red	-		Yes	-	-	Yes	-		Yes	-	-	Yes
Satd. Flow (RTOR)								41				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		244			514			878			525	
Travel Time (s)		5.5			11.7			20.0			11.9	
Confl. Peds. (#/hr)	5	0.0							3			
Confl. Bikes (#/hr)	-								1			
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adi, Flow (vph)	122	644	0	0	0	0	0	1192	248	0	0	0
Shared Lane Traffic (%)		••••	· ·	•	•	•	•			•	•	·
Lane Group Flow (vph)	0	766	0	0	0	0	0	1440	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0	J •		0	J *		0	J -		0	5
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2					1	2				
Detector Template	Left	Thru					Left	Thru				
Leading Detector (ft)	20	100					20	100				
Trailing Detector (ft)	0	0					0	0				
Detector 1 Position(ft)	0	0					0	0				
Detector 1 Size(ft)	20	6					20	6				
Detector 1 Type	CI+Ex	Cl+Ex					Cl+Ex	CI+Ex				
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0				
Detector 1 Queue (s)	0.0	0.0					0.0	0.0				
Detector 1 Delay (s)	0.0	0.0					0.0	0.0				
Detector 2 Position(ft)		94						94				
Detector 2 Size(ft)		6						6				
Detector 2 Type		Cl+Ex						CI+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0				
Turn Type	Perm	NA						NA				
Protected Phases		4						2				
Permitted Phases	4						2					

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings	
6: N East St/N Main St & W Locust St/E Locus	st St

02/22/2024

Lane Group EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR Detector Phase 4 4 2 2 2 2 Minimum Initial (s) 5.0		٨	-	7	1	+	*	1	Ť	1	4	ţ	~
Detector Phase 4 4 2 2 Switch Phase	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase 5.0 5.0 5.0 Minimum Initial (s) 5.0 5.0 5.0 Total Spit (s) 24.0 24.0 46.0 Total Spit (s) 24.0 24.0 46.0 Total Spit (s) 34.3% 35.7% 65.7% Maximum Green (s) 18.5 18.5 40.5 40.5 Velice Time (s) 3.2 3.2 3.2 3.2 3.2 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 Total Lost Time Adjust (s) 5.5 5.5 Lead-Lag Optimize? Vehicle Extension (s) 3.0 3.0 3.0 3.0 Recall Mode None None C-Max C-Max Wait Time (s) 7.0 7.0 7.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 A.0 A Velicit Extension (s) 3.0 0 0 0 0 A A A A A A A A A A	Detector Phase	4	4					2	2				
Minimum Initial (s) 5.0 5.0 5.0 5.0 Minimum Split (s) 23.5 23.5 23.5 23.5 Total Split (s) 24.0 24.0 46.0 46.0 Total Split (%) 34.3% 65.7% 65.7% Maximum Green (s) 18.5 18.5 40.5 Velow Time (s) 2.3 2.3 2.3 Lead Lag 0 0.0 0.0 Total Split (%) 0.0 0.0 0.0 Total Split (%) 10.0 11.0 11.0 Total Split (%) 10.0 11.0 11.0 Peckethin Cals (#hr) 0 0 0 0 Actified Green (s) 18.1 40.9 Actuated grit (Green (s) 18.5 Actuated grit (Green (s) 18.5 Actuated grit (Green (s) 18.5 Actuated grit (Green (s) <	Switch Phase												
Minimum Split (s) 23.5 23.5 23.5 Total Split (s) 24.0 24.0 46.0 46.0 Total Split (s) 24.3% 3.3% 65.7% 65.7% Total Split (s) 34.3% 3.3% 65.7% 65.7% Vellow Time (s) 3.2 3.2 3.2 3.2 All Red Time (s) 2.3 2.3 2.3 2.3 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.5 5.5 Lead-Lag Optimize? Vehicle Extension (s) 3.0 3.0 3.0 3.0 Vehicle Extension (s) 7.0 7.0 7.0 7.0 7.0 7.0 Fleah Dont Walk (s) 11.0 11.0 11.0 11.0 11.0 11.0 11.0 11.0 11.0 11.0 11.0 11.0 11.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0 12.0	Minimum Initial (s)	5.0	5.0					5.0	5.0				
Total Split (%) 24.0 24.0 46.0 46.0 46.0 Total Split (%) 34.3% 34.3% 65.7% 65.7% Waximum Green (s) 18.5 18.5 40.5 40.5 Yellow Time (s) 2.2 3.2 3.2 3.2 3.2 Lost Time Adjust (s) 0.0 0.0 0.0 10.0 Total Lost Time (s) 5.5 5.5 15.5 10.0 Lead/Lag Detrimine (s) 7.0 7.0 7.0 7.0 Total Split (%) 0.0 0 0 0 0 10.0 11.0 11.0 11.0 11.0 11.0 11.0 11.0 11.0 11.0 11.0 11.0 11.0 11.0 11.0 10.0 11.0 <td>Minimum Split (s)</td> <td>23.5</td> <td>23.5</td> <td></td> <td></td> <td></td> <td></td> <td>23.5</td> <td>23.5</td> <td></td> <td></td> <td></td> <td></td>	Minimum Split (s)	23.5	23.5					23.5	23.5				
Total Split (%) 34.3% 36.3% 65.7% 65.7% Maximum Green (s) 18.5 18.5 40.5 40.5 Vellov Time (s) 2.2 3.2 3.2 3.2 Lost Time Adjust (s) 0.0 0.0 10.0 Total Lost Time (s) 5.5 5.5 Lead-Lag Optimize? Vehicle Extension (s) 3.0 3.0 3.0 Recall Mode None None C-Max C-Max Waik Time (s) 7.0 7.0 7.0 7.0 Flesh Dont Walk (s) 11.0 11.0 11.0 11.0 Venice Extension (s) 18.1 40.9 Actuated g/C Ratio 0.26 0.58 Vic Ratio 0.26 0.58 0.0 10.0 <td>Total Split (s)</td> <td>24.0</td> <td>24.0</td> <td></td> <td></td> <td></td> <td></td> <td>46.0</td> <td>46.0</td> <td></td> <td></td> <td></td> <td></td>	Total Split (s)	24.0	24.0					46.0	46.0				
Tax spin (s) Total S 18.5 40.5 40.5 Value Time (s) 3.2 3.2 3.2 3.2 Lost Time (s) 3.2 3.2 3.2 3.2 Lost Time (s) 5.5 5.5 5.5 LeadLag Optimize? Vehicle Extension (s) 3.0 3.0 3.0 Recail Mode None None C-Max C-Max Walk Time (s) 7.0 7.0 7.0 Total Lost Time (s) 7.0 7.0 7.0 Total Lost Walk Time (s) 7.0 7.0 7.0 Total Lost Time (s) 7.0 7.0 Total Lost C-Max Walk Time (s) 1.0 11.0 11.0 11.0 11.0 Pedestrian Calls (#hr) 0 0 0 0 0 A Control Delay 34.4 7.7 Queue Delay 50.0 0.0 0.0 0 0 0 0 0 0 0 0 0 <t< td=""><td>Total Split (%)</td><td>34.3%</td><td>34.3%</td><td></td><td></td><td></td><td></td><td>65.7%</td><td>65.7%</td><td></td><td></td><td></td><td></td></t<>	Total Split (%)	34.3%	34.3%					65.7%	65.7%				
Instruction Image: Second	Maximum Green (s)	18.5	18.5					40.5	40.5				
All-Red Time (s) 2.3 2.3 2.3 2.3 Lost Time Adjust (s) 0.0 0.0 0.0 Total Lost Time (s) 5.5 5.5 Lead-Lag Optimize? Vehicle Extension (s) 3.0 3.0 3.0 Recall Mode None C-Max C-Max Vehicle Extension (s) 7.0 Time (s) 7.0 7.0 7.0 7.0 7.0 Flash Dort Walk (s) 11.0 11.0 11.0 11.0 11.0 Pedestrian Calls (#Int) 0 0 0 0 0 Actuated g/C Ratio 0.26 0.58	Yellow Time (s)	3.2	3.2					3.2	3.2				
Los Time Aglust (s) Los Los Lost Time Aglust (s) 0.0 0.0 Total Lost Time (s) 5.5 5.5 Lead/Lag Lead/Lag Lead/Lag Lead/Lag Optimize? Vehicle Extension (s) 3.0 3.0 Recall Mode None None CMax Walk Time (s) 7.0 7.0 7.0 Flext Dont Walk (s) 11.0 11.0 11.0 Pedestrian Calls (#hr) 0 0 0 Actuated g/C Ratio 0.26 0.58 v/c Ratio Ver Ratio 0.84 0.71 Control Delay 34.4 Ver Ratio 0.84 0.71 Control Delay 34.4 Optowile Green (s) 18.5 18.5 40.5 40.5 Obl Wile Green (s) 18.5 18.5 40.5 40.5 Oth Wile Green (s) 18.5 18.5 40.5 40.5 Oth Wile Green (s) 18.5 18.5 40.5 40.5 Oth Wile Green (s) 18.5	All-Red Time (s)	2.3	2.3					2.3	2.3				
Los min reput (b) Los S.5 Lead Lag Optimize?	Lost Time Adjust (s)	2.0	0.0					2.0	0.0				
Number Cost Cost Lead/Lag Optimize? Vehicle Extension (s) 3.0 3.0 3.0 Recall Mode None C-Max C-Max Walk Time (s) 7.0 7.0 7.0 Flash Dont Walk (s) 11.0 11.0 11.0 Pedestrian Calls (#/hr) 0 0 0 Actuated g/C Ratio 0.26 0.58 vic Ratio 0.84 0.71 Control Delay 34.4 7.7 Queue Delay 50.0 0.0 Total Delay 84.4 7.7 LOS F A Approach Delay 84.4 7.7 Approach Delay 84.4 7.7 Approach LOS F A S0th %ile Green (s) 18.5 18.5 40.5 S0th %ile Green (s) 18.5 18.5 40.5 S0th %ile Term Code Max Max Coord S0th %ile Term Code Max Max Coord S0th %il	Total Lost Time (s)		5.5						5.5				
Lead-Lag Optimize? Vehicle Extension (s) 3.0 3.0 3.0 3.0 Racall Mode None C-Max C-Max Walk Time (s) 7.0 7.0 7.0 Flash Dont Walk (s) 11.0 11.0 11.0 Pedestrian Calls (#hr) 0 0 0 Actitated giC Ratio 0.26 0.58 vic Ratio 0.84 0.71 Control Delay 34.4 7.7 Queue Delay 50.0 0.0 Total Delay 84.4 7.7 Approach Delay 84.4 7.7 Approach Delay 84.4 7.7 Approach Delay 84.4 7.7 Approach Delay 84.4 7.7 Approach Delay 84.4 7.7 Approach Delay 84.4 7.7 Approach Delay 84.4 7.7 Approach Delay 85.5 40.5 40.5 Vith Wiel Green (s) 18.5 18.5 40.5 <			0.0						0.0				
Lear-Lag Opinitie: 3.0 3.0 3.0 3.0 Recall Mode None None C-Max C-Max Walk Time (s) 7.0 7.0 7.0 F Flash Dort Walk (s) 11.0 11.0 11.0 11.0 Peterbanc Calls (#hr) 0 0 0 0 Act Effect Green (s) 18.1 40.9 Actuated g/C Ratio 0.26 0.55 Vic Ratio 0.26 0.53 Vic Ratio 0.0 0 0 Control Delay 34.4 7.7 Coluceue Delay 0.0 0.0 0 0 Control Delay 84.4 7.7 Approach Delay 84.4 7.7 Approach Delay 84.4 7.7 Obly %ile Green (s) 18.5 18.5 40.5 40.5 90 <td>Lead/Lay</td> <td></td>	Lead/Lay												
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Nature Note Onle Onle Walk Time (s) 7.0 7.0 7.0 Flash Dont Walk (s) 11.0 11.0 11.0 11.0 Pedestrian Calls (#hr) 0 0 0 0 Act Effet Green (s) 18.1 40.9 Actuated g/C Ratio 0.26 0.58 v/c Ratio 0.84 0.71 Control Delay 34.4 7.7 Queue Delay 50.0 0.0 Total Delay 84.4 7.7 LOS F A Approach Delay 84.4 7.7 Approach Delay 84.4 7.7 Approach LOS F A Oth %ile Green (s) 18.5 18.5 40.5 Oth %ile Green (s) 18.5 18.5 40.5 40.5 Oth %ile Green (s) 18.5 18.5 40.5 40.5 Oth %ile Green (s) 18.5 18.5 40.5 40.5 Oth %ile Green (s) 18.5 <	Pocall Mode	J.U Nono	J.U Nono					C Max	C Max				
Wath line (s) 1.0 1.0 1.0 Pedestrian Calls (#hr) 0 0 0 Act Lafted Green (s) 18.1 40.9 Actuated g/C Ratio 0.26 0.58 v/c Ratio 0.84 0.71 Control Delay 34.4 7.7 Queue Delay 50.0 0.0 Total Delay 84.4 7.7 LOS F A Approach Delay 84.4 7.7 Approach LOS F A 90th %lie Green (s) 18.5 18.5 40.5 70th %lie Green (s) 18.5 18.5 40.5													
riash Dolft Walk (s) 11.0 11.0 11.0 Predestrian Calls (#/hr) 0 0 0 Actuated g/C Ratio 0.26 0.58 v/c Ratio 0.84 0.71 Control Delay 34.4 7.7 Queue Delay 50.0 0.0 Total Delay 84.4 7.7 Queue Delay 84.4 7.7 Approach Delay 84.4 7.7 Approach LOS F A Approach LOS F A 90th %ile Green (s) 18.5 18.5 40.5 90th %ile Green (s) 16.6 6.2 2.0 90th %ile Green (s) 16.6 6.2 2.0 <td< td=""><td>Walk Tille (S)</td><td>11.0</td><td>11.0</td><td></td><td></td><td></td><td></td><td>11.0</td><td>11.0</td><td></td><td></td><td></td><td></td></td<>	Walk Tille (S)	11.0	11.0					11.0	11.0				
Pedestrain Calls (#/if) 0 0 0 Act Effet Green (s) 18.1 40.9 Actuated g/C Ratio 0.26 0.58 v/c Ratio 0.84 0.71 Control Delay 34.4 7.7 Queue Delay 50.0 0.0 Total Delay 84.4 7.7 LOS F A Approach Delay 84.4 7.7 Approach DoS F A 90th %ile Green (s) 18.5 18.5 40.5 90th %ile Green (s) 18.5 18.5 40.5 90th %ile Green (s) 18.5 18.5 40.5 70th %ile Term Code Max Max Coord Coord 50th %ile Green (s) 18.5 18.5 40.5 40.5 50th %ile Term Code Max Max Coord Coord Coord 50th %ile Term Code Max Max Coord Coord Coord S0th %ile Term Code Max Max Coord Coord S0th %ile Term Code Max Max Coord Coord S0th %	Plash Done Walk (S)	11.0	11.0					11.0	11.0				
Actuated g/C Ratio 0.26 0.58 vic Ratio 0.84 0.71 Control Delay 34.4 7.7 Queue Delay 50.0 0.0 Total Delay 84.4 7.7 LOS F A Approach Lolsy 84.4 7.7 Jointh Sile Term Code Max Max Max Coord Coord 90th %ile Green (s) 18.5 18.5 40.5 90th %ile Green (s) 16.6 16.6 42.4 42.4 10th %ile Green (s) 16.6 16.6 628		0	10.1					0	40.0				
Actuated glC Ratio 0.26 0.58 v/c Ratio 0.84 0.71 Control Delay 34.4 7.7 Queue Delay 50.0 0.0 Total Delay 84.4 7.7 LOS F A Approach LOS F A 90th %ile Green (s) 18.5 18.5 40.5 90th %ile Green (s) 16.6 628 628 Fuel Used(gal) 10 15	Act Effect Green (S)		10.1						40.9				
Wich Ratio 0.84 0.71 Control Delay 34.4 7.7 Queue Delay 50.0 0.0 Total Delay 84.4 7.7 LOS F A Approach Delay 84.4 7.7 Approach Delay 84.4 7.7 Approach LOS F A 90th %ile Green (s) 18.5 18.5 40.5 90th %ile Green (s) 18.5 18.5 40.5 40.5 90th %ile Green (s) 18.5 18.5 40.5 40.5 70th %ile Green (s) 18.5 18.5 40.5 40.5 50th %ile Green (s) 18.5 18.5 40.5 40.5 50th %ile Green (s) 18.5 18.5 40.5 40.5 30th %ile Green (s) 18.5 18.5 40.5 40.5 30th %ile Green (s) 16.6 16.6 42.4 42.4 10th %ile Green (s) 16.6 16.6 628 Fuel Used(gal) 10 1	Actuated g/C Ratio		0.26						0.58				
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Cueue Delay 50.0 0.0 Total Delay 84.4 7.7 COS F A Approach Delay 84.4 7.7 Approach LOS F A 90th %ile Green (s) 18.5 18.5 40.5 70th %ile Green (s) 18.5 18.5 40.5 70th %ile Green (s) 18.5 18.5 40.5 50th %ile Green (s) 18.5 18.5 40.5 50th %ile Green (s) 18.5 18.5 40.5 30th %ile Green (s) 18.5 18.5 40.5 30th %ile Green (s) 16.6 16.6 42.4 10th %ile Green (s) 16.6 16.6 628 Fuel Used(gal) 10 15 CO CO CO Emissions (g/hr) 701 1030 Cord Cord <td>Control Delay</td> <td></td> <td>34.4</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>1.1</td> <td></td> <td></td> <td></td> <td></td>	Control Delay		34.4						1.1				
Total Delay 84.4 7.7 LOS F A Approach Delay 84.4 7.7 Approach LOS F A 90th %ile Green (s) 18.5 18.5 40.5 40.5 90th %ile Green (s) 18.5 18.5 40.5 40.5 90th %ile Green (s) 18.5 18.5 40.5 40.5 70th %ile Green (s) 18.5 18.5 40.5 40.5 70th %ile Green (s) 18.5 18.5 40.5 40.5 50th %ile Green (s) 18.5 18.5 40.5 40.5 30th %ile Green (s) 18.5 18.5 40.5 40.5 30th %ile Green (s) 16.6 16.6 42.4 42.4 10th %ile Green (s) 16.6 16.6 42.4 42.4 10th %ile Green (s) 16.6 16.6 628 Fuel Used(gal) 10 15 Coord Coord 10th %ile Green (s) 16.6 200 VOC Emissions (g/hr) 162 239 Dilemma Vehicles (#) 0 0 0	Queue Delay		50.0						0.0				
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70th %ile Green (s) 18.5 18.5 40.5 40.5 70th %ile Term Code Max Max Coord Coord 50th %ile Green (s) 18.5 18.5 40.5 40.5 50th %ile Green (s) 18.5 18.5 40.5 40.5 30th %ile Green (s) 18.5 18.5 40.5 40.5 30th %ile Green (s) 18.5 18.5 40.5 40.5 30th %ile Green (s) 16.6 16.6 42.4 42.4 10th %ile Green (s) 16.6 16.6 42.4 42.4 10th %ile Green (s) 16.6 16.6 628 Fuel Used(gal) 10 15 666 628 Fuel Used(gal) 10 1030 1030 1030 NOx Emissions (g/hr) 162 239 200 200 200 VOC Emissions (g/hr) 162 239 239 201 201 202 Queue Length 50th (ft) 175 82 202 202 2024 245 Internal Link Dist (ft) 164 434 798	90th %ile Term Code	Max	Max					Coord	Coord				
70th %ile Term Code Max Max Coord Coord 50th %ile Green (s) 18.5 18.5 40.5 40.5 50th %ile Green (s) 18.5 18.5 40.5 40.5 30th %ile Green (s) 18.5 18.5 40.5 40.5 30th %ile Green (s) 18.5 18.5 40.5 40.5 30th %ile Green (s) 16.6 16.6 42.4 42.4 10th %ile Green (s) 16.6 16.6 42.4 42.4 10th %ile Term Code Gap Gap Goord Coord Stops (vph) 666 628 Euel Used(gal) 10 15 CO Emissions (g/hr) 701 1030 NOx Emissions (g/hr) 162 239 Dilemma Vehicles (#) 0 0 0 0 0 Queue Length 50th (ft) 175 82 0 0 1 Internal Link Dist (ft) 164 434 798 445 Turn Bay Length (ft) 82 2024 <	70th %ile Green (s)	18.5	18.5					40.5	40.5				
50th %ile Green (s) 18.5 18.5 40.5 40.5 50th %ile Term Code Max Max Coord Coord 30th %ile Green (s) 18.5 18.5 40.5 40.5 30th %ile Green (s) 18.5 18.5 40.5 40.5 30th %ile Green (s) 16.6 16.6 42.4 42.4 10th %ile Green (s) 16.6 16.6 42.4 42.4 10th %ile Term Code Gap Gap Goord Coord Stops (vph) 666 628 628 Fuel Used(gal) 10 15 CO Emissions (g/hr) 701 NOx Emissions (g/hr) 701 1030 NOx Emissions (g/hr) 162 239 Dilemma Vehicles (#) 0 0 0 0 0 Queue Length 50th (ft) 175 82 0 0 0 Queue Length 95th (ft) 164 434 798 445 445 Turn Bay Length (ft) 164 434 798 445 Turn Bay Length (ft) 2024 2024 2024 <td>70th %ile Term Code</td> <td>Max</td> <td>Max</td> <td></td> <td></td> <td></td> <td></td> <td>Coord</td> <td>Coord</td> <td></td> <td></td> <td></td> <td></td>	70th %ile Term Code	Max	Max					Coord	Coord				
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30th %ile Green (s) 18.5 18.5 40.5 40.5 30th %ile Term Code Max Max Coord Coord 10th %ile Green (s) 16.6 16.6 42.4 42.4 10th %ile Term Code Gap Gap Coord Coord Stops (vph) 666 628 Fuel Used(gal) 10 15 CO Emissions (g/hr) 701 1030 NOx Emissions (g/hr) 162 239 Dilemma Vehicles (#) 0 0 Queue Length 50th (ft) 175 82 Queue Length 95th (ft) #257 98 Internal Link Dist (ft) 164 434 798 445 Turn Bay Length (ft) 827 2024 204	50th %ile Term Code	Max	Max					Coord	Coord				
30th %ile Term Code Max Max Coord Coord 10th %ile Green (s) 16.6 16.6 42.4 42.4 10th %ile Green (s) 16.6 16.6 42.4 42.4 10th %ile Term Code Gap Gap Coord Coord Stops (vph) 666 628 628 Fuel Used(gal) 10 15 CO CO Emissions (g/hr) 701 1030 0 NOx Emissions (g/hr) 162 239 0 VOC Emissions (g/hr) 162 239 0 Dilemma Vehicles (#) 0 0 0 Queue Length 50th (ft) 175 82 0 Queue Length 95th (ft) #257 98 445 Turn Bay Length (ft) 164 434 798 445 Turn Bay Length (ft) 927 2024 2024	30th %ile Green (s)	18.5	18.5					40.5	40.5				
10th %ile Green (s) 16.6 16.6 42.4 42.4 10th %ile Term Code Gap Gap Coord Coord Stops (vph) 666 628 Fuel Used(gal) 10 15 CO Emissions (g/hr) 701 1030 NOx Emissions (g/hr) 136 200 VOC Emissions (g/hr) 162 239 Dilemma Vehicles (#) 0 0 Queue Length 50th (ft) 175 82 Queue Length 95th (ft) #257 98 Internal Link Dist (ft) 164 434 798 445 Turn Bay Length (ft) 827 2024 2024	30th %ile Term Code	Max	Max					Coord	Coord				
10th %ile Term Code Gap Gap Coord Coord Stops (vph) 666 628 Fuel Used(gal) 10 15 CO Emissions (g/hr) 701 1030 NOx Emissions (g/hr) 136 200 VOC Emissions (g/hr) 162 239 Dilemma Vehicles (#) 0 0 Queue Length 50th (ft) 175 82 Queue Length 95th (ft) #257 98 Internal Link Dist (ft) 164 434 798 445 Turn Bay Length (ft) 927 2024 2024	10th %ile Green (s)	16.6	16.6					42.4	42.4				
Stops (vph) 666 628 Fuel Used(gal) 10 15 CO Emissions (g/hr) 701 1030 NOx Emissions (g/hr) 136 200 VOC Emissions (g/hr) 162 239 Dilemma Vehicles (#) 0 0 Queue Length 50th (ft) 175 82 Queue Length 95th (ft) #257 98 Internal Link Dist (ft) 164 434 798 445 Turn Bay Length (ft) 927 2024 45	10th %ile Term Code	Gap	Gap					Coord	Coord				
Fuel Used(gal) 10 15 CO Emissions (g/hr) 701 1030 NOx Emissions (g/hr) 136 200 VOC Emissions (g/hr) 162 239 Dilemma Vehicles (#) 0 0 Queue Length 50th (ft) 175 82 Queue Length 95th (ft) #257 98 Internal Link Dist (ft) 164 434 798 445 Turn Bay Length (ft) 927 2024 2024 Starvation Can Reduction 262 0 0	Stops (vph)		666						628				
CO Emissions (g/hr) 701 1030 NOx Emissions (g/hr) 136 200 VOC Emissions (g/hr) 162 239 Dilemma Vehicles (#) 0 0 Queue Length 50th (ft) 175 82 Queue Length 95th (ft) #257 98 Internal Link Dist (ft) 164 434 798 445 Turn Bay Length (ft) 927 2024 2024 Starvation Can Reduction 262 0 0	Fuel Used(gal)		10						15				
NOx Emissions (g/hr) 136 200 VOC Emissions (g/hr) 162 239 Dilemma Vehicles (#) 0 0 Queue Length 50th (ft) 175 82 Queue Length 95th (ft) #257 98 Internal Link Dist (ft) 164 434 798 445 Turn Bay Length (ft) 827 2024 2024 Starvation Can Reduction 262 0 0	CO Emissions (g/hr)		701						1030				
VOC Emissions (g/hr) 162 239 Dilemma Vehicles (#) 0 0 Queue Length 50th (ft) 175 82 Queue Length 95th (ft) #257 98 Internal Link Dist (ft) 164 434 798 445 Turn Bay Length (ft) 82 2024 2024 Starvation Cap Reduction 262 0 0	NOx Emissions (g/hr)		136						200				
Dilemma Vehicles (#) 0 0 Queue Length 50th (ft) 175 82 Queue Length 95th (ft) #257 98 Internal Link Dist (ft) 164 434 798 445 Turn Bay Length (ft) 927 2024 100 Starvation Cap Reduction 262 0 0	VOC Emissions (g/hr)		162						239				
Queue Length 50th (ft) 175 82 Queue Length 95th (ft) #257 98 Internal Link Dist (ft) 164 434 798 445 Turn Bay Length (ft) 82 2024 100 Base Capacity (vph) 927 2024 100	Dilemma Vehicles (#)		0						0				
Queue Length 95th (ft) #257 98 Internal Link Dist (ft) 164 434 798 445 Turn Bay Length (ft) 164 2024 164 2024 Starvation Cap Reduction 262 0 0	Queue Length 50th (ft)		175						82				
Internal Link Dist (ft) 164 434 798 445 Turn Bay Length (ft) Base Capacity (vph) 927 2024 Starvation Cap Reductr 262 0	Queue Length 95th (ft)		#257						98				
Turn Bay Length (ft) 927 2024 Starvation Cap Reduction 262 0	Internal Link Dist (ft)		164			434			798			445	
Base Capacity (vph) 927 2024 Starvation Cap Reductin 262 0	Turn Bay Length (ft)												
Starvation Can Reductin 262 0	Base Capacity (vph)		927						2024				
	Starvation Cap Reducto		262										

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings
6: N East St/N Main St & W Locust St/E Locust St

02/22/2024	
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Spillback Cap Reductn		0						0				
Storage Cap Reductn		0						0				
Reduced v/c Ratio		1.15						0.71				
Intersection Summary												
Area Type: C	Other											
Cycle Length: 70												
Actuated Cycle Length: 70												
Offset: 10 (14%), Referenced	I to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 60												
Control Type: Actuated-Coord	dinated											
Maximum v/c Ratio: 0.84												
Intersection Signal Delay: 34.	.3			In	tersectior	LOS: C						
Intersection Capacity Utilizati	on 67.1%			IC	U Level o	of Service	С					
Analysis Period (min) 15												
# 95th percentile volume ex	ceeds ca	oacity, qu	eue may	be longer								
Queue shown is maximum	n after two	cycles.										
Splits and Phases: 6: N Ea	ist St/N Ma	ain St & V	/ Locust S	St/E Locu	st St							

Ø2 (R)	A 04	3	
46 s	24 s		

Lanes, Volumes, Timings 9: N Madison St & W Market St

Lane Group EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR Lane Configurations 1
Lane Configurations ↑ ↓
Traffic Volume (vph) 0 125 85 20 150 0 0 0 10 1025 160 Future Volume (vph) 0 125 85 20 150 0 0 0 10 1025 160 Ideal Flow (vphpl) 1900 19
Future Volume (vph) 0 125 85 20 150 0 0 0 10 1025 160 Ideal Flow (vphpl) 1900
Ideal Flow (vphpl) 1900
Lane Width (ft) 12 11 12 11 12
Grade (%) 1% -2% -6% 4% Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 1.00 0.95 0.95 0.95 Ped Bike Factor 0.99 1.00 1.00 1.00 1.00 1.00 1.00
Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.95 0.95 0.95 Ped Bike Factor 0.99 1.00
Ped Bike Factor 0.99 1.00 1.00
Frt 0.946 0.980
Flt Protected 0.950
Satd. Flow (prot) 0 1517 0 1609 1637 0 0 0 0 3050 0
Flt Permitted 0.494
Satd. Flow (perm) 0 1517 0 835 1637 0 0 0 0 3050 0
Right Turn on Red Yes Yes Yes Yes Yes
Satd. Flow (RTOR) 46 43
Link Speed (mph) 30 30 30 30
Link Distance (ft) 705 247 400 860
Travel Time (s) 16.0 5.6 9.1 19.5
Confl. Peds. (#/hr) 1 1 4
Peak Hour Factor 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94
Parking (#/hr) 20 20
Adj. Flow (vph) 0 133 90 21 160 0 0 0 0 11 1090 170
Shared Lane Traffic (%)
Lane Group Flow (vph) 0 223 0 21 160 0 0 0 0 0 1271 0
Enter Blocked Intersection No No No No No No No No No No No No
Lane Alignment Left Left Right Left Left Right Left Right Left Left Left Left Right
Median Width(ft) 12 12 0 0
Link Offset(ft) 0 0 0 0
Crosswalk Width(ft) 16 16 16 16
Two way Left Turn Lane Yes
Headway Factor 1.15 1.20 1.15 1.13 1.18 1.13 1.10 1.10 1.10 1.17 1.17 1.17
Turning Speed (mph) 15 9 15 9 15 9 15 9
Number of Detectors 2 1 2 1 2
Detector Template Thru Left Thru Left Thru Left Thru
Leading Detector (ft) 100 20 100 20 100
Trailing Detector (ft) 0 0 0 0 0
Detector 1 Position(ft) 0 0 0 0 0
Detector 1 Size(ft) 6 20 6 20 6
Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex
Detector 1 Channel
Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0
Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0
Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0
Detector 2 Position(ft) 94 94 94
Detector 2 Size(ft) 6 6
Detector 2 Type CI+Ex CI+Ex CI+Ex
Detector 2 Channel
Detector 2 Extend (s) 0.0 0.0 0.0
Turn Type NA Perm NA Perm NA

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 9: N Madison St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		17.4		17.4	17.4					45.2	45.2	
Total Split (s)		23.0		23.0	23.0					47.0	47.0	
Total Split (%)		32.9%		32.9%	32.9%					67.1%	67.1%	
Maximum Green (s)		17.6		17.6	17.6					41.8	41.8	
Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)		2.2		2.2	2.2					2.0	2.0	
Lost Time Adjust (s)		0.0		0.0	0.0						0.0	
Total Lost Time (s)		5.4		5.4	5.4						5.2	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		None	None					C-Max	C-Max	
Walk Time (s)		1.0		1.0	1.0					29.0	29.0	
Flash Dont Walk (s)		11.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		13.1		13.1	13.1						46.3	
Actuated g/C Ratio		0.19		0.19	0.19						0.66	
v/c Ratio		0.69		0.13	0.52						0.63	
Control Delay		32.0		23.6	30.9						1.5	
Queue Delay		0.0		0.0	0.0						0.0	
Total Delay		32.0		23.6	30.9						1.5	
LOS		С		С	С						А	
Approach Delay		32.0			30.1						1.5	
Approach LOS		С			С						А	
90th %ile Green (s)		17.6		17.6	17.6					41.8	41.8	
90th %ile Term Code		Max		Hold	Hold					Coord	Coord	
70th %ile Green (s)		16.1		16.1	16.1					43.3	43.3	
70th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
50th %ile Green (s)		13.6		13.6	13.6					45.8	45.8	
50th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
30th %ile Green (s)		11.0		11.0	11.0					48.4	48.4	
30th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
10th %ile Green (s)		7.3		7.3	7.3					52.1	52.1	
10th %ile Term Code		Gap		Gap	Gap					Coord	Coord	
Stops (vph)		151		19	128						48	
Fuel Used(gal)		3		0	2						9	
CO Emissions (g/hr)		235		17	136						605	
NOx Emissions (g/hr)		46		3	26						118	
VOC Emissions (g/hr)		54		4	31						140	
Dilemma Vehicles (#)		0		0	0						0	
Queue Length 50th (ft)		71		8	62						12	
Queue Length 95th (ft)		130		24	108						13	
Internal Link Dist (ft)		625			167			320			780	
Turn Bay Length (ft)												

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 9: N Madison St & W Market St

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	1.0		•			20	1	22.03	1	0.50	•	100
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Base Capacity (vph)		415		209	411						2031	
Starvation Cap Reductn		0		0	0						0	
Spillback Cap Reductn		0		0	0						0	
Storage Cap Reductn		0		0	0						0	
Reduced v/c Ratio		0.54		0.10	0.39						0.63	
Intersection Summary												
Area Type:	CBD											
Cycle Length: 70												
Actuated Cycle Length: 7	0											
Offset: 64 (91%), Referen	ced to phase	2: and 6:	SBTL, Sta	art of Gre	en							
Natural Cycle: 65												
Control Type: Actuated-C	oordinated											
Maximum v/c Ratio: 0.69												
Intersection Signal Delay:	8.7			In	tersectior	n LOS: A						
Intersection Capacity Utili	zation 64.8%			IC	U Level	of Service	С					
Analysis Period (min) 15												

Splits and Phases: 9: N Madison St & W Market St

	→ Ø4	
	23 s	
Ø6 (R)	₹Ø8	
47 s	23 s	

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	1	LDR	HUL	<u>.</u>		
Traffic Volume (vph)	155	0	135	1070	0	0
Future Volume (vph)	155	0	135	1070	0	0
Ideal Flow (vphpl)	1000	1000	1000	10/0	1000	1000
Lane Width (ft)	1300	1300	1200	1300	1300	1300
Grade (%)	10	12	12	2 0/	20/	12
	1.00	1.00	0.05	-2%	2 %	1.00
Lane Ulli. Facilui Dod Piko Eastor	1.00	1.00	0.95	1.00	1.00	1.00
	0.90			1.00		
Fft Fit Deata at a d	0.050			0.004		
Fit Protected	0.950	0	•	0.994	0	0
Satd. Flow (prot)	1481	0	0	2782	0	0
FIt Permitted	0.950			0.994		
Satd. Flow (perm)	1447	0	0	2782	0	0
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)						
Link Speed (mph)	30			30	30	
Link Distance (ft)	263			127	878	
Travel Time (s)	60			2.9	20.0	
Confl Peds (#/hr)	11		2	2.0	20.0	
Peak Hour Factor	0.04	0 01	ے 0 01	0.04	0.04	0.04
Parking (#/hr)	0.94	0.94	0.94	0.94	0.94	0.94
Parking (#/nr)	105	0	20	20	0	0
Adj. Flow (vpn)	165	0	144	1138	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	165	0	0	1282	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	13			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Eactor	1 25	1 1/	1 13	1 3/	1 16	1 16
Turning Speed (mph)	1.25	1.14	1.15	1.04	1.10	1.10
Number of Detectors	CI k	9	10	0		9
Number of Detectors	1		1			
Detector Template	Lett		Left	Inru		
Leading Detector (ft)	20		20	100		
Trailing Detector (ft)	0		0	0		
Detector 1 Position(ft)	0		0	0		
Detector 1 Size(ft)	20		20	6		
Detector 1 Type	CI+Ex		CI+Ex	CI+Ex		
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0		
Detector 1 Queue (s)	0.0		0.0	0.0		
Detector 1 Delay (s)	0.0		0.0	0.0		
Detector 2 Position(ft)	0.0		0.0	0.0		
Detector 2 Fusition(II)				54		
Detector 2 Type				CI+EX		
Detector 2 Channel						
Detector 2 Extend (s)				0.0		
Turn Type	Prot		Perm	NA		

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Protected Phases	4			2		
Permitted Phases	т		2	2		
Detector Phase	1		2	2		
Switch Phase	4		2	2		
Minimum Initial (a)	E 0		ΕO	E 0		
Minimum miliar (S)	0.U		0.C	5.U		
Minimum Split (s)	15.4		47.5	47.5		
Total Split (s)	21.0		49.0	49.0		
Total Split (%)	30.0%		70.0%	70.0%		
Maximum Green (s)	15.6		43.5	43.5		
Yellow Time (s)	3.2		3.2	3.2		
All-Red Time (s)	2.2		2.3	2.3		
Lost Time Adjust (s)	0.0			0.0		
Total Lost Time (s)	5.4			5.5		
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0		3.0	3.0		
Recall Mode	None		C-Max	C-Max		
Walk Time (s)	1.0		31.0	31.0		
Flash Dont Walk (s)	9.0		11.0	11.0		
Pedestrian Calls (#/hr)	0.0		0	0		
Act Effet Green (s)	12 4		U	46 7		
Actuated a/C Ratio	0.18			0.67		
v/c Ratio	0.10			0.07		
Control Dolov	27.1			10.03		
	37.1			10.4		
Total Dalay	0.0			10.4		
	37.1			10.4		
	D			B		
Approach Delay	37.1			10.4		
Approach LOS	D		16 -	B		
90th %ile Green (s)	15.6		43.5	43.5		
90th %ile Term Code	Max		Coord	Coord		
70th %ile Green (s)	15.1		44.0	44.0		
70th %ile Term Code	Gap		Coord	Coord		
50th %ile Green (s)	13.0		46.1	46.1		
50th %ile Term Code	Gap		Coord	Coord		
30th %ile Green (s)	10.8		48.3	48.3		
30th %ile Term Code	Gap		Coord	Coord		
10th %ile Green (s)	7.6		51.5	51.5		
10th %ile Term Code	Gap		Coord	Coord		
Stops (vph)	137			717		
Fuel Lised(gal)	2			8		
CO Emissions (a/hr)	157			530		
NOv Emissions (g/III)	31			105		
NOX Emissions (g/m)	31			105		
VUC Emissions (g/nr)	30			125		
	0			0		
Queue Length 50th (ft)	66			156		
Queue Length 95th (ft)	119			256		
Internal Link Dist (ft)	183			47	798	
Turn Bay Length (ft)						

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Base Capacity (vph)	330			1855			
Starvation Cap Reductn	0			0			
Spillback Cap Reductn	0			0			
Storage Cap Reductn	0			0			
Reduced v/c Ratio	0.50			0.69			
Intersection Summary							
Area Type:	CBD						
Cycle Length: 70							
Actuated Cycle Length: 70)						
Offset: 64 (91%), Referen	ced to phase	2:NBTL a	nd 6:, Sta	art of Gree	en		
Natural Cycle: 65							
Control Type: Actuated-C	oordinated						
Maximum v/c Ratio: 0.69							
Intersection Signal Delay:	13.4			Int	tersectior	n LOS: B	
Intersection Capacity Utiliz	zation 55.8%			IC	U Level o	of Service B	
Analysis Period (min) 15							

Splits and Phases: 11: N East St & E Market St

	ر (181 – 182		
49 s		21 s	

Lanes, Volumes, Timings 12: N Madison St & W Monroe St

Lane Group EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT Lane Configurations Image: Configuration in the configuratin the configuration in the configuration in the conf	SBR 7 1900 12 0.95
Lane Configurations Image: configuration in the image: configuratine in the image: configuration in the image: configuration in th	7 7 1900 12 0.95
Traffic Volume (vph)01003528000031168Future Volume (vph)01003528000031168Ideal Flow (vphpl)190019001900190019001900190019001900190019001900Lane Width (ft)121112121212121212121212	7 1900 12 0.95
Future Volume (vph)01003528000031168Ideal Flow (vphpl)190019001900190019001900190019001900190019001900Lane Width (ft)12111212121212121212121212	7 1900 12 0.95
Ideal Flow (vphpl)19001900190019001900190019001900190019001900Lane Width (ft)12111212121212121212121212	1900 12 0.95
Lane Width (ft) 12 11 12 12 12 12 12 12 12 12 12 12 12	12 0.95
	0.95
Grade (%) 0% -3% 6%	0.95
Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0	
Ped Bike Factor	
Frt 0.999	
Flt Protected 0.973	
Satd. Flow (prot) 0 1621 0 0 1631 0 0 0 0 0 3087	0
Flt Permitted 0.973	
Satd. Flow (perm) 0 1621 0 0 1631 0 0 0 0 0 3087	0
Link Speed (mph) 30 30 30 30	
Link Distance (ft) 699 209 307 400	
Travel Time (s) 15.9 4.8 7.0 9.1	
Confl. Peds. (#/hr) 4	2
Peak Hour Factor 0.89 0.89 0.89 0.89 0.89 0.89 0.89 0.89	0.89
Adj. Flow (vph) 0 11 0 39 31 0 0 0 3 1312	8
Shared Lane Traffic (%)	
Lane Group Flow (vph) 0 11 0 0 70 0 0 0 1323	0
Enter Blocked Intersection No No No No No No No No No No No	No
Lane Alignment Left Left Right Left Left Right Left Right Left Left Left Left Left Left Left Lef	Right
Median Width(ft) 0 0 0 0	
Link Offset(ft) 0 0 0 0	
Crosswalk Width(ft) 16 16 16 16	
Two way Left Turn Lane	
Headway Factor 1.14 1.19 1.14 1.14 1.14 1.14 1.12 1.12 1.12 1.19 1.19	1.19
Turning Speed (mph) 15 9 15 9 15 9 15	9
Sign Control Stop Stop Free	
Intersection Summary	
Area Type: CBD	
Control Type: Unsignalized	

Intersection Capacity Utilization 53.3%

ICU Level of Service A

Analysis Period (min) 15

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	3		HUE	41		
Traffic Volume (voh)	45	0	30	1110	0	0
Future Volume (vph)	45	0	30	1110	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	11	12	12
Grade (%)	-2%	12	12	_3%	4%	12
Lane I Itil Factor	1 00	1.00	0.95	0.95	1 00	1 00
Pod Piko Eastor	0.00	1.00	0.95	1.00	1.00	1.00
	0.90			1.00		
Elt Protoctod	0.050			0 000		
Sata Elow (prot)	1110	0	0	0.999	0	0
Salu. Flow (prot)	1440	0	0	2010	0	0
	0.950	0	0	0.999	^	0
Sata. Flow (perm)	1426	0	0	2809	0	0
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)						
Link Speed (mph)	30			30	30	
Link Distance (ft)	221			318	266	
Travel Time (s)	5.0			7.2	6.0	
Confl. Peds. (#/hr)	5		7			
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Parking (#/hr)	0		20	20		
Adj. Flow (vph)	46	0	31	1144	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	46	0	0	1175	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12	, agint	Lon	0	0	
Link Offset/ft)	0			0	0	
Crosswalk Width/ft)	16			16	16	
Two way Left Turn Long	10			10	10	
Hoodway Easter	1.00	1 1 2	1 10	1 24	1 17	1 17
	1.29	1.13	1.12	1.54	1.17	1.17
Turning Speed (mpn)	15	9	15	^		9
Number of Detectors	1		1	- 2		
Detector Template	Left		Left	Ihru		
Leading Detector (ft)	20		20	100		
Trailing Detector (ft)	0		0	0		
Detector 1 Position(ft)	0		0	0		
Detector 1 Size(ft)	20		20	6		
Detector 1 Type	Cl+Ex		Cl+Ex	CI+Ex		
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0		
Detector 1 Queue (s)	0.0		0.0	0.0		
Detector 1 Delay (s)	0.0		0.0	0.0		
Detector 2 Position(ft)	0.0		0.0	94		
Detector 2 Size(ft)				6		
Detector 2 Type						
Detector 2 Channel				OFFLA		
				0.0		
	Dural		Demo	0.0		
iurn iype	Prot		Perm	NA		

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

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Lane Group	FBL	EBR NBL	NBT	SBT	SBR	
Protected Phases	4		2			
Permitted Phases	•	2	2			
Detector Phase	4	2	2			
Switch Phase	•	<u> </u>	2			
Minimum Initial (s)	5.0	5.0	50			
Minimum Solit (s)	13.2	49.2	<u>49</u> 2			
Total Split (s)	17.0	43.2 63.0	63.0			
Total Split (%)	21.3%	78.8%	78.8%			
Maximum Green (s)	11.8	57.8	57.8			
Vellow Time (s)	3.2	3.0	3.2			
All-Red Time (s)	2.0	2.0	2.0			
Lost Time Adjust (s)	0.0	2.0	2.0			
Total Lost Time (s)	5.2		5.2			
	5.2		J.Z			
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3 0	30			
Pecall Mode	J.U Nono	0.0 C Mov	0.0 C Mov			
	10	0-IVIAX				
Valk Time (S)	1.0	33.0	33.0			
Prasti Dorit Walk (S)	7.0	11.0	11.0			
Pedestrian Calls (#/nr)	0	0	0			
Act Effect Green (S)	0.0		00.1			
Actuated g/C Ratio	0.10		0.85			
V/C Ratio	0.32		0.49			
Control Delay	38.4		1.0			
Queue Delay	0.0		0.0			
Total Delay	38.4		1.0			
LUS Annan anh Dalau	D		A			
Approach Delay	38.4		1.0			
Approach LOS	D	50.0	A			
90th %ile Green (s)	11.0	58.6	58.6			
South Wile Lerm Code	Gap	Coord	Coord			
/Utn %ile Green (s)	9.2	60.4	60.4			
70th %ile Term Code	Gap	Coord	Coord			
SUTI WILE Green (S)	7.9	61.7	61./			
SUID %ILE Term Code	Gap	Coord	Coord			
SUTI %ILE Green (S)	0.0	/4.8	/4.8			
Suth %ile Term Code	Skip	Coord	Coord			
10th %ile Green (s)	0.0	/4.8	/4.8			
10th %ile Term Code	Skip	Coord	Coord			
Stops (vph)	42		33			
Fuel Used(gal)	1		3			
CO Emissions (g/hr)	46		226			
NOx Emissions (g/hr)	9		44			
VOC Emissions (g/hr)	11		52			
Dilemma Vehicles (#)	0		0			
Queue Length 50th (ft)	22		11			
Queue Length 95th (ft)	52		16			
Internal Link Dist (ft)	141		238	186		
Turn Bay Length (ft)						

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Base Capacity (vph)	213			2392			
Starvation Cap Reductn	0			0			
Spillback Cap Reductn	0			0			
Storage Cap Reductn	0			0			
Reduced v/c Ratio	0.22			0.49			
Intersection Summary							
Area Type:	CBD						
Cycle Length: 80							
Actuated Cycle Length: 80)						
Offset: 59 (74%), Referen	ced to phase 2	2:NBTL a	nd 6:, Sta	art of Gre	en		
Natural Cycle: 65							
Control Type: Actuated-Co	oordinated						
Maximum v/c Ratio: 0.49							
Intersection Signal Delay:	2.4			In	tersectior	n LOS: A	
Intersection Capacity Utiliz	zation 47.9%			IC	U Level o	of Service A	
Analysis Period (min) 15							

Splits and Phases: 13: N East St & Monroe St

Ø2 (R)	▶ _{Ø4}		355
63 s	17 s	- 35	

Lanes, Volumes, Timings 15: N Madison St & W Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1.			र्स						ef îr	
Traffic Volume (vph)	0	20	5	15	15	0	0	0	0	15	1155	10
Future Volume (vph)	0	20	5	15	15	0	0	0	0	15	1155	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	16	12	12	14	12	12	12	12	12	12	12
Grade (%)		4%			-3%			2%			2%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		1.00			1.00						1.00	
Frt		0.973									0.999	
Flt Protected					0.976						0.999	
Satd. Flow (prot)	0	1804	0	0	1594	0	0	0	0	0	3147	0
Flt Permitted					0.869						0.999	
Satd. Flow (perm)	0	1804	0	0	1413	0	0	0	0	0	3147	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		6									2	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		692			253			310			307	
Travel Time (s)		15.7			5.8			7.0			7.0	
Confl. Peds. (#/hr)			4	4						2		
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Parking (#/hr)					0					20		20
Adj. Flow (vph)	0	24	6	18	18	0	0	0	0	18	1359	12
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	30	0	0	36	0	0	0	0	0	1389	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.17	1.00	1.17	1.12	1.18	1.12	1.16	1.16	1.16	1.16	1.16	1.16
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		CI+Ex		Cl+Ex	CI+Ex					CI+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		CI+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 15: N Madison St & W Jefferson St

02/22/	2024
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		15.1		15.4	15.4					47.4	47.4	
Total Split (s)		20.0		20.0	20.0					60.0	60.0	
Total Split (%)		25.0%		25.0%	25.0%					75.0%	75.0%	
Maximum Green (s)		14.9		14.6	14.6					54.6	54.6	
Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)		1.9		2.2	2.2					2.2	2.2	
Lost Time Adjust (s)		0.0			0.0						0.0	
Total Lost Time (s)		5.1			5.4						5.4	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		Max		None	None					C-Max	C-Max	
Walk Time (s)		1.0		1.0	1.0					31.0	31.0	
Flash Dont Walk (s)		9.0		9.0	9.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		14.9			14.6						54.6	
Actuated g/C Ratio		0.19			0.18						0.68	
v/c Ratio		0.09			0.14						0.65	
Control Delay		23.8			29.1						9.0	
Queue Delay		0.0			0.0						0.0	
Total Delay		23.8			29.1						9.0	
LOS		С			С						А	
Approach Delay		23.8			29.1						9.0	
Approach LOS		С			С						А	
90th %ile Green (s)		14.9		14.6	14.6					54.6	54.6	
90th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
70th %ile Green (s)		14.9		14.6	14.6					54.6	54.6	
70th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
50th %ile Green (s)		14.9		14.6	14.6					54.6	54.6	
50th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
30th %ile Green (s)		14.9		14.6	14.6					54.6	54.6	
30th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
10th %ile Green (s)		14.9		14.6	14.6					54.6	54.6	
10th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
Stops (vph)		20			28						616	
Fuel Used(gal)		0			0						8	
CO Emissions (g/hr)		26			28						586	
NOx Emissions (g/hr)		5			5						114	
VOC Emissions (g/hr)		6			6						136	
Dilemma Vehicles (#)		0			0						0	
Queue Length 50th (ft)		10			15						176	
Queue Length 95th (ft)		30			38						211	
Internal Link Dist (ft)		612			173			230			227	
Turn Bay Length (ft)												

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 15: N Madison St & W Jefferson St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Base Capacity (vph)		340			257						2148	
Starvation Cap Reductn		0			0						0	
Spillback Cap Reductn		0			0						0	
Storage Cap Reductn		0			0						0	
Reduced v/c Ratio		0.09			0.14						0.65	
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 0 (0%), Referenced	to phase 2:	and 6:SB	TL, Start	of Green								
Natural Cycle: 65												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.65												
Intersection Signal Delay: 9	9.8			In	tersectior	n LOS: A						
Intersection Capacity Utiliz	ation 53.8%			IC	U Level o	of Service	А					
Analysis Period (min) 15												

Splits and Phases: 15: N Madison St & W Jefferson St

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	20 s
Ø6 (R)	₹ø8
60 s	20 s

Lanes, Volumes, Timings 16: N East St & E Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्स			Þ			4 î b				
Traffic Volume (vph)	15	40	0	0	45	35	20	1080	20	0	0	0
Future Volume (vph)	15	40	0	0	45	35	20	1080	20	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	11	11	12	12	12
Grade (%)		-3%			3%			0%			3%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor		0.99			0.97			1.00				
Frt					0.941			0.997				
Flt Protected		0.986						0.999				
Satd. Flow (prot)	0	1510	0	0	1556	0	0	3066	0	0	0	0
Flt Permitted		0.913						0.999				
Satd. Flow (perm)	0	1378	0	0	1556	0	0	3065	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					36			5				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		268			710			308			318	
Travel Time (s)		6.1			16.1			7.0			7.2	
Confl. Peds. (#/hr)	25	•••				25	17		4			
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Parking (#/hr)	0.00	0.00	0.00	0.00	0.00	0.00	20	0.00	20	0.00	0.00	0.00
Adi Flow (vnh)	16	42	0	0	47	36	21	1125	21	0	0	0
Shared Lane Traffic (%)			Ū	· ·	••			1120		Ū	Ŭ	Ū
Lane Group Flow (vph)	0	58	0	0	83	0	0	1167	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)	2011	0	rugitt	2011	0	rugin	Lon	0	ragin	Lon	0	i agin
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		10			10			10			10	
Headway Eactor	1 12	1 28	1 12	1 17	1 12	1 17	1 14	1 19	1 19	1 17	1 17	1 17
Turning Speed (mph)	15	1.20	9	15		9	15		9	15		9
Number of Detectors	.0	2	Ŭ		2	Ū	1	2	Ū			Ū
Detector Template	Left	Thru			Thru		Left	Thru				
Leading Detector (ft)	20	100			100		20	100				
Trailing Detector (ft)	0	0			0		0	0				
Detector 1 Position(ft)	0	0			0		0	0				
Detector 1 Size(ft)	20	6			6		20	6				
Detector 1 Type	CI+Ex	CI+Ex			CI+Ex		CI+Ex	CI+Ex				
Detector 1 Channel	OI · LA	OI LA			OI: EX		OF	OFER				
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0				
Detector 2 Position(ft)	0.0	0.0 Q/			0.0 Q/		0.0	0.0 Q/				
Detector 2 Position(it)		94 6			54			54				
Detector 2 Type												
Detector 2 Channel												
Detector 2 Extend (a)		0.0			0.0			0.0				
	Derm	0.0			0.0		Derm	0.0				
rum rype	Perm	NA			NA		Perm	NA				

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 16: N East St & E Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases		4			8			2				
Permitted Phases	4						2					
Detector Phase	4	4			8		2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0		5.0	5.0				
Minimum Split (s)	15.2	15.2			15.2		47.2	47.2				
Total Split (s)	20.0	20.0			20.0		60.0	60.0				
Total Split (%)	25.0%	25.0%			25.0%		75.0%	75.0%				
Maximum Green (s)	14.8	14.8			14.8		54.8	54.8				
Yellow Time (s)	3.2	3.2			3.2		3.2	3.2				
All-Red Time (s)	2.0	2.0			2.0		2.0	2.0				
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.2			5.0			5.2				
lead/Lag		•			·							
Lead-Lag Optimize?												
Vehicle Extension (s)	30	3.0			30		3.0	30				
Recall Mode	Max	Max			None		C-Max	C-Max				
Walk Time (s)	10	1.0			1.0		31.0	31.0				
Flash Dont Walk (s)	9.0	9.0			9.0		11.0	11.0				
Pedestrian Calls (#/hr)	0.0	0.0			0.0		0	0				
Act Effet Green (s)	U	14.8			14.8		U	54.8				
Actuated a/C Ratio		0.18			0.18			0.68				
v/c Ratio		0.10			0.10			0.56				
Control Delay		30.5			20.1			3.0				
		0.0			20.1			0.0				
Total Delay		30.5			20.1			3.2				
		0.00 C			20.1 C			Δ				
Annroach Delay		30.5			20.1			32				
Approach LOS		00.0 C			20.1			Δ				
90th %ile Green (s)	14.8	14.8			14.8		54.8	54.8				
90th %ile Term Code	MayR	MayR			Hold		Coord	Coord				
70th %ile Green (s)	14.8	14.8			14.8		54.8	54.8				
70th %ile Term Code	MayR	MayR			Hold		Coord	Coord				
50th %ile Green (s)	1/1 8	1/1 8			1/1 8		5/ 8	5/ 8				
50th %ile Term Code	MayR	MayR			Hold		Coord	Coord				
30th %ile Green (s)	14.8	14.8			14.8		54.8	54.8				
30th %ile Term Code	MayR	MayR			Hold		Coord	Coord				
10th %ile Green (s)	1/1 8	1/1 8			1/1 8		5/ 8	5/ 8				
10th %ile Term Code	MayR	MayR			Hold		Coord	Coord				
Stope (yph)	Ινιαλί Ν	1010711			/12		COOld	1/3				
		47			42			145				
CO Emissions (g/hr)		51			70			202				
NOv Emissions (g/ll)		10			1/			292				
VOC Emissions (g/III)		10			14			68				
Dilemma Vehicles (#)		12						00				
		0			20			U				
Queue Length Ofth (It)		20			20 E0			4 I 52				
Internal Link Dict (ff)		199			630			20			220	
Turn Bay Length (ft)		100			030			220			200	

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 16: N East St & E Jefferson St

02/22/2024	
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Base Capacity (vph)		254			317			2101				
Starvation Cap Reductn		0			0			221				
Spillback Cap Reductn		0			0			0				
Storage Cap Reductn		0			0			0				
Reduced v/c Ratio		0.23			0.26			0.62				
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 55 (69%), Reference	ed to phase	2:NBTL a	ind 6:, Sta	art of Gre	en							
Natural Cycle: 65												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.56												
Intersection Signal Delay:	5.5			In	tersectior	n LOS: A						
Intersection Capacity Utiliz	ation 53.6%			IC	U Level o	of Service	А					
Analysis Period (min) 15												

Splits and Phases: 16: N East St & E Jefferson St

Ø2 (R)	<u>→</u> _{Ø4}
60 s	20 s
	← Ø8
	20 s

Lanes, Volumes, Timings 19: N Madison St & W Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ħ		5	+					5	† 12	
Traffic Volume (vph)	0	165	35	75	250	0	0	0	0	45	1095	45
Future Volume (vph)	0	165	35	75	250	0	0	0	0	45	1095	45
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	13	12	12	13	12	12	12	12	12	12	12
Grade (%)		3%			-3%			2%			-2%	
Storage Length (ft)	0		0	65		0	0		0	150		0
Storage Lanes	0		0	1		0	0		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95
Ped Bike Factor		1.00		0.99						1.00	1.00	
Frt		0.976									0.994	
Flt Protected				0.950						0.950		
Satd. Flow (prot)	0	1494	0	1617	1758	0	0	0	0	1367	3195	0
Flt Permitted				0.523						0.950		
Satd. Flow (perm)	0	1494	0	885	1758	0	0	0	0	1364	3195	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		12									10	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		690			253			324			310	
Travel Time (s)		15.7			5.8			7.4			7.0	
Confl. Peds. (#/hr)			4	4						2		3
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Parking (#/hr)	-	0					-		-	10		10
Adj. Flow (vph)	0	174	37	79	263	0	0	0	0	47	1153	47
Shared Lane Traffic (%)						-						
Lane Group Flow (vph)	0	211	0	79	263	0	0	0	0	47	1200	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
I wo way Left Turn Lane	4 47	4.00	4 47	4.40	Yes	4.40	4.40	4.40	4.40	4.00	4.40	4 4 0
Headway Factor	1.17	1.28	1.17	1.12	1.08	1.12	1.10	1.10	1.16	1.38	1.13	1.13
Turning Speed (mpn)	15	0	9	15	0	9	15		9	15	0	9
Number of Detectors		Z		ا	Z Thau					ا	Z	
Detector Template		100		Len	1 nru 100					Len	100	
Leading Detector (II)		100		20	100					20	100	
Trailing Detector (It)		0		0	0					0	0	
Detector 1 Position(II)		0		20	0					20	0	
Detector 1 Type												
Detector 1 Channel					OFEX							
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (a)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position/ft)		0.0		0.0	0.0 Q/					0.0	0.0 Q/	
Detector 2 Size(ft)		54			54						34	
Detector 2 Type		CI+Ev			CI+Ev						CI+Ev	

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 19: N Madison St & W Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		17.4		27.4	27.4					35.6	35.6	
Total Split (s)		23.0		23.0	23.0					57.0	57.0	
Total Split (%)		28.8%		28.8%	28.8%					71.3%	71.3%	
Maximum Green (s)		17.6		17.6	17.6					51.4	51.4	
Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)		2.2		2.2	2.2					2.4	2.4	
Lost Time Adjust (s)		0.0		0.0	0.0					0.0	0.0	
Total Lost Time (s)		5.4		5.4	5.4					5.6	5.6	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		Max	Max					C-Max	C-Max	
Walk Time (s)		1.0		11.0	11.0					19.0	19.0	
Flash Dont Walk (s)		11.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		17.6		17.6	17.6					51.4	51.4	
Actuated g/C Ratio		0.22		0.22	0.22					0.64	0.64	
v/c Ratio		0.62		0.41	0.68					0.05	0.58	
Control Delay		35.8		20.7	26.4					3.2	4.3	
Queue Delay		0.0		0.0	15.6					0.0	0.2	
Total Delay		35.8		20.7	42.0					3.2	4.4	
LOS		D		С	D					А	А	
Approach Delay		35.8			37.1						4.4	
Approach LOS		D			D						А	
90th %ile Green (s)		17.6		17.6	17.6					51.4	51.4	
90th %ile Term Code		Max		Ped	Ped					Coord	Coord	
70th %ile Green (s)		17.6		17.6	17.6					51.4	51.4	
70th %ile Term Code		Max		Ped	Ped					Coord	Coord	
50th %ile Green (s)		17.6		17.6	17.6					51.4	51.4	
50th %ile Term Code		Hold		Ped	Ped					Coord	Coord	
30th %ile Green (s)		17.6		17.6	17.6					51.4	51.4	
30th %ile Term Code		Hold		Ped	Ped					Coord	Coord	
10th %ile Green (s)		17.6		17.6	17.6					51.4	51.4	
10th %ile Term Code		Hold		Ped	Ped					Coord	Coord	
Stops (vph)		170		72	246					7	267	
Fuel Used(gal)		3		1	3					0	5	
CO Emissions (g/hr)		243		60	224					12	365	
NOx Emissions (g/hr)		47		12	43					2	71	
VOC Emissions (g/hr)		56		14	52					3	85	
Dilemma Vehicles (#)		0		0	0					0	0	
Queue Length 50th (ft)		90		42	144					4	47	

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 19: N Madison St & W Washington St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)		162		82	#227					m6	57	
Internal Link Dist (ft)		610			173			244			230	
Turn Bay Length (ft)				65						150		
Base Capacity (vph)		338		194	386					876	2056	
Starvation Cap Reductn		0		0	107					0	197	
Spillback Cap Reductn		0		0	0					106	0	
Storage Cap Reductn		0		0	0					0	0	
Reduced v/c Ratio		0.62		0.41	0.94					0.06	0.65	
Intersection Summary												
Area Type: (CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 0 (0%), Referenced to	o phase 2:	and 6:SB	TL, Start	of Green								
Natural Cycle: 65												
Control Type: Actuated-Coor	dinated											
Maximum v/c Ratio: 0.68												
Intersection Signal Delay: 14	.3			In	tersectior	n LOS: B						
Intersection Capacity Utilizat	ion 65.6%			IC	U Level o	of Service	С					
Analysis Period (min) 15												
# 95th percentile volume e	xceeds cap	oacity, qu	eue may l	be longer								
Queue shown is maximur	n after two	cycles.										
m Volume for 95th percent	ile queue is	s metered	l by upstr	eam sign	al.							
Splits and Phases: 19: N N	Madison St	& W Was	shington S	St								

	→ Ø4	
	23 s	
Ø6 (R)	₹Ø8	
57 s	23 s	

Lanes, Volumes, Timings 20: N East St & E Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	1			+	1	7	≜ 1₽				
Traffic Volume (vph)	40	275	0	0	80	100	35	1095	90	0	0	0
Future Volume (vph)	40	275	0	0	80	100	35	1095	90	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	11	10	11	11	11	12	12	12
Grade (%)		-3%			2%			-3%			1%	
Storage Length (ft)	65		0	0		80	80		0	0		0
Storage Lanes	1		0	0		1	1		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.99					0.98	1.00	1.00				
Frt						0.850		0.989				
Flt Protected	0.950						0.950					
Satd. Flow (prot)	1617	1702	0	0	1604	1317	1250	3084	0	0	0	0
Flt Permitted	0.701						0.950					
Satd. Flow (perm)	1186	1702	0	0	1604	1292	1245	3084	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)						64		17				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		262			724			322			308	
Travel Time (s)		6.0			16.5			7.3			7.0	
Confl. Peds. (#/hr)	4					4	4		9			
Confl. Bikes (#/hr)						1						
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Parking (#/hr)							20		20			
Adj. Flow (vph)	43	296	0	0	86	108	38	1177	97	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	43	296	0	0	86	108	38	1274	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			11			11	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.12	1.12	1.12	1.16	1.21	1.26	1.54	1.17	1.17	1.15	1.15	1.15
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2			2	1	1	2				
Detector Template	Left	Thru			Thru	Right	Left	Thru				
Leading Detector (ft)	20	100			100	20	20	100				
Trailing Detector (ft)	0	0			0	0	0	0				
Detector 1 Position(ft)	0	0			0	0	0	0				
Detector 1 Size(ft)	20	6			6	20	20	6				
Detector 1 Type	Cl+Ex	Cl+Ex			Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex				
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 20: N East St & E Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Type		Cl+Ex			Cl+Ex			CI+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA	Perm	Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4					8	2					
Detector Phase	4	4			8	8	2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0	5.0	2.0	2.0				
Minimum Split (s)	31.4	31.4			21.4	21.4	47.0	47.0				
Total Split (s)	29.0	29.0			29.0	29.0	51.0	51.0				
Total Split (%) 3	6.3%	36.3%			36.3%	36.3%	63.8%	63.8%				
Maximum Green (s)	23.6	23.6			23.6	23.6	45.0	45.0				
Yellow Time (s)	3.2	3.2			3.2	3.2	3.2	3.2				
All-Red Time (s)	2.2	2.2			2.2	2.2	2.8	2.8				
Lost Time Adjust (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Total Lost Time (s)	5.4	5.4			5.4	5.4	6.0	6.0				
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0	3.0	3.0	3.0				
Recall Mode	None	None			None	None	C-Max	C-Max				
Walk Time (s)	15.0	15.0			5.0	5.0	30.0	30.0				
Flash Dont Walk (s)	11.0	11.0			11.0	11.0	11.0	11.0				
Pedestrian Calls (#/hr)	0	0			0	0	0	0				
Act Effct Green (s)	18.6	18.6			18.6	18.6	50.0	50.0				
Actuated g/C Ratio	0.23	0.23			0.23	0.23	0.62	0.62				
v/c Ratio	0.16	0.75			0.23	0.31	0.05	0.66				
Control Delay	7.2	19.2			24.9	13.5	3.6	5.8				
Queue Delav	0.0	0.5			0.0	0.0	0.0	0.1				
Total Delay	7.2	19.7			24.9	13.5	3.6	5.9				
LOS	А	В			С	В	A	A				
Approach Delay		18.1			18.5			5.8				
Approach LOS		В			В			A				
90th %ile Green (s)	23.6	23.6			23.6	23.6	45.0	45.0				
90th %ile Term Code	Max	Max			Hold	Hold	Coord	Coord				
70th %ile Green (s)	21.7	21.7			21.7	21.7	46.9	46.9				
70th %ile Term Code	Gap	Gap			Hold	Hold	Coord	Coord				
50th %ile Green (s)	19.1	19.1			19.1	19.1	49.5	49.5				
50th %ile Term Code	Gap	Gap			Hold	Hold	Coord	Coord				
30th %ile Green (s)	16.3	16.3			16.3	16.3	52.3	52.3				
30th %ile Term Code	Gap	Gap			Hold	Hold	Coord	Coord				
10th %ile Green (s)	12.3	12.3			12.3	12.3	56.3	56.3				
10th %ile Term Code	Gap	Gap			Hold	Hold	Coord	Coord				
Stops (vph)	10	109			60	37	6	386				
Fuel Used(gal)	0	2			1	1	Û	6				
CO Emissions (a/hr)	14	157			83	73	10	454				
NOx Emissions (a/hr)	3	30			16	14	2	88				
VOC Emissions (a/hr)	3	36			19	17	2	105				
Dilemma Vehicles (#)	0	0			0	0	0	0				

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 20: N East St & E Washington St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)	4	26			35	17	3	55				
Queue Length 95th (ft)	9	60			66	54	m6	71				
Internal Link Dist (ft)		182			644			242			228	
Turn Bay Length (ft)	65					80	80					
Base Capacity (vph)	349	502			473	426	778	1934				
Starvation Cap Reductn	0	37			0	0	0	92				
Spillback Cap Reductn	0	0			10	0	0	0				
Storage Cap Reductn	0	0			0	0	0	0				
Reduced v/c Ratio	0.12	0.64			0.19	0.25	0.05	0.69				
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 64 (80%), Reference	ed to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 80												
Control Type: Actuated-Coo	ordinated											
Maximum v/c Ratio: 0.75												
Intersection Signal Delay: 9.	9.4 Intersection LOS: A											
Intersection Capacity Utiliza	Itilization 63.1% ICU Level of Service B											
Analysis Period (min) 15												
m Volume for 95th percen	tile queue is	s metered	l by upstr	eam sign	al.							

Splits and Phases: 20: N East St & E Washington St

Ø2 (R)	A 104	22
51 s	29 s	
	Ø8	
	29 s	

Lanes, Volumes, Timings 23: S Center St/N Madison St & W Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,		5	+						đ î þ	
Traffic Volume (vph)	0	30	25	75	55	0	0	0	0	10	1130	30
Future Volume (vph)	0	30	25	75	55	0	0	0	0	10	1130	30
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	13	12	12	12	12	12	12	12	12
Grade (%)		3%			-3%			1%			-1%	
Storage Length (ft)	0		0	60		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	0		0
Taper Length (ft)	25			0			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		0.99		0.98							1.00	
Frt		0.938									0.996	
Flt Protected				0.950								
Satd. Flow (prot)	0	1374	0	1670	1531	0	0	0	0	0	3186	0
Flt Permitted				0.720								
Satd. Flow (perm)	0	1374	0	1244	1531	0	0	0	0	0	3186	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		26									7	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		684			247			660			324	
Travel Time (s)		15.5			5.6			15.0			7.4	
Confl. Peds. (#/hr)			8	8						13		4
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Parking (#/hr)		0			0					20		20
Adj. Flow (vph)	0	31	26	77	57	0	0	0	0	10	1165	31
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	57	0	77	57	0	0	0	0	0	1206	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		13			13			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
I wo way Left I urn Lane												
Headway Factor	1.17	1.33	1.17	1.08	1.28	1.12	1.15	1.15	1.15	1.14	1.14	1.14
Turning Speed (mph)	15	•	9	15	•	9	15		9	15	•	9
Number of Detectors		2		1	2					1	2	
Detector Template		I hru		Left	I hru					Left	I hru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(It)		0		0	0					0	0	
Detector 1 Size(ft)		0		20	0					20	0	
Detector 1 Type		CI+EX		CI+EX	CI+EX					CI+EX	CI+EX	
Detector 1 Channel		0.0		0.0	0.0					0.0	0.0	
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (S)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(It)		94			94						94	
Detector 2 Size(II)					0						0	
Detector 2 Type		UI+EX			UI+EX						UI+EX	

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 23: S Center St/N Madison St & W Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		15.5		25.5	25.5					37.7	37.7	
Total Split (s)		22.0		22.0	22.0					58.0	58.0	
Total Split (%)		27.5%		27.5%	27.5%					72.5%	72.5%	
Maximum Green (s)		16.5		16.5	16.5					52.3	52.3	
Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)		2.3		2.3	2.3					2.5	2.5	
Lost Time Adjust (s)		0.0		0.0	0.0						0.0	
Total Lost Time (s)		5.5		5.5	5.5						5.7	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		None	None					C-Max	C-Max	
Walk Time (s)		1.0		9.0	9.0					21.0	21.0	
Flash Dont Walk (s)		9.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		10.2		10.2	10.2						61.9	
Actuated g/C Ratio		0.13		0.13	0.13						0.77	
v/c Ratio		0.29		0.48	0.29						0.49	
Control Delay		22.7		41.6	33.9						2.6	
Queue Delay		0.0		0.0	0.0						0.0	
Total Delay		22.7		41.6	33.9						2.6	
LOS		С		D	С						А	
Approach Delay		22.7			38.3						2.6	
Approach LOS		С			D						А	
90th %ile Green (s)		14.9		14.9	14.9					53.9	53.9	
90th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
70th %ile Green (s)		12.2		12.2	12.2					56.6	56.6	
70th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
50th %ile Green (s)		10.2		10.2	10.2					58.6	58.6	
50th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
30th %ile Green (s)		8.3		8.3	8.3					60.5	60.5	
30th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
10th %ile Green (s)		0.0		0.0	0.0					74.3	74.3	
10th %ile Term Code		Skip		Skip	Skip					Coord	Coord	
Stops (vph)		31		66	48						161	
Fuel Used(gal)		1		1	1						4	
CO Emissions (g/hr)		50		80	53						312	
NOx Emissions (g/hr)		10		16	10						61	
VOC Emissions (g/hr)		12		19	12						72	
Dilemma Vehicles (#)		0		0	0						0	
Queue Length 50th (ft)		14		36	26						55	

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 23: S Center St/N Madison St & W Front St

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Lane Group	EBL	EBT	FBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)		44		74	56						65	
Internal Link Dist (ft)		604			167			580			244	
Turn Bay Length (ft)				60								
Base Capacity (vph)		304		256	315						2467	
Starvation Cap Reductn		0		0	0						122	
Spillback Cap Reductn		0		0	0						0	
Storage Cap Reductn		0		0	0						0	
Reduced v/c Ratio		0.19		0.30	0.18						0.51	
Intersection Summary												
Area Type: C	BD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 0 (0%), Referenced to	phase 2:	and 6:SB	TL, Start	of Green								
Natural Cycle: 65												
Control Type: Actuated-Coord	linated											
Maximum v/c Ratio: 0.49												
Intersection Signal Delay: 6.9				In	tersectior	n LOS: A						
Intersection Capacity Utilization	on 56.7%			IC	U Level o	of Service	В					
Analysis Period (min) 15												

Splits and Phases: 23: S Center St/N Madison St & W Front St

	→ Ø4
	22 s
₩ Ø6 (R)	₩ Ø8
58 s	22 s

02/22/2024

Lanes, Volumes, Timings 24: S East St/N East St & E Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	•			•	1		đ î þ				
Traffic Volume (vph)	90	130	0	0	45	30	40	990	35	0	0	0
Future Volume (vph)	90	130	0	0	45	30	40	990	35	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	16	12	12	10	10	13	11	11	12	12	12
Grade (%)		-1%			0%			-1%			2%	
Storage Length (ft)	90		0	0		100	0		0	0		0
Storage Lanes	1		0	0		1	0		0	0		0
Taper Length (ft)	0			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.97					0.95		1.00				
Frt						0.850		0.995				
Flt Protected	0.950							0.998				
Satd, Flow (prot)	1547	1910	0	0	1565	1330	0	3071	0	0	0	0
Flt Permitted	0.723		-	-			-	0.998	-	-	-	-
Satd. Flow (perm)	1138	1910	0	0	1565	1267	0	3068	0	0	0	0
Right Turn on Red			Yes	, in the second s		Yes	· ·		Yes	•	•	Yes
Satd Flow (RTOR)						35		8				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		256			710			633			322	
Travel Time (s)		5.8			16.1			14.4			7.3	
Confl Peds (#/hr)	18	0.0			10.1	18	28		1		1.0	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Adi Flow (vph)	103	149	0.01	0.01	52	34	46	1138	40	0.01	0	0.01
Shared Lane Traffic (%)	100	110	Ŭ	Ŭ	02	• •				Ŭ	Ŭ	Ū
Lane Group Flow (vph)	103	149	0	0	52	34	0	1224	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)	Lon	11	rugitt	2011	11	. ugint	2011	11	rugitt	Lon	11	i agin
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1 19	0.97	1 14	1 14	1 25	1 25	1 09	1 19	1 19	1 16	1 16	1 16
Turning Speed (mph)	15	0.01	9	15	1.20	9	15		9	15	1.10	9
Number of Detectors	1	2	Ū		2	1	1	2	Ŭ			U
Detector Template	Left	Thru			Thru	Right	l eft	Thru				
Leading Detector (ft)	20	100			100	20	20	100				
Trailing Detector (ft)	0	0			0	0	0	0				
Detector 1 Position(ft)	0	0			0	0	0	0				
Detector 1 Size(ft)	20	6			6	20	20	6				
Detector 1 Type	CI+Ex	Cl+Ex			Cl+Ex	CI+Ex	CI+Ex	CI+Ex				
Detector 1 Channel	OT EX	ONEX			OF	OFER	OI! EX	OFER				
Detector 1 Extend (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 2 Position(ft)	0.0	94			94	0.0	0.0	94				
Detector 2 Size/ft)		6			6			6				
Detector 2 Type												
Detector 2 Channel												

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 24: S East St/N East St & E Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA	Perm	Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4					8	2					
Detector Phase	4	4			8	8	2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0	5.0	5.0	5.0				
Minimum Split (s)	27.2	27.2			17.2	17.2	50.6	50.6				
Total Split (s)	25.0	25.0			25.0	25.0	55.0	55.0				
Total Split (%)	31.3%	31.3%			31.3%	31.3%	68.8%	68.8%				
Maximum Green (s)	19.8	19.8			19.8	19.8	49.4	49.4				
Yellow Time (s)	3.2	3.2			3.2	3.2	3.2	3.2				
All-Red Time (s)	2.0	2.0			2.0	2.0	2.4	2.4				
Lost Time Adjust (s)	0.0	0.0			0.0	0.0		0.0				
Total Lost Time (s)	5.2	5.2			5.2	5.2		5.6				
Lead/Lag					-							
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0	3.0	3.0	3.0				
Recall Mode	None	None			Max	Max	C-Max	C-Max				
Walk Time (s)	11.0	11.0			1.0	1.0	34.0	34.0				
Flash Dont Walk (s)	11.0	11.0			11.0	11.0	11.0	11.0				
Pedestrian Calls (#/hr)	0	0			0	0	0	0				
Act Effct Green (s)	19.8	19.8			19.8	19.8	-	49.4				
Actuated q/C Ratio	0.25	0.25			0.25	0.25		0.62				
v/c Ratio	0.37	0.32			0.13	0.10		0.65				
Control Delay	29.5	26.8			24.6	9.3		7.3				
Queue Delav	0.0	0.0			0.0	0.0		0.1				
Total Delay	29.5	26.8			24.6	9.3		7.3				
LOS	С	С			С	A		A				
Approach Delay		27.9			18.6			7.3				
Approach LOS		С			В			А				
90th %ile Green (s)	19.8	19.8			19.8	19.8	49.4	49.4				
90th %ile Term Code	Hold	Hold			MaxR	MaxR	Coord	Coord				
70th %ile Green (s)	19.8	19.8			19.8	19.8	49.4	49.4				
70th %ile Term Code	Hold	Hold			MaxR	MaxR	Coord	Coord				
50th %ile Green (s)	19.8	19.8			19.8	19.8	49.4	49.4				
50th %ile Term Code	Hold	Hold			MaxR	MaxR	Coord	Coord				
30th %ile Green (s)	19.8	19.8			19.8	19.8	49.4	49.4				
30th %ile Term Code	Hold	Hold			MaxR	MaxR	Coord	Coord				
10th %ile Green (s)	19.8	19.8			19.8	19.8	49.4	49.4				
10th %ile Term Code	Hold	Hold			MaxR	MaxR	Coord	Coord				
Stops (vph)	73	103			36	9		337				
Fuel Used(gal)	1	2			1	0		9				
CO Emissions (g/hr)	78	108			47	19		608				
NOx Emissions (a/hr)	15	21			9	4		118				
VOC Emissions (g/hr)	18	25			11	4		141				
Dilemma Vehicles (#)	0	0			0	0		0				
Queue Length 50th (ft)	43	61			20	0		90				
Queue Length 95th (ft)	84	107			47	20		106				

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 24: S East St/N East St & E Front St

02/22/2024	ŀ
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		176			630			553			242	
Turn Bay Length (ft)	90					100						
Base Capacity (vph)	281	472			387	339		1897				
Starvation Cap Reductn	0	0			0	0		0				
Spillback Cap Reductn	0	0			0	0		52				
Storage Cap Reductn	0	0			0	0		0				
Reduced v/c Ratio	0.37	0.32			0.13	0.10		0.66				
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 58 (73%), Reference	ed to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 80												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.65												
Intersection Signal Delay:	11.3			In	tersectior	n LOS: B						
Intersection Capacity Utiliz	ation 66.4%			IC	U Level o	of Service	С					
Analysis Period (min) 15												
Splits and Phases: 24: S	East St/N E	ast St & E	E Front St	t								

Ø2 (R)	<u></u> ∠ ₁₀₄
55 s	25 s
	Ø8
	25 s

Lanes, Volumes, Timings 27: N Center St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,			د						đ þ	
Traffic Volume (vph)	0	148	31	37	119	0	0	0	0	37	134	17
Future Volume (vph)	0	148	31	37	119	0	0	0	0	37	134	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt		0.977									0.986	
Flt Protected					0.988						0.990	
Satd. Flow (prot)	0	1606	0	0	1462	0	0	0	0	0	2897	0
Flt Permitted					0.988						0.990	
Satd. Flow (perm)	0	1606	0	0	1462	0	0	0	0	0	2897	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		247			266			264			413	
Travel Time (s)		5.6			6.0			6.0			9.4	
Confl. Peds. (#/hr)			30	30						6		5
Confl. Bikes (#/hr)			1									
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)					0						0	
Adj. Flow (vph)	0	164	34	41	132	0	0	0	0	41	149	19
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	198	0	0	173	0	0	0	0	0	209	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.14	1.14	1.14	1.30	1.14	1.14	1.14	1.14	1.14	1.22	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: 0	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 38.9%			IC	CU Level of	of Service	Α					
Analysis Period (min) 15												

Lanes, Volumes, Timings 29: N Center St & W Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f,			ŧ						đ î þ	
Traffic Volume (vph)	0	39	4	10	44	0	0	0	0	39	106	11
Future Volume (vph)	0	39	4	10	44	0	0	0	0	39	106	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt		0.986									0.989	
Flt Protected					0.991						0.988	
Satd. Flow (prot)	0	1459	0	0	1466	0	0	0	0	0	2900	0
Flt Permitted					0.991						0.988	
Satd. Flow (perm)	0	1459	0	0	1466	0	0	0	0	0	2900	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		253			255			306			278	
Travel Time (s)		5.8			5.8			7.0			6.3	
Confl. Peds. (#/hr)			5	5						20		8
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)		0			0						0	
Adj. Flow (vph)	0	45	5	12	51	0	0	0	0	45	123	13
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	50	0	0	63	0	0	0	0	0	181	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.14	1.14	1.14	1.22	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: 0	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 23.4%			IC	CU Level of	of Service	А					

Analysis Period (min) 15

Lanes, Volumes, Timings 30: N Center St & W Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ħ		5	+						đ î þ	
Traffic Volume (vph)	0	213	6	9	316	0	0	0	0	68	47	8
Future Volume (vph)	0	213	6	9	316	0	0	0	0	68	47	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	65		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		1.00		0.99							0.98	
Frt		0.996									0.991	
Flt Protected				0.950							0.973	
Satd. Flow (prot)	0	1473	0	1562	1480	0	0	0	0	0	2853	0
Flt Permitted				0.542							0.973	
Satd. Flow (perm)	0	1473	0	883	1480	0	0	0	0	0	2804	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		3									8	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		253			254			326			306	
Travel Time (s)		5.8			5.8			7.4			7.0	
Confl. Peds. (#/hr)			10	10						14		12
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)		0			0						0	
Adj. Flow (vph)	0	222	6	9	329	0	0	0	0	71	49	8
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	228	0	9	329	0	0	0	0	0	128	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.14	1.14	1.14	1.22	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		Cl+Ex	CI+Ex					Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		CI+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 30: N Center St & W Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		23.0		23.0	23.0					23.0	23.0	
Total Split (s)		52.0		52.0	52.0					28.0	28.0	
Total Split (%)		65.0%		65.0%	65.0%					35.0%	35.0%	
Maximum Green (s)		47.0		47.0	47.0					23.0	23.0	
Yellow Time (s)		4.0		4.0	4.0					4.0	4.0	
All-Red Time (s)		1.0		1.0	1.0					1.0	1.0	
Lost Time Adjust (s)		0.0		0.0	0.0						0.0	
Total Lost Time (s)		5.0		5.0	5.0						5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		None	None					C-Max	C-Max	
Walk Time (s)		7.0		7.0	7.0					7.0	7.0	
Flash Dont Walk (s)		11.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		24.8		24.8	24.8						45.2	
Actuated g/C Ratio		0.31		0.31	0.31						0.56	
v/c Ratio		0.50		0.03	0.72						0.08	
Control Delay		39.8		4.0	13.5						9.6	
Queue Delav		0.2		0.0	0.1						0.0	
Total Delay		40.1		4.0	13.6						9.6	
LOS		D		A	В						A	
Approach Delay		40.1			13.3						9.6	
Approach LOS		D			В						А	
90th %ile Green (s)		35.1		35.1	35.1					34.9	34.9	
90th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
70th %ile Green (s)		28.7		28.7	28.7					41.3	41.3	
70th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
50th %ile Green (s)		24.6		24.6	24.6					45.4	45.4	
50th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
30th %ile Green (s)		20.6		20.6	20.6					49.4	49.4	
30th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
10th %ile Green (s)		15.0		15.0	15.0					55.0	55.0	
10th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
Stops (vph)		200		1	41						55	
Fuel Used(gal)		3		0	2						1	
CO Emissions (g/hr)		232		2	120						59	
NOx Emissions (a/hr)		45		0	23						11	
VOC Emissions (a/hr)		54		0	28						14	
Dilemma Vehicles (#)		0		0	0						0	
Queue Length 50th (ft)		118		0	19						13	
Queue Length 95th (ft)		165		m1	26						34	

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 30: N Center St & W Washington St

02/22/2024	ŀ
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		173			174			246			226	
Turn Bay Length (ft)				65								
Base Capacity (vph)		866		518	869						1587	
Starvation Cap Reductn		214		0	38						0	
Spillback Cap Reductn		0		0	90						0	
Storage Cap Reductn		0		0	0						0	
Reduced v/c Ratio		0.35		0.02	0.42						0.08	
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 2 (3%), Referenced	I to phase 2:	and 6:SB	TL, Start	of Green								
Natural Cycle: 50												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.72												
Intersection Signal Delay: 2	21.4			In	tersectior	n LOS: C						
Intersection Capacity Utiliz	ation 53.3%			IC	U Level o	of Service	A					
Analysis Period (min) 15												
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m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 30: N Center St & W Washington St

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	52 s	
Ø6 (R)	€ Ø8	
28 s	52 s	
Lanes, Volumes, Timings 31: W Front St & N Center St

02/22/202	24
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,		7	•			\$		7	ħ	
Traffic Volume (vph)	0	59	6	6	122	0	0	0	1	51	3	10
Future Volume (vph)	0	59	6	6	122	0	0	0	1	51	3	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	8	12	12	12	12
Storage Length (ft)	0		0	70		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.988						0.865			0.887	
Flt Protected				0.950						0.950		
Satd. Flow (prot)	0	1395	0	1490	1412	0	0	1176	0	1341	1252	0
Flt Permitted				0.950						0.950		
Satd. Flow (perm)	0	1395	0	1490	1412	0	0	1176	0	1341	1252	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		247			262			221			326	
Travel Time (s)		5.6			6.0			5.0			7.4	
Confl. Peds. (#/hr)			48	48			25		13	13		25
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	9%	9%	9%	9%	9%	9%	9%	9%	9%	9%	9%	9%
Parking (#/hr)		0			0					0	0	
Adj. Flow (vph)	0	72	7	7	149	0	0	0	1	62	4	12
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	79	0	7	149	0	0	1	0	62	16	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		13			13			0			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane					Yes							
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.37	1.14	1.30	1.30	1.14
Turning Speed (mph)	15	-	9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: C	BD											
Control Type: Unsignalized												
Intersection Capacity Utilization	on 30.0%			IC	U Level o	of Service	А					
Analysis Period (min) 15												

Lanes, Volumes, Timings 34: N Main St & W Market St/E Market St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ŧ			Þ			đ î de				
Traffic Volume (vph)	36	144	0	0	127	9	48	64	28	0	0	0
Future Volume (vph)	36	144	0	0	127	9	48	64	28	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.991			0.970				
Flt Protected		0.990						0.983				
Satd. Flow (prot)	0	1479	0	0	1481	0	0	2857	0	0	0	0
Flt Permitted		0.990						0.983				
Satd. Flow (perm)	0	1479	0	0	1481	0	0	2857	0	0	0	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		266			263			249			404	
Travel Time (s)		6.0			6.0			5.7			9.2	
Confl. Peds. (#/hr)	19					19	20		18			
Confl. Bikes (#/hr)						1			1			
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Parking (#/hr)		0			0			0				
Adj. Flow (vph)	42	167	0	0	148	10	56	74	33	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	209	0	0	158	0	0	163	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.22	1.14	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: 0	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 39.9%			IC	CU Level o	of Service	А					
Analysis Period (min) 15												

Lanes, Volumes, Timings 36: N Main St & W Jefferson St/E Jefferson St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ŧ			ħ			đ þ				
Traffic Volume (vph)	37	39	0	0	31	17	16	83	9	0	0	0
Future Volume (vph)	37	39	0	0	31	17	16	83	9	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.953			0.987				
Flt Protected		0.976						0.993				
Satd. Flow (prot)	0	1404	0	0	1371	0	0	2827	0	0	0	0
Flt Permitted		0.976						0.993				
Satd. Flow (perm)	0	1404	0	0	1371	0	0	2827	0	0	0	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		255			268			306			272	
Travel Time (s)		5.8			6.1			7.0			6.2	
Confl. Peds. (#/hr)	11					11	3		22			
Confl. Bikes (#/hr)									2			
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83
Heavy Vehicles (%)	7%	7%	7%	7%	7%	7%	7%	7%	7%	7%	7%	7%
Parking (#/hr)		0			0			0				
Adj. Flow (vph)	45	47	0	0	37	20	19	100	11	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	92	0	0	57	0	0	130	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.22	1.14	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: C	BD											
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 26.5%			IC	CU Level o	of Service	А					
Analysis Period (min) 15												

Lanes, Volumes, Timings	
37: N Main St & W Washington St/E Washington St	t

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	1			f)			é.	1			
Traffic Volume (vph)	19	266	0	0	321	41	12	29	17	0	0	0
Future Volume (vph)	19	266	0	0	321	41	12	29	17	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	65		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99				1.00			0.99	0.93			
Frt					0.985				0.850			
Flt Protected	0.950							0.986				
Satd. Flow (prot)	1562	1480	0	0	1454	0	0	1459	1258	0	0	0
Flt Permitted	0.368							0.986				
Satd. Flow (perm)	602	1480	0	0	1454	0	0	1451	1168	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					14				27			
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		254			262			326			306	
Travel Time (s)		5.8			6.0			7.4			7.0	
Confl. Peds. (#/hr)	8					8	9		23			
Confl. Bikes (#/hr)						1			1			
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)		0	-	-	0			0	0	-	-	
Adj. Flow (vph)	19	271	0	0	328	42	12	30	17	0	0	0
Shared Lane Traffic (%)		a = 4									•	
Lane Group Flow (vph)	19	271	0	0	370	0	0	42	17	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
		16			16			16			16	
I wo way Left Turn Lane		Yes			Yes	4 4 4		4 00	4 00			4 4 4
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.30	1.30	1.14	1.14	1.14
Turning Speed (mpn)	15	0	9	15	0	9	15	0	9	15		9
Number of Detectors	1	 			 		1	Z	Diacht			
Detector Template	Len	1 nru			1 nru 100		Leit	1 nru 100	Right			
Leading Detector (ft)	20	100			100		20	100	20			
Trailing Detector (It)	0	0			0		0	0	0			
Detector 1 Position(it)	20	0			0		20	0	20			
Detector 1 Size(II)												
Detector 1 Type	CI+EX	UI+EX			UI+EX		CI+EX	CI+EX	U+EX			
Detector 1 Channel	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 1 Exterio (s)	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 1 Delev (s)	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 2 Position(ft)	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 2 Sizo(#)		54			54			54				
Detector 2 Size(II)												
Detector z Type		OI+EX			OI+EX			UI+⊏X				

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings	
37: N Main St & W Washington St/E Washington	St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA		Perm	NA	Perm			
Protected Phases		4			8			2				
Permitted Phases	4						2		2			
Detector Phase	4	4			8		2	2	2			
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0		5.0	5.0	5.0			
Minimum Split (s)	23.0	23.0			23.0		23.0	23.0	23.0			
Total Split (s)	52.0	52.0			52.0		28.0	28.0	28.0			
Total Split (%)	65.0%	65.0%			65.0%		35.0%	35.0%	35.0%			
Maximum Green (s)	47.0	47.0			47.0		23.0	23.0	23.0			
Yellow Time (s)	4.0	4.0			4.0		4.0	4.0	4.0			
All-Red Time (s)	1.0	1.0			1.0		1.0	1.0	1.0			
Lost Time Adjust (s)	0.0	0.0			0.0			0.0	0.0			
Total Lost Time (s)	5.0	5.0			5.0			5.0	5.0			
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0	3.0			
Recall Mode	None	None			None		C-Max	C-Max	C-Max			
Walk Time (s)	7.0	7.0			7.0		7.0	7.0	7.0			
Flash Dont Walk (s)	11.0	11.0			11.0		11.0	11.0	11.0			
Pedestrian Calls (#/hr)	0	0			0		0	0	0			
Act Effct Green (s)	27.1	27.1			27.1			42.9	42.9			
Actuated g/C Ratio	0.34	0.34			0.34			0.54	0.54			
v/c Ratio	0.09	0.54			0.74			0.05	0.03			
Control Delay	8.8	12.6			27.3			12.3	4.5			
Queue Delay	0.0	0.1			0.2			0.0	0.0			
Total Delay	8.8	12.7			27.5			12.3	4.5			
LOS	A	В			С			В	A			
Approach Delay		12.4			27.5			10.1				
Approach LOS		В			С			В				
90th %ile Green (s)	38.1	38.1			38.1		31.9	31.9	31.9			
90th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
70th %ile Green (s)	31.4	31.4			31.4		38.6	38.6	38.6			
70th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
50th %ile Green (s)	26.9	26.9			26.9		43.1	43.1	43.1			
50th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
30th %ile Green (s)	22.5	22.5			22.5		47.5	47.5	47.5			
30th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
10th %ile Green (s)	16.4	16.4			16.4		53.6	53.6	53.6			
10th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
Stops (vpn)	6	/4			242			24	4			
Fuel Used(gal)	0	2			4			0	0			
CO Emissions (g/hr)	1	113			286			24	6			
NOX Emissions (g/hr)	1	22			56			5	1			
VUC Emissions (g/nr)	2	26			66			6	1			
Dilemma venicles (#)	0	0			0			0	0			
Queue Length 50th (ft)	3	37			121			10	0			

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 37: N Main St & W Washington St/E Washington St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)	10	70			149			m32	m8			
Internal Link Dist (ft)		174			182			246			226	
Turn Bay Length (ft)	65											
Base Capacity (vph)	353	869			860			778	639			
Starvation Cap Reductn	0	71			96			0	0			
Spillback Cap Reductn	0	0			0			0	0			
Storage Cap Reductn	0	0			0			0	0			
Reduced v/c Ratio	0.05	0.34			0.48			0.05	0.03			
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 78 (98%), Reference	ed to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 50												
Control Type: Actuated-Coc	ordinated											
Maximum v/c Ratio: 0.74												
Intersection Signal Delay: 2	0.0			In	tersectior	LOS: B						
Intersection Capacity Utiliza	tion 53.3%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
m Volume for 95th percen	ntile queue is	s metered	l by upstr	eam sign	al.							

Splits and Phases: 37: N Main St & W Washington St/E Washington St

1 Ø2 (R)		
28 s	52 s	
9430-44	+	
	Ø8	
	52 s	

02/22/2024

Lanes, Volumes, Timings 38: W Front St/E Front St & N Main St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	ĥ			\$			\$				
Traffic Volume (vph)	23	87	0	0	119	23	0	0	0	0	0	0
Future Volume (vph)	23	87	0	0	119	23	0	0	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	8	12	12	12	12
Storage Length (ft)	70		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.978							
Flt Protected	0.950											
Satd. Flow (prot)	1504	1425	0	0	1394	0	0	1308	0	0	0	0
Flt Permitted	0.950											
Satd. Flow (perm)	1504	1425	0	0	1394	0	0	1308	0	0	0	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		262			256			210			326	
Travel Time (s)		6.0			5.8			4.8			7.4	
Confl. Peds. (#/hr)	13		7	7		13	4		32			
Peak Hour Factor	0.82	0.82	0.92	0.92	0.82	0.82	0.92	0.92	0.92	0.82	0.92	0.82
Heavy Vehicles (%)	8%	8%	2%	2%	8%	8%	2%	2%	2%	8%	2%	8%
Parking (#/hr)		0			0			0				
Adj. Flow (vph)	28	106	0	0	145	28	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	28	106	0	0	173	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.55	1.14	1.14	1.14	1.14
Turning Speed (mph)	15		60	60		9	60		60	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type: C	BD											
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 37.0%			IC	U Level o	of Service	А					
Analysia Dariad (min) 15												

Analysis Period (min) 15

	4	•	t	1	1	ţ		
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations		1	† 1>					
Traffic Volume (vph)	0	29	1224	7	0	0		
Future Volume (vph)	0	29	1224	7	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Lane Width (ft)	12	12	11	11	12	12		
Grade (%)	0%		-2%			2%		
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00		
Frt		0.865	0.999					
Flt Protected								
Satd. Flow (prot)	0	1305	3107	0	0	0		
Flt Permitted								
Satd. Flow (perm)	0	1305	3107	0	0	0		
Link Speed (mph)	30		30			30		
Link Distance (ft)	558		266			127		
Travel Time (s)	12.7		6.0			2.9		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94		
Parking (#/hr)		0						
Adj. Flow (vph)	0	31	1302	7	0	0		
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	31	1309	0	0	0		
Enter Blocked Intersection	No	No	No	No	No	No		
Lane Alignment	Left	Right	Left	Right	Left	Left		
Median Width(ft)	0		0			0		
Link Offset(ft)	0		0			0		
Crosswalk Width(ft)	16		16			16		
Two way Left Turn Lane								
Headway Factor	1.14	1.30	1.18	1.18	1.16	1.16		
Turning Speed (mph)	15	9		9	15			
Sign Control	Stop		Free			Stop		
Intersection Summary								
Area Type: 0	CBD							
Control Type: Unsignalized								
Intersection Capacity Utilizat	ion 47.8%			IC	U Level o	of Service	γA	
Analysis Period (min) 15								

Lanes, Volumes, Timings 47: S Center St & W Olive St

02/22/2024	02	122	2	024
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1	1	7	1						41	1
Traffic Volume (vph)	0	90	45	10	30	0	0	0	0	55	1175	50
Future Volume (vph)	0	90	45	10	30	0	0	0	0	55	1175	50
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	11	10	12	12	12	12	12	12	12	12	12
Grade (%)		1%			-3%			3%			-1%	
Storage Length (ft)	0		80	0		0	0		0	0		50
Storage Lanes	0		1	1		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00
Frt			0.850									0.850
Flt Protected				0.950							0.998	
Satd. Flow (prot)	0	1792	1470	1796	1891	0	0	0	0	0	3550	1591
Flt Permitted				0.694							0.998	
Satd, Flow (perm)	0	1792	1470	1312	1891	0	0	0	0	0	3550	1591
Right Turn on Red	-		Yes			Yes	-	-	Yes	-		Yes
Satd, Flow (RTOR)			49									35
Link Speed (mph)		30			30			30			30	
Link Distance (ff)		777			330			770			660	
Travel Time (s)		17.7			7.5			17.5			15.0	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adi, Flow (vph)	0	98	49	11	33	0	0	0	0	60	1277	54
Shared Lane Traffic (%)	-					-	-	-	-			
Lane Group Flow (vph)	0	98	49	11	33	0	0	0	0	0	1337	54
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12	0 -		12	J •		0	J -		0	J -
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.01	1.05	1.10	0.98	0.98	0.98	1.02	1.02	1.02	0.99	0.99	0.99
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors		2	1	1	2					1	2	1
Detector Template		Thru	Right	Left	Thru					Left	Thru	Right
Leading Detector (ft)		100	20	20	100					20	100	20
Trailing Detector (ft)		0	0	0	0					0	0	0
Detector 1 Position(ft)		0	0	0	0					0	0	0
Detector 1 Size(ft)		6	20	20	6					20	6	20
Detector 1 Type		CI+Ex	CI+Ex	CI+Ex	CI+Ex					Cl+Ex	Cl+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 1 Queue (s)		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 1 Delay (s)		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		CI+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA	Perm	Perm	NA					Perm	NA	Perm
Detector 1 Delay (s) Detector 2 Position(ft) Detector 2 Size(ft) Detector 2 Type Detector 2 Channel Detector 2 Extend (s) Turn Type		0.0 94 6 Cl+Ex 0.0 NA	0.0 Perm	0.0 Perm	0.0 94 6 Cl+Ex 0.0 NA					0.0 Perm	0.0 94 6 Cl+Ex 0.0 NA	0.0 Perm

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Lanes, Volumes, Timings 47: S Center St & W Olive St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases		4			8						6	
Permitted Phases			4	8						6		6
Detector Phase		4	4	8	8					6	6	6
Switch Phase												
Minimum Initial (s)		5.0	5.0	5.0	5.0					5.0	5.0	5.0
Minimum Split (s)		13.5	13.5	13.5	13.5					37.9	37.9	37.9
Total Split (s)		18.0	18.0	18.0	18.0					62.0	62.0	62.0
Total Split (%)		22.5%	22.5%	22.5%	22.5%					77.5%	77.5%	77.5%
Maximum Green (s)		12.5	12.5	12.5	12.5					56.1	56.1	56.1
Yellow Time (s)		3.2	3.2	3.2	3.2					3.2	3.2	3.2
All-Red Time (s)		2.3	2.3	2.3	2.3					2.7	2.7	2.7
Lost Time Adjust (s)		0.0	0.0	0.0	0.0						0.0	0.0
Total Lost Time (s)		5.5	5.5	5.5	5.5						5.9	5.9
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0	3.0	3.0	3.0					3.0	3.0	3.0
Recall Mode		Max	Max	None	None					C-Max	C-Max	C-Max
Walk Time (s)		1.0	1.0	1.0	1.0					21.0	21.0	21.0
Flash Dont Walk (s)		7.0	7.0	7.0	7.0					11.0	11.0	11.0
Pedestrian Calls (#/hr)		0	0	0	0					0	0	0
Act Effct Green (s)		12.5	12.5	12.5	12.5						56.1	56.1
Actuated g/C Ratio		0.16	0.16	0.16	0.16						0.70	0.70
v/c Ratio		0.35	0.18	0.05	0.11						0.54	0.05
Control Delay		34.2	11.2	32.2	32.0						4.1	0.7
Queue Delay		0.0	0.0	0.0	0.0						0.0	0.0
Total Delay		34.2	11.2	32.2	32.0						4.1	0.7
LOS		С	В	С	С						А	А
Approach Delay		26.5			32.1						4.0	
Approach LOS		С			С						А	
90th %ile Green (s)		12.5	12.5	12.5	12.5					56.1	56.1	56.1
90th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
70th %ile Green (s)		12.5	12.5	12.5	12.5					56.1	56.1	56.1
70th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
50th %ile Green (s)		12.5	12.5	12.5	12.5					56.1	56.1	56.1
50th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
30th %ile Green (s)		12.5	12.5	12.5	12.5					56.1	56.1	56.1
30th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
10th %ile Green (s)		12.5	12.5	12.5	12.5					56.1	56.1	56.1
10th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
Stops (vph)		79	13	12	30						308	3
Fuel Used(gal)		2	0	0	0						9	0
CO Emissions (g/hr)		112	31	11	31						633	20
NOx Emissions (g/hr)		22	6	2	6						123	4
VOC Emissions (g/hr)		26	7	3	7						147	5
Dilemma Vehicles (#)		0	0	0	0						0	0
Queue Length 50th (ft)		44	0	5	16						91	1
Queue Length 95th (ft)		89	29	m17	m40						46	m1
Internal Link Dist (ft)		697			250			690			580	
Turn Bay Length (ft)			80									50

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Lanes, Volumes, Timings 47: S Center St & W Olive St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Base Capacity (vph)		280	271	205	295					-	2489	1126
Starvation Cap Reductn		0	0	0	0						0	0
Spillback Cap Reductn		0	0	0	0						0	0
Storage Cap Reductn		0	0	0	0						0	0
Reduced v/c Ratio		0.35	0.18	0.05	0.11						0.54	0.05
Intersection Summary												
Area Type: O	ther											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 16 (20%), Referenced	to phase	2: and 6:	SBTL, Sta	art of Gree	en							
Natural Cycle: 55												
Control Type: Actuated-Coord	dinated											
Maximum v/c Ratio: 0.54												
Intersection Signal Delay: 6.9				In	tersectior	LOS: A						
Intersection Capacity Utilization	on 56.5%			IC	U Level o	of Service	В					
Analysis Period (min) 15												
m Volume for 95th percentil	e queue is	s metereo	l by upstr	eam sign	al.							
Splits and Phases: 47: S Co	enter St &	W Olive	St									
												25



Lanes, Volumes, Timings 48: S East St & E Olive St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		÷.			≜ î≽			đ î je				
Traffic Volume (vph)	95	70	0	0	40	30	20	855	15	0	0	0
Future Volume (vph)	95	70	0	0	40	30	20	855	15	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	12	12	12	12	12	12	12	12	12
Grade (%)		2%			-4%			4%			-4%	
Storage Length (ft)	0		0	0		0	300		0	0		0
Storage Lanes	0		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00
Frt					0.935			0.998				
Flt Protected		0.972						0.999				
Satd, Flow (prot)	0	1672	0	0	3279	0	0	3359	0	0	0	0
Flt Permitted		0.777						0.999				
Satd, Flow (perm)	0	1336	0	0	3279	0	0	3359	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					33			4				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		330			647			682			633	
Travel Time (s)		7.5			14.7			15.5			14.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%
Parking (#/hr)	0,0	0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0
Adi, Flow (vph)	103	76	0	0	43	33	22	929	16	0	0	0
Shared Lane Traffic (%)			•	· ·						•	•	•
Lane Group Flow (vph)	0	179	0	0	76	0	0	967	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0		_0.1	0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.01	1.07	1.01	0.97	0.97	0.97	1.03	1.03	1.03	0.97	0.97	0.97
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors	1	2			2		1	2				
Detector Template	Left	Thru			Thru		l eft	Thru				
Leading Detector (ft)	20	100			100		20	100				
Trailing Detector (ft)	0	0			0		0	0				
Detector 1 Position(ft)	0	0			0		0	0				
Detector 1 Size(ft)	20	6			6		20	6				
Detector 1 Type	CI+Ex	Cl+Ex			CI+Ex		CI+Ex	Cl+Fx				
Detector 1 Channel	OI: EX	OI LA			OFFER		OI LA	OT LA				
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0				
Detector 2 Position(ft)	0.0	0.0 Q/			Q/1		0.0	Q <u>/</u>				
Detector 2 Size/ft)		6			6			6				
Detector 2 Tupe												
Detector 2 Channel												

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 48: S East St & E Olive St

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Lane Group EBL EBT EBR WBL WBR NBL NBT NBR SBL SBT SBR Detector 2 Extend (s) 0.0 0.0 0.0 0.0 0.0 0.0 Tum Type Perm NA NA Perm NA NA <td< th=""><th></th><th>٨</th><th>-</th><th>7</th><th>1</th><th>+</th><th>*</th><th>1</th><th>Ť</th><th>1</th><th>4</th><th>ţ</th><th>~</th></td<>		٨	-	7	1	+	*	1	Ť	1	4	ţ	~
Detector 2 Extend (s) 0.0 0.0 0.0 Turn Type Permited Phases 4 NA Permited Phases Permited Phases 4 8 2 Detector Phase 4 8 2 Swite Phase 4 8 2 Swite Phase 4 8 2 Minimum Initial (s) 5.0 5.0 5.0 Total Split (s) 19.5 19.5 58.6 58.6 Total Split (s) 21.0 25.0 25.0 55.0 55.0 Total Split (s) 31.3% 31.3% 31.3% 68.8% Maximum Green (s) 19.5 19.5 49.4 49.4 Vellow Time (s) 3.2 2.3 2.3 2.3 2.4 2.4 Lot Time Aquit (s) 0.0 0.0 0.0 0.0 0.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 </td <td>Lane Group</td> <td>EBL</td> <td>EBT</td> <td>EBR</td> <td>WBL</td> <td>WBT</td> <td>WBR</td> <td>NBL</td> <td>NBT</td> <td>NBR</td> <td>SBL</td> <td>SBT</td> <td>SBR</td>	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type Perm NA NA Perm NA Protected Phases 4 8 2 2 Detector Phase 4 4 8 2 2 Detector Phase 4 4 8 2 2 Minimum Spit(s) 19.5 19.5 59.6 55.0 55.0 Total Spit(s) 25.0 25.0 25.0 55.0 55.0 Total Spit(s) 31.3% 31.3% 68.8% 68.8% Maximum Green (s) 19.5 19.5 49.4 49.4 Velice Time (s) 3.2 3.2 3.2 3.2 3.2 Lead/Lag 0.0 0.0 0.0 0.0 10.0 Total Lost Time (s) 3.0 3.0 3.0 3.0 3.0 3.0 Recall Mode Max Max None C-Max C-Max Vehice Extension (s) 3.0 3.0 3.0 3.0 3.0 Lead/Lag 0.10	Detector 2 Extend (s)		0.0			0.0			0.0				
Protector Phases 4 8 2 Permitted Phases 4 2 Switch Phase	Turn Type	Perm	NA			NA		Perm	NA				
Permited Phases 4 4 8 2 Detector Phase 4 4 8 2 Minimum Initial (s) 5.0 5.0 5.0 5.0 Minimum Initial (s) 5.0 5.0 5.0 5.0 Total Split (s) 25.0 25.0 25.0 55.0 Total Split (s) 31.3% 31.3% 68.8% 68.8% Maximum Green (s) 19.5 19.5 49.4 49.4 Valor Time (s) 3.2 3.2 3.2 3.2 3.2 Lack Lag Optimize (s) 0.0 0.0 0.0 10.0 10.0 Total Lost Time (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lead-Lag Lead-Lag Lead-Lag C-Max C-Max Max Lead-Lag (Shirize?	Protected Phases		4			8			2				
Detector Phase 4 4 8 2 2 Switch Phase Switch Phase Switch Phase Switch Phase Switch Phase Minimum Split (s) 19.5 50. 5.0 5.0 Sol Total Split (s) 25.0 25.0 25.0 55.0 Sol Sol Total Split (%) 31.3% 31.3% Sol 32.0 Sol Sol Vallow Time (s) 2.3 2.3 2.3 2.3 Sol Sol Ustor Time (s) 2.3 2.3 2.3 2.3 Sol Sol Lead-Lag Optimize? Velicle Extension (s) 3.0 3.0 3.0 Sol Sol Velicle Extension (s) 3.0 3.0 3.0 3.0 A2.0 42.0 Flesh Dont Walk (s) 11.0 11.0 11.0 11.0 11.0 11.0 Podestrian Calls (Mr) 0 0 0 0 O A2.4 C62 Wick Raio 0.0 Concot Delay	Permitted Phases	4						2					
Switch Phase Source Minimum Initial (s) 5.0 5.0 5.0 5.0 Total Spitt (s) 25.0 25.0 25.0 55.0 55.0 Total Spitt (s) 31.3% 33.3% 31.3% 68.8% 68.8% Maximum Green (s) 19.5 19.5 49.4 49.4 Velice Time (s) 3.2 3.2 3.2 3.2 3.2 AlR-ed Time (s) 5.5 5.5 5.6 1.6 1.6 Lead-Lag Optimize? Velice Extension (s) 3.0 3.0 3.0 3.0 Velice Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Tead Uptimize? Velice Extension (s) 3.0 3.0 3.0 42.0 42.0 Tead Velice (stims on (s) 3.0 3.0 3.0 42.0 42.0 42.0 Lead-Lag Optimize? 0.0 0 0 0 0 0 0 1.0 11.0 11.0 11.0 11.0 <	Detector Phase	4	4			8		2	2				
$\begin{array}{l l l l l l l l l l l l l l l l l l l $	Switch Phase												
Minimum Split (s) 19.5 19.5 19.5 58.6 58.6 Total Split (s) 25.0 25.0 25.0 55.0 55.0 Total Split (s) 31.3% 31.3% 31.3% 68.8% 68.8% Maximum Green (s) 19.5 19.5 49.4 49.4 49.4 Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 AlRed Time (s) 3.2 3.2 3.2 3.2 3.2 Last Time Adjust (s) 0.0 0.0 0.0 0.0 Lead-Lag Optimize?	Minimum Initial (s)	5.0	5.0			5.0		5.0	5.0				
Total Split (s) 25.0 25.0 25.0 55.0 Total Split (%) 31.3% 31.3% 38.8% 68.8% Maximum Green (s) 19.5 19.5 19.5 49.4 49.4 Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 Lost Time Agust (s) 0.0 0.0 0.0 0.0 Total Split (s) 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.5 5.5 5.6 5.6 Lead-Lag Optimize? Vehicle Extension (s) 3.0 3.0 3.0 3.0 Recall Mode Max Max None C-Max C-Max Walk Time (s) 3.0 3.0 3.0 42.0 42.0 Feash Dort Walk (s) 11.0 11.0 11.0 11.0 11.0 Pedestrian Calls (#mr) 0 0 0 0 0 0 Queue Delay 0.0 0.0 0.0 0.0 10 10 10 Queue Delay 32.0 15.5 9.1 49.4 49.4	Minimum Split (s)	19.5	19.5			19.5		58.6	58.6				
Total Split (%) 31.3% 31.3% 31.3% 68.8% 68.8% Maximum Green (s) 19.5 19.5 19.5 49.4 49.4 Vellow Time (s) 3.2 3.2 3.2 3.2 3.2 All-Red Time (s) 2.3 2.3 2.3 2.4 2.4 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.5 5.5 5.6 Lead-Lag Optimize?	Total Split (s)	25.0	25.0			25.0		55.0	55.0				
Maximum Green (s) 19.5 19.5 19.5 49.4 49.4 Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 AlR-Red Time (s) 2.3 2.3 2.3 2.4 2.4 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.5 5.5 5.6 Lead/Lag Detimine (s) 3.0 3.0 3.0 3.0 Recail Mode Max Max None C-Max C-Max Walk Time (s) 3.0 3.0 3.0 42.0 42.0 Flash Dont Walk (s) 11.0 11.0 11.0 11.0 11.0 Pedestrian Calls (#htr) 0 0 0 0 0 0 Queue Delay 32.0 15.5 9.1 Queue Delay 0.2 15.5 9.1 LOS C B A A A A A Obl %ile Green (s) 19.5 19.5 19.5 9.1 A A Obl %ile Green (s) 19.5 19.5	Total Split (%)	31.3%	31.3%			31.3%		68.8%	68.8%				
Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 3.2 All-Red Time (s) 2.3 2.3 2.4 2.4 2.4 Lost Time A(g)us(s) 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.5 5.5 5.6 Lead-Lag Optimize? Vehicle Extension (s) 3.0 3.0 3.0 3.0 Walk Time (s) 3.0 3.0 3.0 4.2.0 42.0 Flash Dont Walk (s) 11.0 11.0 11.0 11.0 11.0 Pedestrian Calls (#hr) 0 0 0 0 0 Actuated g/C Ratio 0.24 0.24 0.62 v/c Ratio 0.0 Control Delay 3.2.0 15.5 9.1 0.0 0.0 0.0 Control Delay 3.2.0 15.5 9.1 0.0 0.0 0.0 Oth %ile Green (s) 19.5 19.5 49.4 49.4 0.0 0.0 0.0 0.0 0.0 0.0	Maximum Green (s)	19.5	19.5			19.5		49.4	49.4				
All-Red Time (s) 2.3 2.3 2.3 2.4 2.4 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.5 5.5 5.6 Lead-Lag Optimize? Vehicle Extension (s) 3.0 3.0 3.0 3.0 Recall Mode Max Max None C-Max C-Max Walk Time (s) 3.0 3.0 3.0 42.0 42.0 Flash Dont Walk (s) 11.0 11.0 11.0 11.0 11.0 Podestrian Calls (#hr) 0 0 0 0 0 0 Ve Ratio 0.55 0.09 0.47 0.0 0.0 0.0 Control Delay 32.0 15.5 9.1 0.0 0.0 0.0 Colab Delay 32.0 15.5 9.1 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Yellow Time (s)	3.2	3.2			3.2		3.2	3.2				
Lost Time Adjust (s) 0.0 0.0 0.0 Total Lost Time (s) 5.5 5.5 5.6 Lead/Lag Lead/Lag Lead/Lag Lead/Lag Lead/Lag Optimize? Vehicle Extension (s) 3.0 3.0 3.0 3.0 Recall Mode Max None C-Max C-Max Walx Walk Time (s) 3.0 3.0 3.0 42.0 42.0 Flash Dont Walk (s) 11.0 11.0 11.0 11.0 11.0 Acturated g/C Ratio 0.24 0.24 0.62 .062 .062 Vic Ratio 0.55 0.09 0.47 .00 .00 .00 Control Delay 32.0 15.5 9.1 .00 <	All-Red Time (s)	2.3	2.3			2.3		2.4	2.4				
Total Lost Time (s) 5.5 5.5 5.6 Lead/Lag Optimize?	Lost Time Adjust (s)	-	0.0			0.0			0.0				
Lead/Lag No No No Lead/Lag Optimize? Vehicle Extension (s) 3.0 3.0 3.0 3.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Recall Mode Max Max None C-Max C-Max Walk Time (s) 3.0 3.0 3.0 42.0 42.0 Flash Dont Walk (s) 11.0 11.0 11.0 11.0 Pedestrian Calls (#/hr) 0 0 0 0 Actuated g/C Ratio 0.24 0.24 0.62	Total Lost Time (s)		5.5			5.5			5.6				
Lead-Lag Optimize? Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Recall Mode Max Max None C-Max C-Max Walk Time (s) 3.0 3.0 3.0 42.0 42.0 Flash Dont Walk (s) 11.0 11.0 11.0 11.0 11.0 Pedestrian Calls (#hrr) 0 0 0 0 0 Actuated g/C Ratio 0.24 0.24 0.62	Lead/Lag		0.0			0.0			0.0				
Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Recall Mode Max Max None C-Max C-Max Walk Time (s) 3.0 3.0 3.0 3.0 3.0 42.0 Elash Dont Walk (s) 11.0 11.0 11.0 11.0 11.0 11.0 Pedestrian Calls (#hr) 0 0 0 0 0 0 0 Actuated g/C Ratio 0.24 0.24 0.62 0.62 0.62 Vic Ratio 0.55 0.09 0.47 0.00 0.0 0.0 Control Delay 32.0 15.5 9.1 0.00 0.0 0.0 Call Delay 32.0 15.5 9.1 0.05 0.0 0.0 LOS C B A A 0.00 0.0 0.0 Oth %ile Green (s) 19.5 19.5 19.5 49.4 49.4 0.0 Oth %ile Green (s) 19.5 19.5 19.	Lead-Lag Optimize?												
Recail Mode Max Nane C-Max C-Max Walk Time (s) 3.0 3.0 3.0 42.0 42.0 Flash Dont Walk (s) 11.0 11.0 11.0 11.0 11.0 Pedestrian Calls (#hr) 0 0 0 0 0 0 Actuated g/C Ratio 0.24 0.24 0.62 v/c Ratio 0.55 0.09 0.47 Control Delay 32.0 15.5 9.1 0 <	Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0				
Walk Time (s) 3.0 3.0 3.0 42.0 42.0 Flash Dont Walk (s) 11.0 11.0 11.0 11.0 11.0 Pedestrian Calls (#hr) 0 0 0 0 0 Act Effet Green (s) 19.5 19.5 49.4 Actuated g/C Ratio 0.24 0.24 0.62 v/c Ratio 0.55 0.09 0.47 Control Delay 32.0 15.5 9.1 Queue Delay 0.0 0.0 0.0 Total Delay 32.0 15.5 9.1 LOS C B A Approach LOS C B A 90th %ile Green (s) 19.5 19.5 19.4 49.4 90th %ile Green (s) 19.5 19.5 49.4 49.4 90th %ile Green (s) 19.5 19.5	Recall Mode	Max	Max			None		C-Max	C-Max				
Flash Dort Walk (s) 11.0 11.0 11.0 11.0 11.0 Pedestrian Calls (#hr) 0 0 0 0 0 Act Effct Green (s) 19.5 19.5 49.4 Actuated g/C Ratio 0.24 0.24 0.62 Vic Ratio 0.55 0.09 0.47 Control Delay 32.0 15.5 9.1 Queue Delay 0.0 0.0 0.0 Total Delay 32.0 15.5 9.1 LOS C B A Approach LOS C B A 90th %ile Green (s) 19.5 19.5 49.4 90th %ile Term Code MaxR MaxR Hold Coord 50th %ile Green (s) 19.5 19.5 49.4 49.4 50th %ile Term Code MaxR MaxR Hold Coord Coord 50th	Walk Time (s)	3.0	3.0			3.0		42.0	42.0				
Index Form Coll (s) Index Index Index Index Pedestrian Calls (#hr) 0 0 0 0 0 0 Actuated g/C Ratio 0.24 0.24 0.62	Flash Dont Walk (s)	11.0	11.0			11.0		11.0	11.0				
Act Effc Green (s) 19.5 19.5 49.4 Actuated g/C Ratio 0.24 0.24 0.62 V/c Ratio 0.55 0.09 0.47 Control Delay 32.0 15.5 9.1 Queue Delay 0.0 0.0 0.0 Total Delay 32.0 15.5 9.1 LOS C B A Approach Delay 32.0 15.5 9.1 LOS C B A Approach LOS C B A 90th %ile Green (s) 19.5 19.5 49.4 90th %ile Green (s) 19.5 19.5 <td< td=""><td>Pedestrian Calls (#/hr)</td><td>0</td><td>0</td><td></td><td></td><td>0</td><td></td><td>0</td><td>0</td><td></td><td></td><td></td><td></td></td<>	Pedestrian Calls (#/hr)	0	0			0		0	0				
Actuated g/C Ratio 0.24 0.24 0.62 v/c Ratio 0.55 0.09 0.47 Control Delay 32.0 15.5 9.1 Queue Delay 0.0 0.0 0.0 Total Delay 32.0 15.5 9.1 LOS C B A Approach Delay 32.0 15.5 9.1 LOS C B A Approach LOS C B A 90th %ile Green (s) 19.5 19.5 49.4 90th %ile Green (s) 19.5 19.5 49.4 90th %ile Green (s) 19.5 19.5 49.4 70th %ile Green (s) 19.5 19.5 <t< td=""><td>Act Effct Green (s)</td><td>•</td><td>19.5</td><td></td><td></td><td>19.5</td><td></td><td>Ū</td><td>49.4</td><td></td><td></td><td></td><td></td></t<>	Act Effct Green (s)	•	19.5			19.5		Ū	49.4				
vic Ratio 0.55 0.09 0.47 Control Delay 32.0 15.5 9.1 Queue Delay 0.0 0.0 0.0 Total Delay 32.0 15.5 9.1 LOS C B A Approach Delay 32.0 15.5 9.1 LOS C B A Approach LOS C B A 90th %ile Green (s) 19.5 19.5 19.4 90th %ile Green (s) 19.5 19.5 49.4 90th %ile Green (s) 19.5 19.5 <	Actuated g/C Ratio		0.24			0.24			0.62				
Notation Notation Notation Notation Queue Delay 0.0 0.0 0.0 Queue Delay 32.0 15.5 9.1 LOS C B A Approach Delay 32.0 15.5 9.1 LOS C B A Approach LOS C B A 90th %ile Green (s) 19.5 19.5 49.4 70th %ile Green (s) 19.5 19.5 49.4 50th %ile Green (s) 19.5 19.5 49.4 30th %ile Green (s) 19.	v/c Ratio		0.55			0.09			0.47				
Outer Delay O.0 O.0 O.0 Total Delay 32.0 15.5 9.1 LOS C B A Approach Delay 32.0 15.5 9.1 LOS C B A Approach Delay 32.0 15.5 9.1 Approach LOS C B A 90th %ile Green (s) 19.5 19.5 49.4 90th %ile Green (s) 19.5 19.5 49.4 90th %ile Green (s) 19.5 19.5 49.4 70th %ile Green (s) 19.5 19.5 49.4 70th %ile Green (s) 19.5 19.5 49.4 50th %ile Green (s) 19.5 19.5 49.4 30th %ile Green (s) 19.5 19.5 49.4 30th %ile Term Code MaxR MaxR Hold Coord 30th %ile Term Code MaxR MaxR Hold Coord Coord 30th %ile Term Code MaxR MaxR Hold <	Control Delay		32.0			15.5			91				
Total Delay 32.0 15.5 9.1 LOS C B A Approach Delay 32.0 15.5 9.1 Approach LOS C B A 90th %ile Green (s) 19.5 19.5 49.4 49.4 70th %ile Green (s) 19.5 19.5 49.4 49.4 50th %ile Green (s) 19.5 19.5 49.4 49.4 50th %ile Green (s) 19.5 19.5 49.4 49.4 50th %ile Green (s) 19.5 19.5 49.4 49.4 30th %ile Green (s) 19.5 19.5 19.5 49.4 30th %ile Green (s) 19.5 19.5 49.4 49.4 30th %ile Green (s) 19.5 19.5 49.4 49.4 10th %i	Queue Delay		0.0			0.0			0.0				
LOS C B A Approach Delay 32.0 15.5 9.1 Approach LOS C B A 90th %ile Green (s) 19.5 19.5 19.5 49.4 70th %ile Green (s) 19.5 19.5 49.4 49.4 50th %ile Green (s) 19.5 19.5 49.4 49.4 30th %ile Green (s) 19.5 19.5 49.4 49.4 10th %ile Green (s) 19.5 19.5 49.4 49.4 10th %ile Term Code MaxR MaxR Hold Coord	Total Delay		32.0			15.5			9.1				
Approach Delay 32.0 15.5 9.1 Approach LOS C B A 90th %ile Green (s) 19.5 19.5 49.4 49.4 70th %ile Green (s) 19.5 19.5 49.4 49.4 70th %ile Green (s) 19.5 19.5 49.4 49.4 70th %ile Green (s) 19.5 19.5 49.4 49.4 50th %ile Green (s) 19.5 19.5 49.4 49.4 50th %ile Green (s) 19.5 19.5 49.4 49.4 30th %ile Green (s) 19.5 19.5 49.4 49.4 30th %ile Term Code MaxR MaxR Hold Coord Coord 10th %ile Green (s) 19.5 19.5 49.4 49.4 10th %ile Term Code MaxR MaxR Hold Coord Coord Stops (vph)	LOS		C			B			A				
Approach LOS C B A 90th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 90th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 90th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 70th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 70th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 50th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 50th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 50th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 30th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 30th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 10th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 10th %ile Green (s) 19.5 19.5 49.4 49.4 10th %ile Green (s) 19.5 19.5 49.4 40.4	Approach Delay		32.0			15.5			9.1				
90th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 90th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 90th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 70th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 70th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 70th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 50th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 50th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 30th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 30th %ile Green (s) 19.5 19.5 49.4 49.4 10th %ile Green (s) 19.5 19.5 49.4 <t< td=""><td>Approach LOS</td><td></td><td>C</td><td></td><td></td><td>B</td><td></td><td></td><td>A</td><td></td><td></td><td></td><td></td></t<>	Approach LOS		C			B			A				
Oth Max Max Hold Coord Coord 90th %ile Term Code Max Max Hold Coord Coord Coord Toth Vile Green (s) 19.5 19.5 49.4 49.4 Toth Sourd Coord Coord <t< td=""><td>90th %ile Green (s)</td><td>19.5</td><td>19.5</td><td></td><td></td><td>19.5</td><td></td><td>49.4</td><td>49.4</td><td></td><td></td><td></td><td></td></t<>	90th %ile Green (s)	19.5	19.5			19.5		49.4	49.4				
70th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 70th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 70th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 50th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 50th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 50th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 30th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 30th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 30th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 10th %ile Green (s) 19.5 19.5 49.4 40.4 10th %ile Green (s) 19.5 34 440 4	90th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
Note Toto Toto Toto Toto 70th %ile Term Code MaxR MaxR Hold Coord Coord 50th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 50th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 30th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 30th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 30th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 30th %ile Green (s) 19.5 19.5 49.4 49.4 10th %ile Green (s) 19.5 19.5 49.4 49.4 10th %ile Green (s) 19.5 19.5 49.4 49.4 10th %ile Term Code MaxR MaxR Hold Coord Coord 10th %ile Term Code MaxR MaxR Hold Coord Coord Stops (vph) 152 34 440 Fuel Used(gal) 2 1 9 CO Emissions (g/hr) 32 10 12	70th %ile Green (s)	19.5	19.5			19.5		49.4	49.4				
50th Wile Green (s) 19.5 19.5 19.5 49.4 49.4 50th Wile Term Code MaxR MaxR Hold Coord Coord 30th Wile Green (s) 19.5 19.5 19.5 49.4 49.4 30th Wile Green (s) 19.5 19.5 19.5 49.4 49.4 30th Wile Green (s) 19.5 19.5 19.5 49.4 49.4 30th Wile Green (s) 19.5 19.5 19.5 49.4 49.4 10th Wile Green (s) 19.5 19.5 19.5 49.4 49.4 10th Wile Green (s) 19.5 19.5 19.5 49.4 49.4 10th Wile Term Code MaxR MaxR Hold Coord Coord Stops (vph) 152 34 440 Fuel Used(gal) 2 1 9 CO Emissions (g/hr) 163 53 616 Nox Emissions (g/hr) 32 10 120 VOC Emissions (g/hr) 38 12 143 143 143 144 Dilemma Vehicles (#) 0 0 0 <td>70th %ile Term Code</td> <td>MaxR</td> <td>MaxR</td> <td></td> <td></td> <td>Hold</td> <td></td> <td>Coord</td> <td>Coord</td> <td></td> <td></td> <td></td> <td></td>	70th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
South Wile Term Code MaxR MaxR Hold Coord Coord 30th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 30th %ile Term Code MaxR MaxR Hold Coord Coord 30th %ile Term Code MaxR MaxR Hold Coord Coord 10th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 10th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 10th %ile Term Code MaxR MaxR Hold Coord Coord Stops (vph) 152 34 440 Fuel Used(gal) 2 1 9 CO Emissions (g/hr) 163 53 616 O O 120 VOC Emissions (g/hr) 38 12 143 O O 0 Oliemma Vehicles (#) 0 0 0 0 0 0 Queue Length 50th (ft) 87 8 121 00 0	50th %ile Green (s)	19.5	19.5			19.5		49.4	49.4				
30th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 30th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 30th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 10th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 10th %ile Green (s) 19.5 19.5 19.5 49.4 49.4 10th %ile Term Code MaxR MaxR Hold Coord Coord Stops (vph) 152 34 440 440 Fuel Used(gal) 2 1 9 9 CO Emissions (g/hr) 163 53 616 NOx Emissions (g/hr) 32 10 120 VOC Emissions (g/hr) 38 12 143 Dilemma Vehicles (#) 0 0 0 0 Queue Length 50th (ft) 87 8 121 142	50th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
South Write Term Code MaxR MaxR Hold Coord Coord 10th Write Term Code MaxR MaxR Hold Coord Coord 10th Write Term Code MaxR MaxR Hold Coord Coord 10th Write Term Code MaxR MaxR Hold Coord Coord Stops (vph) 152 34 440 Fuel Used(gal) 2 1 9 CO Emissions (g/hr) 163 53 616 O O 120 VOC Emissions (g/hr) 38 12 143 O O 0 Ueue Length 50th (ft) 87 8 121 143 143 143	30th %ile Green (s)	19.5	19.5			19.5		49.4	49.4				
10th %ile Green (s) 19.5 19.5 19.5 19.5 49.4 49.4 10th %ile Term Code MaxR MaxR Hold Coord Coord Stops (vph) 152 34 440 Fuel Used(gal) 2 1 9 CO Emissions (g/hr) 163 53 616 NOx Emissions (g/hr) 32 10 120 VOC Emissions (g/hr) 38 12 143 Dilemma Vehicles (#) 0 0 0 0 0 0 Queue Length 50th (ft) 87 8 121 163 <td< td=""><td>30th %ile Term Code</td><td>MaxR</td><td>MaxR</td><td></td><td></td><td>Hold</td><td></td><td>Coord</td><td>Coord</td><td></td><td></td><td></td><td></td></td<>	30th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
Note of our of the form Note of our of the form Note of our of the form 10th %ile Term Code MaxR MaxR Hold Coord Coord Stops (vph) 152 34 440 Fuel Used(gal) 2 1 9 CO Emissions (g/hr) 163 53 616 NOx Emissions (g/hr) 32 10 120 VOC Emissions (g/hr) 38 12 143 Dilemma Vehicles (#) 0 0 0 Queue Length 50th (ft) 87 8 121	10th %ile Green (s)	19.5	19.5			19.5		49.4	49.4				
Story (vph) 152 34 440 Fuel Used(gal) 2 1 9 CO Emissions (g/hr) 163 53 616 NOx Emissions (g/hr) 32 10 120 VOC Emissions (g/hr) 38 12 143 Dilemma Vehicles (#) 0 0 0 Queue Length 50th (ft) 87 8 121	10th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
Fuel Used(gal) 2 1 9 CO Emissions (g/hr) 163 53 616 NOx Emissions (g/hr) 32 10 120 VOC Emissions (g/hr) 38 12 143 Dilemma Vehicles (#) 0 0 0 Queue Length 50th (ft) 87 8 121	Stops (vph)		152			34		00014	440				
CO Emissions (g/hr) 163 53 616 NOx Emissions (g/hr) 32 10 120 VOC Emissions (g/hr) 38 12 143 Dilemma Vehicles (#) 0 0 0 Queue Length 50th (ft) 87 8 121	Fuel Used(gal)		2			1							
NOx Emissions (g/hr) 32 10 120 VOC Emissions (g/hr) 38 12 143 Dilemma Vehicles (#) 0 0 0 Queue Length 50th (ft) 87 8 121	CO Emissions (g/hr)		163			53			616				
VOC Emissions (g/hr) 38 12 143 Dilemma Vehicles (#) 0 0 0 Queue Length 50th (ft) 87 8 121	NOx Emissions (g/hr)		.32			10			120				
Dilemma Vehicles (#) 0 0 0 Queue Length 50th (ft) 87 8 121	VOC Emissions (g/hr)		38			10			143				
Queue Length 50th (ft) 87 8 121 Queue Length 06th (ft) 152 05 100	Dilemma Vehicles (#)		0			0			0				
	Queue Length 50th (ff)		87			8			121				
Queue Lengin 95th (11) 152 25 162	Queue Length 95th (ft)		152			25			162				

2041 PM-IDOT Downtown Bloomington 4:07 pm 12/16/2022 2 lane option BSE

Lanes, Volumes, Timings 48: S East St & E Olive St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		250			567			602		-	553	
Turn Bay Length (ft)												
Base Capacity (vph)		325			824			2075				
Starvation Cap Reductn		0			0			0				
Spillback Cap Reductn		0			0			0				
Storage Cap Reductn		0			0			0				
Reduced v/c Ratio		0.55			0.09			0.47				
Intersection Summary												
Area Type:	Other											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 54 (68%), Reference	d to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 80												
Control Type: Actuated-Coo	rdinated											
Maximum v/c Ratio: 0.55												
Intersection Signal Delay: 12	2.8			In	tersectior	LOS: B						
Intersection Capacity Utiliza	tion 49.6%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
Splits and Phases: 48: S	East St & E	Olive St						-				

Ø2 (R)	
55 s	25 s
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	25 s

02/22/2024

Lanes, Volumes, Timings 3: N Madison St/N Center St & W Locust St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑ ⊅			र्स						-fî	7
Traffic Volume (vph)	0	459	57	4	51	0	0	0	0	164	1041	148
Future Volume (vph)	0	459	57	4	51	0	0	0	0	164	1041	148
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	0		0	0		150
Storage Lanes	0		0	0		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00
Ped Bike Factor		1.00									1.00	
Frt		0.983										0.850
Flt Protected					0.996						0.993	
Satd. Flow (prot)	0	3407	0	0	1820	0	0	0	0	0	3447	1553
Flt Permitted					0.951						0.993	
Satd. Flow (perm)	0	3407	0	0	1737	0	0	0	0	0	3445	1553
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		19										180
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		659			244			860			526	
Travel Time (s)		15.0			5.5			19.5			12.0	
Confl. Peds. (#/hr)			2							4		
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	560	70	5	62	0	0	0	0	200	1270	180
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	630	0	0	67	0	0	0	0	0	1470	180
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0	Ŭ		0	Ŭ		0	Ŭ		0	Ŭ
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	1
Detector Template		Thru		Left	Thru					Left	Thru	Right
Leading Detector (ft)		100		20	100					20	100	20
Trailing Detector (ft)		0		0	0					0	0	0
Detector 1 Position(ft)		0		0	0					0	0	0
Detector 1 Size(ft)		6		20	6					20	6	20
Detector 1 Type		Cl+Ex		CI+Ex	CI+Ex					CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		Cl+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 3: N Madison St/N Center St & W Locust St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type		NA		Perm	NA					Perm	NA	Perm
Protected Phases		4			8						6	
Permitted Phases				8						6		6
Detector Phase		4		8	8					6	6	6
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	5.0
Minimum Split (s)		23.4		22.5	22.5					23.4	23.4	23.4
Total Split (s)		23.7		22.5	22.5					46.3	46.3	46.3
Total Split (%)		33.9%		32.1%	32.1%					66.1%	66.1%	66.1%
Maximum Green (s)		18.3		18.0	18.0					40.9	40.9	40.9
Yellow Time (s)		3.2		3.5	3.5					3.2	3.2	3.2
All-Red Time (s)		2.2		1.0	1.0					2.2	2.2	2.2
Lost Time Adjust (s)		0.0			0.0						0.0	0.0
Total Lost Time (s)		5.4			4.5						5.4	5.4
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	3.0
Recall Mode		None		None	None					C-Max	C-Max	C-Max
Walk Time (s)		7.0		7.0	7.0					7.0	7.0	7.0
Flash Dont Walk (s)		11.0		11.0	11.0					11.0	11.0	11.0
Pedestrian Calls (#/hr)		0		0	0					0	0	0
Act Effct Green (s)		16.9			17.8						42.3	42.3
Actuated g/C Ratio		0.24			0.25						0.60	0.60
v/c Ratio		0.75			0.15						0.71	0.18
Control Delay		30.0			16.1						12.3	1.6
Queue Delay		52.8			0.0						0.0	0.0
Total Delay		82.8			16.1						12.3	1.6
LOS		F			В						В	А
Approach Delay		82.8			16.1						11.2	
Approach LOS		F			В						В	
90th %ile Green (s)		18.3		19.2	19.2					40.9	40.9	40.9
90th %ile Term Code		Max		Hold	Hold					Coord	Coord	Coord
70th %ile Green (s)		18.3		19.2	19.2					40.9	40.9	40.9
70th %ile Term Code		Max		Hold	Hold					Coord	Coord	Coord
50th %ile Green (s)		18.3		19.2	19.2					40.9	40.9	40.9
50th %ile Term Code		Max		Hold	Hold					Coord	Coord	Coord
30th %ile Green (s)		16.6		17.5	17.5					42.6	42.6	42.6
30th %ile Term Code		Gap		Hold	Hold					Coord	Coord	Coord
10th %ile Green (s)		13.1		14.0	14.0					46.1	46.1	46.1
10th %ile Term Code		Gap		Hold	Hold					Coord	Coord	Coord
Stops (vph)		445			48						787	12
Fuel Used(gal)		8			1						12	1
CO Emissions (g/hr)		578			38						861	51
NOx Emissions (g/hr)		112			7						167	10
VOC Emissions (g/hr)		134			9						200	12
Dilemma Vehicles (#)		0			0						0	0
Queue Length 50th (ft)		124			26						216	0
Queue Length 95th (ft)		158			m42						245	17
Internal Link Dist (ft)		579			164			780			446	

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 3: N Madison St/N Center St & W Locust St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												150
Base Capacity (vph)		904			476						2080	1009
Starvation Cap Reductn		0			0						0	0
Spillback Cap Reductn		367			0						0	0
Storage Cap Reductn		0			0						0	0
Reduced v/c Ratio		1.17			0.14						0.71	0.18
Intersection Summary												
Area Type: C)ther											
Cycle Length: 70												
Actuated Cycle Length: 70												
Offset: 10 (14%), Referenced	l to phase	2: and 6:8	SBTL, Sta	art of Gree	en							
Natural Cycle: 60												
Control Type: Actuated-Coord	dinated											
Maximum v/c Ratio: 0.75												
Intersection Signal Delay: 30.	.5			In	tersectior	LOS: C						
Intersection Capacity Utilizati	on 57.1%			IC	U Level o	of Service	В					
Analysis Period (min) 15												
m Volume for 95th percenti	le queue i	s metered	l by upstr	eam sign	al.							
Splits and Phases: 3: N Ma	adison St/I	V Center	St & W Lo	ocust St				164				20

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	23.7 s	
Ø6 (R)	₩ Ø8	
46.3 s	22.5 s	

Lanes, Volumes, Timings <u>6: N East St/N Main St & W Locust St/E Locust St</u>

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		-€‡						đ þ				
Traffic Volume (vph)	92	528	0	0	0	0	56	875	103	0	0	0
Future Volume (vph)	92	528	0	0	0	0	56	875	103	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	0.95	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor		1.00										
Frt								0.985				
Flt Protected		0.993						0.997				
Satd. Flow (prot)	0	3447	0	0	0	0	0	3409	0	0	0	0
Flt Permitted		0.993						0.997				
Satd. Flow (perm)	0	3446	0	0	0	0	0	3409	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)								27				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		244			514			878			525	
Travel Time (s)		5.5			11.7			20.0			11.9	
Confl. Peds. (#/hr)	2											
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Adi, Flow (vph)	106	607	0	0	0	0	64	1006	118	0	0	0
Shared Lane Traffic (%)			•	•	•	, in the second s	• .			•	•	·
Lane Group Flow (vph)	0	713	0	0	0	0	0	1188	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0	0 -		0	J *		0	J -		0	5
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2					1	2				
Detector Template	Left	Thru					Left	Thru				
Leading Detector (ft)	20	100					20	100				
Trailing Detector (ft)	0	0					0	0				
Detector 1 Position(ft)	0	0					0	0				
Detector 1 Size(ft)	20	6					20	6				
Detector 1 Type	CI+Ex	Cl+Ex					Cl+Ex	CI+Ex				
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0				
Detector 1 Queue (s)	0.0	0.0					0.0	0.0				
Detector 1 Delay (s)	0.0	0.0					0.0	0.0				
Detector 2 Position(ft)		94						94				
Detector 2 Size(ft)		6						6				
Detector 2 Type		Cl+Ex						CI+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0				
Turn Type	Perm	NA					Perm	NA				
Protected Phases		4					,	2				
Permitted Phases	4						2					

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings
6: N East St/N Main St & W Locust St/E Locust St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4					2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0					5.0	5.0				
Minimum Split (s)	23.5	23.5					23.5	23.5				
Total Split (s)	26.0	26.0					44.0	44.0				
Total Split (%)	37.1%	37.1%					62.9%	62.9%				
Maximum Green (s)	20.5	20.5					38.5	38.5				
Yellow Time (s)	3.2	3.2					3.2	3.2				
All-Red Time (s)	2.3	2.3					2.3	2.3				
Lost Time Adjust (s)		0.0						0.0				
Total Lost Time (s)		5.5						5.5				
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0				
Recall Mode	None	None					C-Max	C-Max				
Walk Time (s)	7.0	7.0					7.0	7.0				
Flash Dont Walk (s)	11.0	11.0					11.0	11.0				
Pedestrian Calls (#/hr)	0	0					0	0				
Act Effct Green (s)	Ţ	18.9					Ū.	40.1				
Actuated g/C Ratio		0.27						0.57				
v/c Ratio		0.77						0.60				
Control Delay		19.3						7.2				
Queue Delav		46.9						0.0				
Total Delay		66.3						7.2				
LOS		E						A				
Approach Delay		66.3						7.2				
Approach LOS		E						A				
90th %ile Green (s)	20.5	20.5					38.5	38.5				
90th %ile Term Code	Max	Max					Coord	Coord				
70th %ile Green (s)	20.5	20.5					38.5	38.5				
70th %ile Term Code	Max	Max					Coord	Coord				
50th %ile Green (s)	20.5	20.5					38.5	38.5				
50th %ile Term Code	Max	Max					Coord	Coord				
30th %ile Green (s)	18.3	18.3					40.7	40.7				
30th %ile Term Code	Gap	Gap					Coord	Coord				
10th %ile Green (s)	14.7	14.7					44.3	44.3				
10th %ile Term Code	Gap	Gap					Coord	Coord				
Stops (vph)		584						450				
Fuel Used(gal)		7						11				
CO Emissions (g/hr)		479						774				
NOx Emissions (g/hr)		93						151				
VOC Emissions (g/hr)		111						179				
Dilemma Vehicles (#)		0						0				
Queue Length 50th (ft)		167						61				
Queue Length 95th (ft)		217						72				
Internal Link Dist (ft)		164			434			798			445	
Turn Bay Length (ft)												
Base Capacity (vph)		1009						1964				
Starvation Cap Reductn		354						0				

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings
6: N East St/N Main St & W Locust St/E Locust St

02/22/2024	
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Spillback Cap Reductn		0						0				
Storage Cap Reductn		0						0				
Reduced v/c Ratio		1.09						0.60				
Intersection Summary												
Area Type:	Other											
Cycle Length: 70												
Actuated Cycle Length: 70)											
Offset: 23 (33%), Referen	ced to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 55												
Control Type: Actuated-Co	oordinated											
Maximum v/c Ratio: 0.77												
Intersection Signal Delay:	29.4			In	tersectior	n LOS: C						
Intersection Capacity Utilization 55.5%					U Level o	of Service	В					
Analysis Period (min) 15												
Solits and Phases: 6: N	East St/N Ma	ain St & V	/ Locust \$	St/E Locu	st St							

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Lanes, Volumes, Timings 9: N Madison St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,		7	†						đ î þ	
Traffic Volume (vph)	0	71	89	11	83	0	0	0	0	7	833	93
Future Volume (vph)	0	71	89	11	83	0	0	0	0	7	833	93
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	11	12	12	11	12	12	12	12	12	12	12
Grade (%)		1%			-2%			-6%			4%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		0.99		1.00								
Frt		0.925									0.985	
Flt Protected				0.950								
Satd. Flow (prot)	0	1447	0	1578	1605	0	0	0	0	0	3016	0
Flt Permitted				0.496								
Satd. Flow (perm)	0	1447	0	820	1605	0	0	0	0	0	3016	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		83									29	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		705			247			400			860	
Travel Time (s)		16.0			5.6			9.1			19.5	
Confl. Peds. (#/hr)			3	3								
Confl. Bikes (#/hr)			1									
Peak Hour Factor	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74	0.74
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Adj. Flow (vph)	0	96	120	15	112	0	0	0	0	9	1126	126
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	216	0	15	112	0	0	0	0	0	1261	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.15	1.20	1.15	1.13	1.18	1.13	1.10	1.10	1.10	1.17	1.17	1.17
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		Cl+Ex	Cl+Ex					Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		Cl+Ex			Cl+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 9: N Madison St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		15.4		15.4	15.4					47.2	47.2	
Total Split (s)		24.0		24.0	24.0					46.0	46.0	
Total Split (%)		34.3%		34.3%	34.3%					65.7%	65.7%	
Maximum Green (s)		18.6		18.6	18.6					40.8	40.8	
Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)		2.2		2.2	2.2					2.0	2.0	
Lost Time Adjust (s)		0.0		0.0	0.0						0.0	
Total Lost Time (s)		5.4		5.4	5.4						5.2	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		None	None					C-Max	C-Max	
Walk Time (s)		1.0		1.0	1.0					31.0	31.0	
Flash Dont Walk (s)		9.0		9.0	9.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		12.2		12.2	12.2					•	47.2	
Actuated g/C Ratio		0.17		0.17	0.17						0.67	
v/c Ratio		0.68		0.11	0.40						0.62	
Control Delay		26.4		23.0	28.3						2.0	
Queue Delav		0.0		0.0	0.0						0.0	
Total Delay		26.4		23.0	28.3						2.0	
LOS		С		C	C						A	
Approach Delay		26.4		-	27.7						2.0	
Approach LOS		С			С						A	
90th %ile Green (s)		18.6		18.6	18.6					40.8	40.8	
90th %ile Term Code		Max		Hold	Hold					Coord	Coord	
70th %ile Green (s)		14.8		14.8	14.8					44.6	44.6	
70th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
50th %ile Green (s)		12.1		12.1	12.1					47.3	47.3	
50th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
30th %ile Green (s)		9.3		9.3	9.3					50.1	50.1	
30th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
10th %ile Green (s)		6.1		6.1	6.1					53.3	53.3	
10th %ile Term Code		Hold		Hold	Hold					Coord	Coord	
Stops (vph)		90		11	68						62	
Fuel Used(gal)		2		0	1						7	
CO Emissions (a/hr)		156		9	71						488	
NOx Emissions (a/hr)		30		2	14						95	
VOC Emissions (a/hr)		36		2	16						113	
Dilemma Vehicles (#)		0		0	0						0	
Queue Length 50th (ff)		53		6	43						20	
Queue Length 95th (ft)		76		15	62						16	
Internal Link Dist (ft)		625			167			320			780	

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 9: N Madison St & W Market St

02/22/2024	ŀ
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)		445		217	426						2043	
Starvation Cap Reductn		0		0	0						0	
Spillback Cap Reductn		0		0	0						0	
Storage Cap Reductn		0		0	0						0	
Reduced v/c Ratio		0.49		0.07	0.26						0.62	
Intersection Summary												
Area Type: Cł	3D											
Cycle Length: 70												
Actuated Cycle Length: 70												
Offset: 28 (40%), Referenced	to phase	2: and 6:8	SBTL, Sta	art of Gre	en							
Natural Cycle: 65												
Control Type: Actuated-Coord	inated											
Maximum v/c Ratio: 0.68												
Intersection Signal Delay: 7.3				In	tersectior	n LOS: A						
Intersection Capacity Utilization	on 48.3%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
Splits and Phases: 9: N Ma	dison St &	& W Mark	et St									

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46 s	24 s	

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	*			41		0011
Traffic Volume (vnh)	65	0	88	951	0	0
Future Volume (vph)	65	0	88	951	0	0
I deal Flow (vphpl)	1000	1000	1000	1000	1000	1000
Lana Width (ft)	1300	1300	1300	1300	1300	1300
	10	12	12	00/	۲۲ ۵۵/	12
	0%	1.00	0.05	-2%	Z %	1.00
Lane Ulli. Factor	1.00	1.00	0.95	0.95	1.00	1.00
				1.00		
Frt	0.050			0.000		
Fit Protected	0.950		_	0.996	_	
Satd. Flow (prot)	1467	0	0	2761	0	0
Flt Permitted	0.950			0.996		
Satd. Flow (perm)	1467	0	0	2760	0	0
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)						
Link Speed (mph)	30			30	30	
Link Distance (ft)	263			127	878	
Travel Time (s)	60			2.9	20.0	
Confl Peds (#/hr)	0.0		1	2.0	20.0	
Peak Hour Factor	0.82	0.82	0.82	0.82	በ 82	0.82
Heavy Vehicles (%)	3%	3%	30/2	3%	3%	20/2
Derking (#/br)	570	J /0	270	2/0	J /0	J /0
Adi Elaw (wah)	70	0	20	20	0	0
	79	0	107	1160	0	0
Shared Lane Traffic (%)			_		_	
Lane Group Flow (vph)	79	0	0	1267	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	13			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.25	1.14	1.13	1.34	1,16	1.16
Turning Speed (mph)	15	9	15			9
Number of Detectors	1	5	1	2		5
Namber of Delectors	Loft		Loft	Thru		
				100		
	20		20	100		
Trailing Detector (ft)	0		0	0		
Detector 1 Position(ft)	0		0	0		
Detector 1 Size(ft)	20		20	6		
Detector 1 Type	CI+Ex		Cl+Ex	CI+Ex		
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0		
Detector 1 Queue (s)	0.0		0.0	0.0		
Detector 1 Delay (s)	0.0		0.0	0.0		
Detector 2 Position(ft)				94		
Detector 2 Size(ft)				6		
Detector 2 Type				CI+Ev		
Detector 2 Channel						
Detector 2 Originite				0.0		
Delector 2 Extend (S)				0.0		

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Turn Type	Prot		Perm	NA			
Protected Phases	4			2			
Permitted Phases			2				
Detector Phase	4		2	2			
Switch Phase							
Minimum Initial (s)	5.0		5.0	5.0			
Minimum Split (s)	13.4		49.5	49.5			
Total Split (s)	16.0		54.0	54.0			
Total Split (%)	22.9%		77.1%	77.1%			
Maximum Green (s)	10.6		48.5	48.5			
Yellow Time (s)	3.2		3.2	3.2			
All-Red Time (s)	2.2		2.3	2.3			
Lost Time Adjust (s)	0.0			0.0			
Total Lost Time (s)	5.4			5.5			
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)	3.0		3.0	3.0			
Recall Mode	None		C-Max	C-Max			
Walk Time (s)	1.0		33.0	33.0			
Flash Dont Walk (s)	7.0		11.0	11.0			
Pedestrian Calls (#/hr)	0		0	0			
Act Effct Green (s)	8.7		-	53.7			
Actuated g/C Ratio	0.12			0.77			
v/c Ratio	0.43			0.60			
Control Delay	35.4			6.5			
Queue Delav	0.0			0.0			
Total Delay	35.4			6.5			
LOS	D			А			
Approach Delay	35.4			6.5			
Approach LOS	D			А			
90th %ile Green (s)	10.6		48.5	48.5			
90th %ile Term Code	Max		Coord	Coord			
70th %ile Green (s)	10.6		48.5	48.5			
70th %ile Term Code	Max		Coord	Coord			
50th %ile Green (s)	9.1		50.0	50.0			
50th %ile Term Code	Gap		Coord	Coord			
30th %ile Green (s)	7.6		51.5	51.5			
30th %ile Term Code	Gap		Coord	Coord			
10th %ile Green (s)	0.0		64.5	64.5			
10th %ile Term Code	Skip		Coord	Coord			
Stops (vph)	58			452			
Fuel Used(gal)	1			5			
CO Emissions (g/hr)	64			343			
NOx Emissions (a/hr)	13			67			
VOC Emissions (g/hr)	15			79			
Dilemma Vehicles (#)	0			0			
Queue Length 50th (ft)	32			119			
Queue Length 95th (ft)	62			157			
Internal Link Dist (ft)	183			47	798		

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Turn Bay Length (ft)						
Base Capacity (vph)	222			2117		
Starvation Cap Reductn	0			0		
Spillback Cap Reductn	0			0		
Storage Cap Reductn	0			0		
Reduced v/c Ratio	0.36			0.60		
Intersection Summary						
Area Type:	CBD					
Cycle Length: 70						
Actuated Cycle Length: 70						
Offset: 4 (6%), Referenced	to phase 2:N	IBTL and	6:, Start	of Green		
Natural Cycle: 65						
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.60						
Intersection Signal Delay:	8.2			Int	ersection	LOS: A
Intersection Capacity Utiliz	ation 45.3%			IC	U Level o	f Service A
Analysis Period (min) 15						

Splits and Phases: 11: N East St & E Market St

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54 s	16 s	

Lanes, Volumes, Timings 12: N Madison St & W Monroe St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,			र्स						đ þ	
Traffic Volume (vph)	0	10	0	7	4	0	0	0	0	19	904	10
Future Volume (vph)	0	10	0	7	4	0	0	0	0	19	904	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	11	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			-3%			6%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt											0.998	
Flt Protected					0.969						0.999	
Satd. Flow (prot)	0	1605	0	0	1609	0	0	0	0	0	3051	0
Flt Permitted					0.969						0.999	
Satd. Flow (perm)	0	1605	0	0	1609	0	0	0	0	0	3051	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		699			209			307			400	
Travel Time (s)		15.9			4.8			7.0			9.1	
Confl. Peds. (#/hr)			3	3						4		1
Confl. Bikes (#/hr)			1									
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	0	13	0	9	5	0	0	0	0	25	1174	13
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	13	0	0	14	0	0	0	0	0	1212	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.19	1.14	1.14	1.14	1.14	1.12	1.12	1.12	1.19	1.19	1.19
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Free	
Intersection Summary												
Area Type: C	BD											
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 42.5%			IC	CU Level	of Service	A					
Analysis Period (min) 15												

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	5			≜ t⊾		
Traffic Volume (vph)	3	0	31	1021	0	0
Future Volume (vph)	3	0	31	1021	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	11	12	12
Grade (%)	2%	12	12	3%	12	12
	-2 /0 1 00	1.00	0.05	-3 /0	4 /0	1.00
Lane Ulli. Faciul Dod Piko Eastor	1.00	1.00	0.95	1.00	1.00	1.00
Feu DIKE FAULUI				1.00		
FIL Elt Danta etc.el	0.050			0.000		
	0.950	0	0	0.999	0	0
Sata. Flow (prot)	1434	U	U	2783	U	U
Fit Permitted	0.950	_		0.999		
Satd. Flow (perm)	1434	0	0	2783	0	0
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)						
Link Speed (mph)	30			30	30	
Link Distance (ft)	221			318	266	
Travel Time (s)	5.0			7.2	6.0	
Confl. Peds. (#/hr)			1		0.0	
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%
Parking (#/br)	0.0	J /0	20	20	J /0	570
Adi Flow (uph)	0	0	20	1045	٥	0
	4	0	30	1240	0	0
Shared Lane Traffic (%)		<u>,</u>	<u>^</u>	4000	<u>,</u>	<u>^</u>
Lane Group Flow (vph)	4	0	0	1283	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.29	1.13	1.12	1.34	1.17	1.17
Turning Speed (mph)	15	9	15			9
Number of Detectors	1	J	1	2		0
Detector Template	۱ toft		off	∠ Thru		
Loading Detector (ff)	20		20	100		
Trailing Detector (II)	20		20	100		
Trailing Detector (ft)	U		U	0		
Detector 1 Position(ft)	0		0	0		
Detector 1 Size(tt)	20		20	6		
Detector 1 Type	CI+Ex		Cl+Ex	Cl+Ex		
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0		
Detector 1 Queue (s)	0.0		0.0	0.0		
Detector 1 Delay (s)	0.0		0.0	0.0		
Detector 2 Position(ft)				94		
Detector 2 Size(ft)				6		
Detector 2 Type				CI+Ex		
Detector 2 Channel						
Detector 2 Extend (c)				0.0		
Delector 2 Extend (S)				0.0		

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Turn Type	Prot		Perm	NA			
Protected Phases	4			2			
Permitted Phases			2				
Detector Phase	4		2	2			
Switch Phase							
Minimum Initial (s)	5.0		5.0	5.0			
Minimum Split (s)	13.2		49.2	49.2			
Total Split (s)	15.0		65.0	65.0			
Total Split (%)	18.8%		81.3%	81.3%			
Maximum Green (s)	9.8		59.8	59.8			
Yellow Time (s)	3.2		3.2	3.2			
All-Red Time (s)	2.0		2.0	2.0			
Lost Time Adjust (s)	0.0			0.0			
Total Lost Time (s)	52			5.2			
Lead/Lag	0.2			0.2			
Lead-Lag Optimize?							
Vehicle Extension (s)	3.0		3.0	3.0			
Recall Mode	None		C-Max	C-Max			
Walk Time (s)	10		33.0	33.0			
Flash Dont Walk (s)	7.0		11.0	11.0			
Pedestrian Calls (#/hr)	0		0	0			
Act Effet Green (s)	59		U	76.6			
Actuated a/C Ratio	0.07			0.96			
v/c Ratio	0.07			0.30			
Control Delay	35.0			0.40			
	0.0			0.0			
Total Delay	35.0			0.0			
	00.0 C			0.5			
Annroach Delay	35.0			0.5			
Approach LOS	00.0			Δ			
Approach 200	66		63.0	63.0			
90th %ile Term Code	Gan		Coord	Coord			
70th %ile Green (s)	0.0		7/ 8	7/ 8			
70th %ile Term Code	Skin		Coord	Coord			
50th %ile Green (s)			7/ 8	7/ 8			
50th %ile Term Code	Skin		Coord	Coord			
30th %ile Green (s)			7/ 8	7/ 8			
30th %ile Term Code	0.0 Skip		Coord	Coord			
10th %ile Green (s)			7/ 9	7/1 9			
10th %ile Term Code	Skin		Coord	Coord			
Stope (uph)	Б		COOld	200010			
Stops (vpr)	0			3			
CO Emissions (a/br)	1			ن 101			
NOv Emissions (g/III)	4			191			
NOX Emissions (g/nr)	1			31			
VOC Emissions (g/nr)				44			
	0			0			
Queue Length 50th (ft)	2			0			
Queue Length 95th (ft)	10			1	100		
Internal Link Dist (ft)	141			238	186		

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

1 2 2 2 1 4 4

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Turn Bay Length (ft)							
Base Capacity (vph)	175			2665			
Starvation Cap Reductn	0			9			
Spillback Cap Reductn	0			0			
Storage Cap Reductn	0			0			
Reduced v/c Ratio	0.02			0.48			
Intersection Summary							
Area Type:	CBD						
Cycle Length: 80							
Actuated Cycle Length: 80							
Offset: 61 (76%), Reference	ed to phase 2	2:NBTL a	nd 6:, Sta	art of Gree	en		
Natural Cycle: 65							
Control Type: Actuated-Co	ordinated						
Maximum v/c Ratio: 0.48							
Intersection Signal Delay: (0.6			In	tersection	LOS: A	
Intersection Capacity Utiliz	ation 45.2%			IC	U Level o	f Service A	
Analysis Period (min) 15							
Splits and Phases: 13: N	I East St & M	onroe St					

Ø2 (R)	1	<u></u>	22
65 s		15 s	

Lanes, Volumes, Timings 15: N Madison St & W Jefferson St

02/22/2024

Lane Gongung EBL EBT EBR WBL WBR NBL NBT NBR SBL SBT SBR Lane Configurations 1 4 13 0 0 0 0 13 903 12 Future Volume (vph) 0 17 4 4 13 0 0 0 0 13 903 12 Lane Volume (vph) 100 100 100 100 100 100 100 1900 1313 0 0 0 0 0 1313 0 0 0		٠	→	7	4	+	*	1	Ť	1	4	ŧ	~
Lane Configurations p	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph) 0 17 4 4 13 0 0 0 13 903 12 Ideal Flow (vphp) 1900	Lane Configurations		1÷			र्स						4 P	
Fulure Volume (vph) 0 17 4 4 13 0 0 0 13 903 12 Glael Flow (vph) 1900 100 1.0	Traffic Volume (vph)	0	17	4	4	13	0	0	0	0	13	903	12
Ideal Flow (vphp) 1900 12 13 0 1300 1301 1301 1301 1301 1301 1301 1301 1301 1301 1	Future Volume (vph)	0	17	4	4	13	0	0	0	0	13	903	12
Lane Width (ft) 12 16 12 <th13< th=""> 03 13</th13<>	Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%) 4% -3% 2% 2% 2% Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 1.00 0.95	Lane Width (ft)	12	16	12	12	14	12	12	12	12	12	12	12
Lane Uli Factor 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0	Grade (%)		4%			-3%			2%			2%	
Ped Bike Factor 1.00 1.00 .0.975 .0.998 Fit Protected 0.989 0.999 0.999 .0.998 Satd. Flow (prot) 0 1791 0 0 0 0 0 0 0 3113 0 Satd. Flow (prot) 0 1791 0 0 1528 0 0 0 0 3113 0 Right Turn on Red Yes Yes </td <td>Lane Util. Factor</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>0.95</td> <td>0.95</td> <td>0.95</td>	Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Frit 0.975 0.989 0.999 Filt Protected 0.989 0.999 0.999 Stad. Flow (prot) 0 1791 0 0 160 0 0 0 0.999 Stad. Flow (prot) 0 1791 0 0 1528 0 0 0 0 3113 0 Righ Tum on Red Yes Yes Yes Yes Yes Yes Yes Link Speed (mph) 30 30 30 300 <td>Ped Bike Factor</td> <td></td> <td>1.00</td> <td></td> <td></td> <td>1.00</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	Ped Bike Factor		1.00			1.00							
File Protected 0.989 0.000 0.000 0.999 Satd. Flow (prot) 0.1791 0.000 0.1600 0.000 0.000 0.1133 0.000 Satd. Flow (perm) 0.1791 0.000 1528 0.000 0.000 0.00000 3113 0.000 Righ Tum on Red Yes Yes Yes Yes Yes Yes Yes Satd. Flow (RTOR) 5	Frt		0.975									0.998	
Satel Flow (prot) 0 1791 0 0 1600 0 0 0 3113 0 File Permitted 0 944 0 0.999 0 0.999 Satel, Flow (perm) 0 1791 0 0 1528 0 0 0 0 0.999 Right Turn on Red Yes Yes Yes Yes Yes Yes Yes Yes Satel, Flow (prot) 3 3 30 30 30 30 30 Link Speed (mph) 30 3 3 7.0 7.0 7.0 Confi. Peds. (#/hr) 3 3 3 3% <	Flt Protected					0.989						0.999	
FILP Emitted 0.946 0.999 Satd. Flow (perm) 0 1791 0 0 1528 0 0 0 0 3113 0 Satd. Flow (RTOR) 5 Yes Yes Yes Yes Yes Yes Yes Satd. Flow (RTOR) 5 Yes Yes Yes Yes 4 Link Speed (mph) 30 30 30 30 30 30 Confl. Peds. (#hr) 62 253 310 0.76<	Satd. Flow (prot)	0	1791	0	0	1600	0	0	0	0	0	3113	0
Satid, Flow (perm) 0 1791 0 0 1528 0 0 0 0 3113 0 Right Tum on Red Yes Yes Yes Yes Yes Yes Yes Link Speed (mph) 30 30 30 30 30 30 Link Distance (ft) 692 253 310 307 Travel Time (s) 157 5.8 7.0 7.0 Confl. Peds. (#/hr) 3 3 7.0 0.76	Flt Permitted					0.946						0.999	
Right Turu on Red Yes Yes Yes Yes Yes Yes Sald. Flow (RTOR) 5	Satd. Flow (perm)	0	1791	0	0	1528	0	0	0	0	0	3113	0
Said. Flow (RTOR) 5 4 Link Speed (mph) 30 30 30 30 Link Distance (ft) 692 253 310 307 Travel Time (s) 15.7 5.8 7.0 7.0 Peak Hour Factor 0.76 <t< td=""><td>Right Turn on Red</td><td></td><td></td><td>Yes</td><td></td><td></td><td>Yes</td><td></td><td></td><td>Yes</td><td></td><td></td><td>Yes</td></t<>	Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph) 30 30 30 30 30 Link Distance (ft) 692 253 310 307 Travel Time (s) 15.7 5.8 7.0 7.0 Confl. Peds. (#hr) 3 3 7.0 7.0 7.0 Peak Hour Factor 0.76	Satd. Flow (RTOR)		5									4	
Link Distance (ft) 692 253 310 307 Travel Time (s) 15.7 5.8 7.0 7.0 Confl. Peds. (#hr) 3 3 7.0 7.0 Peak Hour Factor 0.76	Link Speed (mph)		30			30			30			30	
Travel Time (s) 15.7 5.8 7.0 7.0 Confl. Peds. (#hr) 3 3 3 7.0 7.0 Peak Hour Factor 0.76	Link Distance (ft)		692			253			310			307	
Confl. Peds. (#/hr) 3 3 Peak Hour Factor 0.76 0.7	Travel Time (s)		15.7			5.8			7.0			7.0	
Peak Hour Factor 0.76	Confl. Peds. (#/hr)			3	3								
Heavy Vehicles (%) 3% 3	Peak Hour Factor	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76
Parking (#/hr) 0 20 20 20 Adj. Flow (vph) 0 22 5 5 17 0 0 0 17 1188 16 Shared Lane Traffic (%) 0 27 0 0 22 0 0 0 112 10 Enter Blocked Intersection No	Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Adj. Flow (vph) 0 22 5 5 17 0 0 0 17 1188 16 Shared Lane Traffic (%)	Parking (#/hr)					0					20		20
Shared Lane Traffic (%) Lane Group Flow (vph) 0 27 0 0 22 0 0 0 0 121 0 Enter Blocked Intersection No No <td>Adj. Flow (vph)</td> <td>0</td> <td>22</td> <td>5</td> <td>5</td> <td>17</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>17</td> <td>1188</td> <td>16</td>	Adj. Flow (vph)	0	22	5	5	17	0	0	0	0	17	1188	16
Lane Group Flow (vph) 0 27 0 0 22 0 0 0 0 121 0 Enter Blocked Intersection No	Shared Lane Traffic (%)												
Enter Blocked Intersection No No <th< td=""><td>Lane Group Flow (vph)</td><td>0</td><td>27</td><td>0</td><td>0</td><td>22</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>1221</td><td>0</td></th<>	Lane Group Flow (vph)	0	27	0	0	22	0	0	0	0	0	1221	0
Lane Alignment Left Left Right Median Width(ft) 10 0 0 12 12 12 Link Offset(ft) 0 <td>Enter Blocked Intersection</td> <td>No</td>	Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Median Width(ft) 0 0 12 12 12 Link Offset(ft) 0	Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Link Offset(ft) 0 0 0 0 0 Crosswalk Width(ft) 16 16 16 16 16 Two way Left Turn Lane	Median Width(ft)		0			0	-		12			12	-
Crosswalk Width(ft) 16 16 16 16 16 Two way Left Turn Lane	Link Offset(ft)		0			0			0			0	
Two way Left Turn Lane Headway Factor 1.17 1.00 1.17 1.12 1.18 1.12 1.16 1.17 1.12 1.12 1.12 1.16 1.16 1.16 1.16 1.16 1.16 1.16 1.16 1.16 1.16 1.16 1.16 1.16 1.16	Crosswalk Width(ft)		16			16			16			16	
Headway Factor 1.17 1.00 1.17 1.12 1.18 1.12 1.16	Two way Left Turn Lane												
Turning Speed (mph) 15 9 15 20 15 20 15 20 15 10 20 100	Headway Factor	1.17	1.00	1.17	1.12	1.18	1.12	1.16	1.16	1.16	1.16	1.16	1.16
Number of Detectors 2 1 2 1 2 Detector Template Thru Left Thru Left Thru Leading Detector (ft) 100 20 100 20 100 Trailing Detector (ft) 0 0 0 0 0 0 Detector 1 Position(ft) 0 0 0 0 0 0 0 0 Detector 1 Position(ft) 6 20 1 20 1 20	Turning Speed (mph)	15		9	15		9	15		9	15		9
Detector Template Thru Left Thru Left Thru Leading Detector (ft) 100 20 100 20 100 Trailing Detector (ft) 0 <td>Number of Detectors</td> <td></td> <td>2</td> <td></td> <td>1</td> <td>2</td> <td></td> <td></td> <td></td> <td></td> <td>1</td> <td>2</td> <td></td>	Number of Detectors		2		1	2					1	2	
Leading Detector (ft) 100 20 100 20 100 Trailing Detector (ft) 0 <t< td=""><td>Detector Template</td><td></td><td>Thru</td><td></td><td>Left</td><td>Thru</td><td></td><td></td><td></td><td></td><td>Left</td><td>Thru</td><td></td></t<>	Detector Template		Thru		Left	Thru					Left	Thru	
Trailing Detector (ft) 0 0 0 0 0 0 Detector 1 Position(ft) 0 <t< td=""><td>Leading Detector (ft)</td><td></td><td>100</td><td></td><td>20</td><td>100</td><td></td><td></td><td></td><td></td><td>20</td><td>100</td><td></td></t<>	Leading Detector (ft)		100		20	100					20	100	
Detector 1 Position(ft) 0	Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Size(ft) 6 20 6 20 6 Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex Detector Detector 1 Channel 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Detector Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 94 94 Detector 2 Size(ft) 6 6 6 6 Detector 2 Cl+Ex Cl+Ex Cl+Ex Detector 2 Cl+Ex Cl+Ex Detector 2 Channel Detector 2 Channel Detector 2 Extend (s) 0.0	Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex Detector CI+Ex CI+Ex CI+Ex CI+Ex Detector CI+Ex CI+Ex CI+Ex Detector CI+Ex CI+Ex CI+Ex CI+Ex Detector CI+Ex CI+Ex CI+Ex Detector CI+Ex Output Output <td>Detector 1 Size(ft)</td> <td></td> <td>6</td> <td></td> <td>20</td> <td>6</td> <td></td> <td></td> <td></td> <td></td> <td>20</td> <td>6</td> <td></td>	Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Channel Detector 1 Extend (s) 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 94 Detector 2 Size(ft) 6 6 6 Detector 2 Type Cl+Ex Cl+Ex Cl+Ex Detector 2 Channel 0.0 0.0 0.0	Detector 1 Type		CI+Ex		Cl+Ex	CI+Ex					Cl+Ex	CI+Ex	
Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0	Detector 1 Channel												
Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0	Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 94 94 94 0.0 0.0 0.0 Detector 2 Size(ft) 6 6 6 0<	Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft) 94 94 94 94 Detector 2 Size(ft) 6 6 6 Detector 2 Type CI+Ex CI+Ex CI+Ex Detector 2 Channel 0.0 0.0 0.0	Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Size(ft) 6 6 6 Detector 2 Type CI+Ex CI+Ex CI+Ex Detector 2 Channel 0.0 0.0 0.0	Detector 2 Position(ft)		94			94						94	
Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 0.0	Detector 2 Size(ft)		6			6						6	
Detector 2 Channel Detector 2 Extend (s) 0.0 0.0 0.0 0.0	Detector 2 Type		CI+Ex			CI+Ex						CI+Ex	
Detector 2 Extend (s) 0.0 0.0 0.0	Detector 2 Channel												
	Detector 2 Extend (s)		0.0			0.0						0.0	

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Lanes, Volumes, Timings 15: N Madison St & W Jefferson St

Lane Group EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR Turn Type NA Perm NA Perm NA Perm NA Protected Phases 4 8 6 6 6 Permitted Phases 8 6 6 6 Detector Phase 4 8 8 6 6 Switch Phase 5.0 5.0 5.0 5.0 5.0 5.0 Minimum Initial (s) 5.0 5.0 5.0 5.0 5.0 5.0 Minimum Split (s) 15.1 15.4 15.4 47.4 47.4 Total Split (s) 18.0 18.0 18.0 66.6 66.6 Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 3.2 All-Red Time (s) 1.9 2.2 2.2 2.2 2.2 2.2 2.2 2.2 2.2 2.2 2.2<
Turn Type NA Perm NA Perm NA Protected Phases 4 8 6 6 Permitted Phases 8 6 6 Detector Phase 4 8 8 6 Switch Phase 4 8 8 6 6 Switch Phase
Protected Phases 4 8 6 Permitted Phases 8 6 Detector Phase 4 8 8 6 Detector Phase 4 8 8 6 6 Switch Phase 4 8 8 6 6 Minimum Initial (s) 5.0 5.0 5.0 5.0 5.0 Minimum Split (s) 15.1 15.4 15.4 15.4 47.4 47.4 Total Split (s) 18.0 18.0 18.0 62.0 62.0 Total Split (s) 12.9 12.6 12.6 25.6 56.6 56.6 Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 3.2 3.2 All-Red Time (s) 1.9 2.2 <th2< td=""></th2<>
Permitted Phases 8 6 Detector Phase 4 8 8 6 6 Switch Phase 5.0 5.0 5.0 5.0 Minimum Initial (s) 5.0 5.0 5.0 5.0 5.0 Minimum Split (s) 15.1 15.4 15.4 47.4 47.4 Total Split (s) 18.0 18.0 18.0 6 6 Total Split (s) 12.9 12.6 12.6 56.6 <td< td=""></td<>
Detector Phase 4 8 8 6 6 Switch Phase
Switch Phase Minimum Initial (s) 5.0 5.0 5.0 5.0 Minimum Split (s) 15.1 15.4 15.4 47.4 47.4 Total Split (s) 18.0 18.0 18.0 62.0 62.0 Total Split (%) 22.5% 22.5% 22.5% 77.5% 77.5% Maximum Green (s) 12.9 12.6 12.6 56.6 56.6 Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 All-Red Time (s) 1.9 2.2 2.2 2.2 2.2 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0
Minimum Initial (s) 5.0 5.0 5.0 5.0 Minimum Split (s) 15.1 15.4 15.4 47.4 47.4 Total Split (s) 18.0 18.0 18.0 62.0 62.0 Total Split (%) 22.5% 22.5% 22.5% 77.5% 77.5% Maximum Green (s) 12.9 12.6 12.6 56.6 56.6 Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 All-Red Time (s) 1.9 2.2 2.2 2.2 2.2 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0
Minimum Split (s) 15.1 15.4 15.4 15.4 47.4 47.4 Total Split (s) 18.0 18.0 18.0 62.0 62.0 Total Split (s) 22.5% 22.5% 22.5% 77.5% 77.5% Maximum Green (s) 12.9 12.6 12.6 56.6 56.6 Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 All-Red Time (s) 1.9 2.2 2.2 2.2 2.2 Lost Time Adjust (s) 0.0 0.0 0.0 0.0
Total Split (s) 18.0 18.0 18.0 62.0 62.0 Total Split (%) 22.5% 22.5% 22.5% 77.5% Maximum Green (s) 12.9 12.6 12.6 56.6 56.6 Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 All-Red Time (s) 1.9 2.2 2.2 2.2 2.2 Lost Time Adjust (s) 0.0 0.0 0.0 0.0
Total Split (%) 22.5% 22.5% 22.5% 77.5% Maximum Green (s) 12.9 12.6 12.6 56.6 56.6 Yellow Time (s) 3.2 3.2 3.2 3.2 3.2 3.2 All-Red Time (s) 1.9 2.2 2.2 2.2 2.2 2.2 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0
Maximum Green (s) 12.9 12.6 12.6 56.6 56.6 Yellow Time (s) 3.2 <td< td=""></td<>
Yellow Time (s) 3.2 2.2
All-Red Time (s) 1.9 2.2
Lost Time Adjust (s) 0.0 0.0 0.0
10tal Lost Time (s) 5.1 5.4 5.4
Lead/Lag
Lead-Lag Optimize?
Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0
Recall Mode None Max Max C-Max C-Max
Walk Time (s) 1.0 1.0 1.0 31.0 31.0
Flash Dont Walk (s) 9.0 9.0 9.0 11.0 11.0
Pedestrian Calls $(\#/hr)$ 0 0 0 0 0 0
Act Effct Green (s) 12.9 12.6 56.6
Actuated g/C Ratio 0.16 0.16 0.71
v/c Ratio 0.09 0.09 0.55
Control Delay 25.8 30.0 6.8
Queue Delay 0.0 0.0 0.0
Total Delay 25.8 30.0 6.8
LOS C C A
Approach Delay 25.8 30.0 6.8
Approach LOS C C A
90th %ile Green (s) 12.9 12.6 12.6 56.6 56.6
90th %ile Term Code Hold MaxR MaxR Coord Coord
70th %ile Green (s) 12.9 12.6 12.6 56.6 56.6
70th %ile Term Code Hold MaxR MaxR Coord Coord
50th %ile Green (s) 12.9 12.6 12.6 56.6 56.6
50th %ile Term Code Hold MaxR MaxR Coord Coord
30th %ile Green (s) 12.9 12.6 12.6 56.6 56.6
30th %ile Term Code Hold MaxR MaxR Coord Coord
10th %ile Green (s) 12.9 12.6 12.6 56.6 56.6
10th %ile Term Code Hold MaxR MaxR Coord Coord
Stops (vph) 18 16 402
Fuel Used(gal) 0 0 6
CO Emissions (a/hr) 23 16 400
Nox Emissions (q/hr) 4 3 78
VOC Emissions (g/hr) 5 4 93
Dilemma Vehicles (#) 0 0 0
Queue Length 50th (ft) 9 10 127
Queue Length 95th (ft) 26 25 127
Internal Link Dist (ft) 612 173 230 227

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Lanes, Volumes, Timings 15: N Madison St & W Jefferson St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)		292			240						2203	
Starvation Cap Reductn		0			0						0	
Spillback Cap Reductn		0			0						0	
Storage Cap Reductn		0			0						0	
Reduced v/c Ratio		0.09			0.09						0.55	
Intersection Summary												
Area Type: C	BD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 11 (14%), Referenced	to phase 2	2: and 6:8	SBTL, Sta	art of Gre	en							
Natural Cycle: 65												
Control Type: Actuated-Coord	linated											
Maximum v/c Ratio: 0.55												
Intersection Signal Delay: 7.6				In	tersectior	n LOS: A						
Intersection Capacity Utilization	on 42.2%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
Splits and Phases: 15: N M	adison St	& W Jeffe	erson St									

	→ø4	1
	18 s	
▼ Ø6 (R)	T ØR	3
62 s	18 s	

Lanes, Volumes, Timings 16: N East St & E Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्स			P			4 î b				
Traffic Volume (vph)	7	26	0	0	12	4	23	1047	29	0	0	0
Future Volume (vph)	7	26	0	0	12	4	23	1047	29	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	11	11	12	12	12
Grade (%)		-3%			3%			0%			3%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor		0.99			0.99			1.00				
Frt					0.964			0.996				
Flt Protected		0.990						0.999				
Satd. Flow (prot)	0	1501	0	0	1607	0	0	3032	0	0	0	0
Flt Permitted		0.947						0.999				
Satd. Flow (perm)	0	1425	0	0	1607	0	0	3031	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					5			9				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		268			710			308			318	
Travel Time (s)		6.1			16.1			7.0			7.2	
Confl. Peds. (#/hr)	17					17	8		8			
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Parking (#/hr)		0					20		20			
Adj. Flow (vph)	8	31	0	0	14	5	28	1261	35	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	39	0	0	19	0	0	1324	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.12	1.28	1.12	1.17	1.12	1.17	1.14	1.19	1.19	1.17	1.17	1.17
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2			2		1	2				
Detector Template	Left	Thru			Thru		Left	Thru				
Leading Detector (ft)	20	100			100		20	100				
Trailing Detector (ft)	0	0			0		0	0				
Detector 1 Position(ft)	0	0			0		0	0				
Detector 1 Size(ft)	20	6			6		20	6				
Detector 1 Type	Cl+Ex	Cl+Ex			CI+Ex		Cl+Ex	CI+Ex				
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0				
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			Cl+Ex			Cl+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 16: N East St & E Jefferson St

02/22/2024	02	22	20)24
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA			NA		Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4						2					
Detector Phase	4	4			8		2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0		5.0	5.0				
Minimum Split (s)	15.2	15.2			15.2		47.2	47.2				
Total Split (s)	17.0	17.0			17.0		63.0	63.0				
Total Split (%)	21.3%	21.3%			21.3%		78.8%	78.8%				
Maximum Green (s)	11.8	11.8			11.8		57.8	57.8				
Yellow Time (s)	3.2	3.2			3.2		3.2	3.2				
All-Red Time (s)	2.0	2.0			2.0		2.0	2.0				
Lost Time Adjust (s)	2.0	0.0			0.0		2.0	0.0				
Total Lost Time (s)		5.2			5.2			5.2				
Lead/Lag		0.2			0.2			0.2				
Lead-Lag Ontimize?												
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0				
Recall Mode	None	None			Max		C-Max	C-Max				
Walk Time (s)	10	1 0			10		31.0	31.0				
Flash Dont Walk (s)	0.0	0.0			0.0		11.0	11.0				
Pedestrian Calls (#/br)	9.0	9.0			9.0		0	0				
Act Effet Groop (c)	U	11.0			11 0		0	57.8				
Actuated a/C Patio		0.15			0.15			0 72				
Actualed y/C Natio		0.15			0.15			0.72				
V/C Rallo		20.19			0.00			0.00				
Ouque Delay		0.0			25.4			2.0				
Queue Delay		20.0			0.0			0.1				
		52.4			20.4			2.0				
LUS Approach Dolov		20 1			25.4			A 20				
Approach LOS		32.4			20.4			Z.0				
Approach LOS	11 0	11 0			11.0		E7 0	E7 0				
90th %ile Green (S)	11.0 Hold	.0 ∐old			II.0 MovD		0.1C	0.1C				
							57.0	57.0				
70th %ile Green (S)	0.11	0.11			II.0 MaxD		0.1C	0.1C				
							C00rd	C00rd				
South %ile Green (S)	0.11	0.11			II.0 MaxD		0.1C	0.1C				
	HOID	HOID			MaxR			Coord				
30th %ile Green (s)	11.8	11.8			11.8 MauD		57.8	57.8				
	HOID	HOID			MaxR			Coord				
10th %ile Green (s)	11.8	11.8			11.8		57.8	57.8				
10th %ile Term Code	Hold	Hold			MaxR		Coord	Coord				
Stops (vph)		30			13			124				
Fuel Used(gal)		0			0			4				
CO Emissions (g/hr)		31			1/			2/6				
NOx Emissions (g/hr)		6			3			54				
VOC Emissions (g/hr)		7			4			64				
Dilemma Vehicles (#)		0			0			0				
Queue Length 50th (ft)		17			6			43				
Queue Length 95th (ft)		41			23			51				
Internal Link Dist (ft)		188			630			228			238	

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 16: N East St & E Jefferson St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)												
Base Capacity (vph)		210			241			2192				
Starvation Cap Reductn		0			0			95				
Spillback Cap Reductn		0			0			0				
Storage Cap Reductn		0			0			0				
Reduced v/c Ratio		0.19			0.08			0.63				
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 55 (69%), Reference	d to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 65												
Control Type: Actuated-Coo	rdinated											
Maximum v/c Ratio: 0.60												
Intersection Signal Delay: 4.	.0			In	tersectior	n LOS: A						
Intersection Capacity Utiliza	tion 52.0%			IC	U Level o	of Service	А					
Analysis Period (min) 15												

Splits and Phases: 16: N East St & E Jefferson St

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63 s	17 s
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	17 s
Lanes, Volumes, Timings 19: N Madison St & W Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,		7	+					7	† 1 ₂	
Traffic Volume (vph)	0	107	10	38	208	0	0	0	0	41	845	25
Future Volume (vph)	0	107	10	38	208	0	0	0	0	41	845	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	13	12	12	13	12	12	12	12	12	12	12
Grade (%)		3%			-3%			2%			-2%	
Storage Length (ft)	0		0	65		0	0		0	150		0
Storage Lanes	0		0	1		0	0		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95
Frt		0.988									0.996	
Flt Protected				0.950						0.950		
Satd. Flow (prot)	0	1488	0	1585	1552	0	0	0	0	1262	3143	0
Flt Permitted				0.651						0.950		
Satd. Flow (perm)	0	1488	0	1086	1552	0	0	0	0	1262	3143	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		5									7	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		690			253			324			310	
Travel Time (s)		15.7			5.8			7.4			7.0	
Peak Hour Factor	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)		0			0					20		20
Adj. Flow (vph)	0	137	13	49	267	0	0	0	0	53	1083	32
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	150	0	49	267	0	0	0	0	53	1115	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane					Yes							
Headway Factor	1.17	1.28	1.17	1.12	1.23	1.12	1.16	1.16	1.16	1.49	1.13	1.13
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		CI+Ex	CI+Ex					CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		CI+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 19: N Madison St & W Washington St

02/22/	2024
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		15.4		25.4	25.4					37.6	37.6	
Total Split (s)	:	23.0		23.0	23.0					57.0	57.0	
Total Split (%)	28	.8%		28.8%	28.8%					71.3%	71.3%	
Maximum Green (s)		17.6		17.6	17.6					51.4	51.4	
Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)		2.2		2.2	2.2					2.4	2.4	
Lost Time Adjust (s)		0.0		0.0	0.0					0.0	0.0	
Total Lost Time (s)		5.4		5.4	5.4					5.6	5.6	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode	N	lone		Max	Max					C-Max	C-Max	
Walk Time (s)		1.0		9.0	9.0					21.0	21.0	
Flash Dont Walk (s)		9.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		17.6		17.6	17.6					51.4	51.4	
Actuated g/C Ratio		0.22		0.22	0.22					0.64	0.64	
v/c Ratio	(0.45		0.21	0.78					0.07	0.55	
Control Delay	4	31.3		16.8	31.7					5.5	7.4	
Queue Delay		0.0		0.0	1.4					0.0	0.1	
Total Delay	4	31.3		16.8	33.0					5.5	7.4	
LOS		С		В	С					А	А	
Approach Delay	4	31.3			30.5						7.4	
Approach LOS		С			С						А	
90th %ile Green (s)		17.6		17.6	17.6					51.4	51.4	
90th %ile Term Code		Max		Ped	Ped					Coord	Coord	
70th %ile Green (s)		17.6		17.6	17.6					51.4	51.4	
70th %ile Term Code	ł	Hold		Ped	Ped					Coord	Coord	
50th %ile Green (s)		17.6		17.6	17.6					51.4	51.4	
50th %ile Term Code	ł	Hold		Ped	Ped					Coord	Coord	
30th %ile Green (s)		17.6		17.6	17.6					51.4	51.4	
30th %ile Term Code	ł	Hold		Ped	Ped					Coord	Coord	
10th %ile Green (s)		17.6		17.6	17.6					51.4	51.4	
10th %ile Term Code	ŀ	Hold		Ped	Ped					Coord	Coord	
Stops (vph)		96		12	99					13	275	
Fuel Used(gal)		2		0	2					0	5	
CO Emissions (g/hr)		133		19	161					15	344	
NOx Emissions (g/hr)		26		4	31					3	67	
VOC Emissions (g/hr)		31		4	37					4	80	
Dilemma Vehicles (#)		0		0	0					0	0	
Queue Length 50th (ft)		63		4	22					9	102	
Queue Length 95th (ft)		100		22	#133					m16	106	

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 19: N Madison St & W Washington St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		610			173			244			230	
Turn Bay Length (ft)				65						150		
Base Capacity (vph)		331		238	341					810	2021	
Starvation Cap Reductn		0		0	14					0	117	
Spillback Cap Reductn		0		0	0					0	0	
Storage Cap Reductn		0		0	0					0	0	
Reduced v/c Ratio		0.45		0.21	0.82					0.07	0.59	
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 78 (98%), Referenc	ed to phase	2: and 6:	SBTL, Sta	art of Gre	en							
Natural Cycle: 65												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.78												
Intersection Signal Delay: 1	14.0			In	tersectior	n LOS: B						
Intersection Capacity Utilization	ation 48.2%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
# 95th percentile volume	exceeds cap	oacity, qu	eue may	be longer								
Queue shown is maxim	um after two	cycles.										
m Volume for 95th perce	ntile queue i	s metered	l by upstr	eam sign	al.							

Splits and Phases: 19: N Madison St & W Washington St

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	23 s	
₩ Ø6 (R)	₹Ø8	1.20
57 s	23 s	

Lanes, Volumes, Timings 20: N East St & E Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	+			1	1	٦	† 12				
Traffic Volume (vph)	11	189	0	0	261	85	26	993	58	0	0	0
Future Volume (vph)	11	189	0	0	261	85	26	993	58	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	11	10	11	11	11	12	12	12
Grade (%)		-3%			2%			-3%			1%	
Storage Length (ft)	65		0	0		80	80		0	0		0
Storage Lanes	1		0	0		1	1		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	1.00					0.98	1.00	1.00				
Frt						0.850		0.992				
Flt Protected	0.950						0.950					
Satd. Flow (prot)	1585	1502	0	0	1574	1291	1226	3037	0	0	0	0
Flt Permitted	0.352						0.950					
Satd. Flow (perm)	585	1502	0	0	1574	1266	1220	3037	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)						68		13				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		262			724			322			308	
Travel Time (s)		6.0			16.5			7.3			7.0	
Confl. Peds. (#/hr)	5					5	5		2			
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)		0					20		20			
Adj. Flow (vph)	14	236	0	0	326	106	33	1241	73	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	14	236	0	0	326	106	33	1314	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			11			11	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.12	1.28	1.12	1.16	1.21	1.26	1.54	1.17	1.17	1.15	1.15	1.15
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	- 2			- 2	1	1	2				
Detector Template	Left	I hru			I hru	Right	Left	I hru				
Leading Detector (ft)	20	100			100	20	20	100				
Trailing Detector (ft)	0	0			0	0	0	0				
Detector 1 Position(ft)	0	0			0	0	0	0				
Detector 1 Size(ft)	20	6			6	20	20	6				
Detector 1 Type	CI+EX	CI+Ex			CI+EX	CI+EX	CI+EX	CI+Ex				
Detector 1 Channel	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Extend (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 20: N East St & E Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Type		CI+Ex			Cl+Ex			Cl+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA	Perm	Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4					8	2					
Detector Phase	4	4			8	8	2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0	5.0	5.0	5.0				
Minimum Split (s)	31.4	31.4			21.4	21.4	47.0	47.0				
Total Split (s)	26.0	26.0			26.0	26.0	54.0	54.0				
Total Split (%)	32.5%	32.5%			32.5%	32.5%	67.5%	67.5%				
Maximum Green (s)	20.6	20.6			20.6	20.6	48.0	48.0				
Yellow Time (s)	3.2	3.2			3.2	3.2	3.2	3.2				
All-Red Time (s)	2.2	2.2			2.2	2.2	2.8	2.8				
Lost Time Adjust (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Total Lost Time (s)	5.4	5.4			5.4	5.4	6.0	6.0				
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0	3.0	3.0	3.0				
Recall Mode	Max	Max			None	None	C-Max	C-Max				
Walk Time (s)	15.0	15.0			5.0	5.0	30.0	30.0				
Flash Dont Walk (s)	11.0	11.0			11.0	11.0	11.0	11.0				
Pedestrian Calls (#/hr)	0	0			0	0	0	0				
Act Effct Green (s)	20.6	20.6			20.6	20.6	48.0	48.0				
Actuated g/C Ratio	0.26	0.26			0.26	0.26	0.60	0.60				
v/c Ratio	0.09	0.61			0.80	0.28	0.05	0.72				
Control Delay	22.8	26.7			45.2	12.8	3.4	6.3				
Queue Delay	0.0	0.3			0.0	0.0	0.0	0.1				
Total Delay	22.8	27.0			45.2	12.8	3.4	6.4				
LOS	С	С			D	В	A	A				
Approach Delay	-	26.7			37.2			6.4				
Approach LOS		С			D			A				
90th %ile Green (s)	20.6	20.6			20.6	20.6	48.0	48.0				
90th %ile Term Code	Ped	Ped			Max	Max	Coord	Coord				
70th %ile Green (s)	20.6	20.6			20.6	20.6	48.0	48.0				
70th %ile Term Code	Ped	Ped			Max	Max	Coord	Coord				
50th %ile Green (s)	20.6	20.6			20.6	20.6	48.0	48.0				
50th %ile Term Code	Ped	Ped			Max	Max	Coord	Coord				
30th %ile Green (s)	20.6	20.6			20.6	20.6	48.0	48.0				
30th %ile Term Code	Ped	Ped			Hold	Hold	Coord	Coord				
10th %ile Green (s)	20.6	20.6			20.6	20.6	48.0	48.0				
10th %ile Term Code	Ped	Ped			Hold	Hold	Coord	Coord				
Stops (vph)	6	104			226	30	3	301				
Fuel Used(gal)	0	2			5	1	0	6				
CO Emissions (a/hr)	7	139			358	61	7	396				
NOx Emissions (g/hr)	. 1	27			70	12	. 1	77				
VOC Emissions (g/hr)	2	32			83	14	2	92				
Dilemma Vehicles (#)	0	0			0	0	0	0				

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Lanes, Volumes, Timings 20: N East St & E Washington St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)	3	45			152	15	2	51				
Queue Length 95th (ft)	13	103			#230	44	m4	57				
Internal Link Dist (ft)		182			644			242			228	
Turn Bay Length (ft)	65					80	80					
Base Capacity (vph)	150	386			405	376	732	1827				
Starvation Cap Reductn	0	14			0	0	0	47				
Spillback Cap Reductn	0	0			0	0	0	0				
Storage Cap Reductn	0	0			0	0	0	0				
Reduced v/c Ratio	0.09	0.63			0.80	0.28	0.05	0.74				
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 64 (80%), Reference	d to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 80												
Control Type: Actuated-Coo	rdinated											
Maximum v/c Ratio: 0.80												
Intersection Signal Delay: 15	5.4			In	tersectior	n LOS: B						
Intersection Capacity Utilizat	tion 59.8%			IC	CU Level o	of Service	В					
Analysis Period (min) 15												
# 95th percentile volume e	xceeds cap	bacity, que	eue may	be longer								
Queue shown is maximu	m after two	cycles.										
m Volume for 95th percent	tile queue is	s metered	l by upstr	eam sign	al.							
Splits and Phases: 20: N I	East St & E	Washing	ton St									
		Ŭ						A				354

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54 s	26 s
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Lanes, Volumes, Timings 23: S Center St/N Madison St & W Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		Į.		7	1						đ þ	
Traffic Volume (vph)	0	66	17	23	35	0	0	0	0	64	778	37
Future Volume (vph)	0	66	17	23	35	0	0	0	0	64	778	37
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	13	12	12	12	12	12	12	12	12
Grade (%)		3%			-3%			1%			-1%	
Storage Length (ft)	0		0	60		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	0		0
Taper Length (ft)	25			0			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		0.99		0.98							1.00	
Frt		0.972									0.994	
Flt Protected				0.950							0.996	
Satd. Flow (prot)	0	1392	0	1623	1488	0	0	0	0	0	3075	0
Flt Permitted				0.685							0.996	
Satd. Flow (perm)	0	1392	0	1144	1488	0	0	0	0	0	3075	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		14									12	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		684			247			660			324	
Travel Time (s)		15.5			5.6			15.0			7.4	
Confl. Peds. (#/hr)			11	11						3		5
Peak Hour Factor	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%
Parking (#/hr)		0			0					20		20
Adj. Flow (vph)	0	88	23	31	47	0	0	0	0	85	1037	49
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	111	0	31	47	0	0	0	0	0	1171	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		13			13			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.17	1.33	1.17	1.08	1.28	1.12	1.15	1.15	1.15	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		Cl+Ex	CI+Ex					Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	

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Lanes, Volumes, Timings 23: S Center St/N Madison St & W Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Type		Cl+Ex			CI+Ex						Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	
Permitted Phases				8	-					6	-	
Detector Phase		4		8	8					6	6	
Switch Phase				-	-					-	-	
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		15.5		25.5	25.5					37.7	37.7	
Total Split (s)		20.0		20.0	20.0					60.0	60.0	
Total Split (%)		25.0%		25.0%	25.0%					75.0%	75.0%	
Maximum Green (s)		14.5		14.5	14.5					54.3	54.3	
Yellow Time (s)		32		32	32					32	32	
All-Red Time (s)		2.3		2.3	2.3					2.5	2.5	
Lost Time Adjust (s)		0.0		0.0	0.0					2.0	0.0	
Total Lost Time (s)		5.5		5.5	5.5						5.7	
Lead/Lag		0.0		0.0	0.0						0.1	
Lead-Lag Ontimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		Max		None	None					C-Max	C-Max	
Walk Time (s)		1.0		9.0	9.0					21.0	21.0	
Flash Dont Walk (s)		9.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0.0		0	0					0	0	
Act Effet Green (s)		14 5		14 5	14 5					U	54.3	
Actuated q/C Ratio		0.18		0.18	0.18						0.68	
v/c Ratio		0.10		0.10	0.10						0.00	
Control Delay		30.9		29.7	29.7						2.0	
Queue Delay		0.0		0.0	0.0						0.0	
Total Delay		30.9		29.7	29.7						2.0	
		C.00		20.1 C	20.1 C						Δ	
Approach Delay		30.9		Ũ	29.7						20	
Approach LOS		C.00			20.7 C						Δ.0	
90th %ile Green (s)		14.5		14.5	14.5					54.3	54.3	
90th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
70th %ile Green (s)		14 5		14 5	14.5					54.3	54.3	
70th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
50th %ile Green (s)		14 5		14 5	14.5					54.3	54.3	
50th %ile Term Code		MayR		Hold	Hold					Coord	Coord	
30th %ile Green (s)		14 5		14 5	14.5					54.3	54.3	
30th %ile Term Code		MayR		Hold	Hold					Coord	Coord	
10th %ile Green (s)		14 5		14 5	14.5					54.3	54.3	
10th %ile Term Code		May R		Hold	Hold					Coord	Coord	
Stops (uph)		63		21	31					COOld	55	
		1		21	0						00	
CO Emissions (a/br)		00		21	21						201	
NOv Emissions (g/III)		92 10		21	51						201	
VOC Emissions (g/hr)		10		4	7						39	
Dilomma Vohiclos (#)		21		0	1						47	
Dilemma vehicles (#)		U		U	U						0	

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 23: S Center St/N Madison St & W Front St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)		43		13	20						19	
Queue Length 95th (ft)		73		30	41						19	
Internal Link Dist (ft)		604			167			580			244	
Turn Bay Length (ft)				60								
Base Capacity (vph)		263		207	269						2091	
Starvation Cap Reductn		0		0	0						57	
Spillback Cap Reductn		0		0	0						0	
Storage Cap Reductn		0		0	0						0	
Reduced v/c Ratio		0.42		0.15	0.17						0.58	
Intersection Summary												
Area Type: CB	D											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 4 (5%), Referenced to p	hase 2:	and 6:SB	TL, Start	of Green								
Natural Cycle: 65												
Control Type: Actuated-Coordin	nated											
Maximum v/c Ratio: 0.56												
Intersection Signal Delay: 6.0				In	tersectior	n LOS: A						
Intersection Capacity Utilization	n 45.0%			IC	U Level o	of Service	A					
Analysis Period (min) 15												

Splits and Phases: 23: S Center St/N Madison St & W Front St

	→ Ø4	22
	20 s	
Ø6 (R)	Ø8	30
60 s	20 s	

02/22/2024

Lanes, Volumes, Timings 24: S East St/N East St & E Front St

02/22/2024

Lane Group EBL EBT EBR WBL WBT NBL NBT NBT NBR SBL SBT SBR Lane Configurations 1 4 4 7 4		٠	-	7	4	+	*	1	t	1	1	ŧ	~
Lane Configurations T <tht< th=""> T <tht< th=""></tht<></tht<>	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Oxlume (vph) 15 33 0 0 141 40 45 999 18 0 0 0 ideal Flow (vph) 1900 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 100 <	Lane Configurations	ሻ	^			†	7		đ þ				
Future (vph) 15 33 0 141 40 45 999 18 0 0 0 ideal Flow (vphp) 1900 100 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 </td <td>Traffic Volume (vph)</td> <td>15</td> <td>33</td> <td>0</td> <td>0</td> <td>141</td> <td>40</td> <td>45</td> <td>999</td> <td>18</td> <td>0</td> <td>0</td> <td>0</td>	Traffic Volume (vph)	15	33	0	0	141	40	45	999	18	0	0	0
Ideal Flow (rphp) 1900 100	Future Volume (vph)	15	33	0	0	141	40	45	999	18	0	0	0
Lane Width (t) 11 16 12 12 10 10 13 11 11 12 12 12 12 13 12 12 12 13 12 12 12 13 12 12 13 12 12 12 13 13 11 11 11 12 12 12 12 13 13 14 11 11 12 12 12 12 13 13 14 11 11 12 12 12 12 13 13 14 11 11 12 13 12 12 13 13 14 11 11 12 13 13 14 11 11 12 13 13 14 11 11 12 13 13 14 11 11 12 13 13 14 11 11 12 13 13 14 11 11 12 13 13 14 11 11 12 13 13 14 11 11 12 13 13 14 11 11 12 13 13 14 11 11 12 13 13 14 11 11 12 13 13 14 11 11 12 13 13 14 11 11 12 13 13 14 11 11 12 13 13 14 11 11 12 13 13 14 11 11 12 13 13 14 11 11 12 13 13 14 11 11 11 12 13 13 14 11 11 12 13 13 14 11 11 12 13 13 14 11 11 12 13 13 14 11 13 14 13 13 14 11 14 12 13 13 14 11 14 12 13 13 14 11 14 12 13 13 14 11 14 13 13 14 14 13 13 14 14 15 13 13 14 10 10 10 0 155 125 0 0 130 15 10.0 10 0 0 0 10 10 100 100 1515 130 10 0 1535 1320 0 3017 0 0 0 0 0 0 1515 130 0 1535 1320 0 3017 0 0 0 0 0 0 0 1514 53 1304 0 3019 0 0 0 0 0 0 0 1514 53 1304 0 3019 0 0 0 0 0 0 0 1514 53 1304 0 3019 0 0 0 0 0 0 0 1514 53 1304 0 3019 0 0 0 0 0 0 0 0 1514 53 1304 0 3019 0 0 0 0 0 0 0 0 1514 53 1304 0 3019 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%) 1% 0% 1% 2% Storage Length (ft) 90 0 0 100 0	Lane Width (ft)	11	16	12	12	10	10	13	11	11	12	12	12
Shorage Length (ft) 90 0 0 1 0	Grade (%)		-1%			0%			-1%			2%	
Shorage Lanes 1 0 0 1 0 0 0 0 Taper Length (ft) 0 25	Storage Length (ft)	90		0	0		100	0		0	0		0
Taper Length (ft) 0 25 25 25 Lane Util, Factor 1.00 1.00 1.00 1.00 0.95 0.95 0.95 1.00 1.00 1.00 Ped Bike Factor 0.98 0.96 1.00 0.998 0.998 0.998 0.998 0.998 0.00 0	Storage Lanes	1		0	0		1	0		0	0		0
Lane UBI, Factor 1.00	Taper Length (ft)	0			25			25			25		
Ped Bike Factor 0.98 0.96 1.00 Frt 0.850 0.997 Fit Protected 0.950 0.998 Satd Flow (prot) 1518 1873 0 0 1535 1304 0 3019 0 <td>Lane Util. Factor</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>0.95</td> <td>0.95</td> <td>0.95</td> <td>1.00</td> <td>1.00</td> <td>1.00</td>	Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Fit 0.850 0.997 Fit Protected 0.950 0.950 Satd. Flow (prot) 1518 1873 0 0 1535 1304 0 0.998 Satd. Flow (perm) 945 1873 0 0 1535 1250 0 3017 0	Ped Bike Factor	0.98					0.96		1.00				
Fit Protected 0.950 0 0.938 Satd. Flow (prot) 1518 1873 0 0 1335 1304 0 0.0998 Satd. Flow (perm) 945 1873 0 0 1535 1250 0 3017 0 0 0 0 Right Tum on Red Yes	Frt						0.850		0.997				
Satd. Flow (prot) 1518 1873 0 0 1535 1304 0 3019 0 0 0 0 FIL Permitted 0.605 0 1535 1250 0 3017 0<	Flt Protected	0.950							0.998				
Fit Permitted 0.605 0.998 Satd. Flow (perm) 945 1873 0 0 155 1250 0 3017 0	Satd. Flow (prot)	1518	1873	0	0	1535	1304	0	3019	0	0	0	0
Satd, Flow (perm) 945 1873 0 0 1535 1250 0 3017 0 0 0 0 Right Turn on Red Yes Xes Yes	Flt Permitted	0.605							0.998				
Right Turn on Red Yes Yes Yes Yes Yes Yes Satd. Flow (RTOR) 47 4 4 1<	Satd. Flow (perm)	945	1873	0	0	1535	1250	0	3017	0	0	0	0
Said. Flow (RTOR) 47 4 Link Speed (mph) 30 30 30 30 Link Distance (ft) 256 710 633 322 Travel Time (s) 5.8 16.1 14.4 7.3 Confl. Peds. (#hr) 15 19 1 1 Peak Hour Factor 0.85	Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph) 30 30 30 30 30 30 Link Distance (ft) 256 710 633 322 Travel Time (s) 5.8 16.1 14.4 7.3 Confl. Peds, (#hr) 15 19 1 1 Peak Hour Factor 0.85 <t< td=""><td>Satd. Flow (RTOR)</td><td></td><td></td><td></td><td></td><td></td><td>47</td><td></td><td>4</td><td></td><td></td><td></td><td></td></t<>	Satd. Flow (RTOR)						47		4				
Link Distance (ft) 256 710 633 322 Travel Time (s) 5.8 16.1 14.4 7.3 Confl. Peds. (#/hr) 15 19 1	Link Speed (mph)		30			30			30			30	
Travel Time (s) 5.8 16.1 14.4 7.3 Confl. Peds. (#/hr) 15 15 15 19 1 Peak Hour Factor 0.85	Link Distance (ft)		256			710			633			322	
Confl. Peds. (#/hr) 15 15 19 1 Peak Hour Factor 0.85 </td <td>Travel Time (s)</td> <td></td> <td>5.8</td> <td></td> <td></td> <td>16.1</td> <td></td> <td></td> <td>14.4</td> <td></td> <td></td> <td>7.3</td> <td></td>	Travel Time (s)		5.8			16.1			14.4			7.3	
Peak Hour Factor 0.85 <td>Confl. Peds. (#/hr)</td> <td>15</td> <td></td> <td></td> <td></td> <td></td> <td>15</td> <td>19</td> <td></td> <td>1</td> <td></td> <td></td> <td></td>	Confl. Peds. (#/hr)	15					15	19		1			
Heavy Vehicles (%) 4% 4	Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Adj. Flow (vph) 18 39 0 0 166 47 53 1175 21 0 0 0 Shared Lane Traffic (%) Lane Group Flow (vph) 18 39 0 0 166 47 0 1249 0	Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Shared Lane Traffic (%) Lane Group Flow (vph) 18 39 0 0 166 47 0 1249 0 0 0 0 Enter Blocked Intersection No	Adi, Flow (vph)	18	39	0	0	166	47	53	1175	21	0	0	0
Lane Group Flow (vph) 18 39 0 0 166 47 0 1249 0 0 0 0 Enter Blocked Intersection No	Shared Lane Traffic (%)												-
Enter Blocked Intersection No No <th< td=""><td>Lane Group Flow (vph)</td><td>18</td><td>39</td><td>0</td><td>0</td><td>166</td><td>47</td><td>0</td><td>1249</td><td>0</td><td>0</td><td>0</td><td>0</td></th<>	Lane Group Flow (vph)	18	39	0	0	166	47	0	1249	0	0	0	0
Lane Alignment Left Left Right	Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Median Width(ft) 11<	Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Link Offset(ft) 0 0 0 0 0 Crosswalk Width(ft) 16 16 16 16 16 Two way Left Turn Lane 1.19 0.97 1.14 1.14 1.25 1.25 1.09 1.19 1.19 1.16 <t< td=""><td>Median Width(ft)</td><td></td><td>11</td><td>J -</td><td></td><td>11</td><td>J -</td><td></td><td>11</td><td>J -</td><td></td><td>11</td><td>J -</td></t<>	Median Width(ft)		11	J -		11	J -		11	J -		11	J -
Crosswalk Width(ft) 16 16 16 16 Two way Left Turn Lane	Link Offset(ft)		0			0			0			0	
Two way Left Turn Lane 1.19 0.97 1.14 1.14 1.25 1.25 1.09 1.19 1.16 <	Crosswalk Width(ft)		16			16			16			16	
Headway Factor 1.19 0.97 1.14 1.14 1.25 1.25 1.09 1.19 1.19 1.16<	Two way Left Turn Lane												
Turning Speed (mph) 15 9 15<	Headway Factor	1.19	0.97	1.14	1.14	1.25	1.25	1.09	1.19	1.19	1.16	1.16	1.16
Number of Detectors 1 2 2 1 1 2 Detector Template Left Thru Right Left Thru Right Left Thru Leading Detector (ft) 20 100 100 20 20 100 Trailing Detector (ft) 0 0 0 0 0 0 0 Detector 1 Position(ft) 0 0 0 0 0 0 0 Detector 1 Size(ft) 20 6 6 20 20 6 Detector 1 Size(ft) 20 6 6 20 20 6 Detector 1 Size(ft) 20 6 6 20 20 6 Detector 1 Channel	Turning Speed (mph)	15		9	15		9	15		9	15		9
Detector Template Left Thru Right Left Thru Leading Detector (ft) 20 100 100 20 20 100 Trailing Detector (ft) 0 0 0 0 0 0 Detector 1 Position(ft) 0 0 0 0 0 0 Detector 1 Size(ft) 20 6 6 20 20 6 Detector 1 Size(ft) 20 6 6 20 20 6 Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex Detector 1 Channel	Number of Detectors	1	2	-		2	1	1	2	-			-
Leading Detector (ft) 20 100 100 20 20 100 Trailing Detector (ft) 0 0 0 0 0 0 Detector 1 Position(ft) 0 0 0 0 0 0 Detector 1 Size(ft) 20 6 6 20 20 6 Detector 1 Size(ft) 20 6 6 20 20 6 Detector 1 Type Cl+Ex Cl+Ex Cl+Ex Cl+Ex Cl+Ex Detector 1 Channel Detector 1 Queue (s) 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 94 94 Detector 2 Size(ft) 6 6 6 Detector 2 Type Cl+Ex Cl+Ex Cl+Ex	Detector Template	Left	Thru			Thru	Right	Left	Thru				
Trailing Detector (ft) 0 0 0 0 0 Detector 1 Position(ft) 0 0 0 0 0 Detector 1 Position(ft) 0 0 0 0 0 Detector 1 Size(ft) 20 6 6 20 20 6 Detector 1 Size(ft) 20 6 CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex Detector 1 Channel	Leading Detector (ft)	20	100			100	20	20	100				
Detector 1 Position(ft) 0 0 0 0 0 0 Detector 1 Size(ft) 20 6 6 20 20 6 Detector 1 Size(ft) 20 6 CI+Ex CI+Ex CI+Ex CI+Ex Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex Detector 1 Channel 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 94 94 94 Detector 2 Size(ft) 6 6 6 6 Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex	Trailing Detector (ft)	0	0			0	0	0	0				
Detector 1 Size(ft) 20 6 6 20 20 6 Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex Detector 1 Channel Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 94 94 Detector 2 Size(ft) 6 6 6 Detector 2 Type CI+Ex CI+Ex CI+Ex	Detector 1 Position(ft)	0	0			0	0	0	0				
Detector 1 Type CI+Ex	Detector 1 Size(ft)	20	6			6	20	20	6				
Detector 1 Channel 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 94 94 Detector 2 Size(ft) 6 6 6 Detector 2 Type Cl+Ex Cl+Ex Cl+Ex	Detector 1 Type	CI+Ex	Cl+Fx			Cl+Ex	Cl+Ex	Cl+Ex	CI+Ex				
Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 94 Detector 2 Size(ft) 6 6 6 Detector 2 Type CI+Ex CI+Ex CI+Ex	Detector 1 Channel	••• =••	••• =••			0	0/.	0	•• =/				
Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 94 Detector 2 Size(ft) 6 6 6 Detector 2 Type CI+Ex CI+Ex CI+Ex	Detector 1 Extend (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Delay (s) 0.0	Detector 1 Queue (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 2 Position(ft) 94 94 Detector 2 Size(ft) 6 6 Detector 2 Type CI+Ex CI+Ex	Detector 1 Delay (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 2 Size(ft) 6 6 6 Detector 2 Type CI+Ex CI+Ex	Detector 2 Position(ft)	0.0	94			.94	0.0	0.0	94				
	Detector 2 Size(ft)		6			6			6				
	Detector 2 Type		CI+Fx			Cl+Fx			Cl+Fx				

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 24: S East St/N East St & E Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA	Perm	Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4					8	2					
Detector Phase	4	4			8	8	2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0	5.0	5.0	5.0				
Minimum Split (s)	27.2	27.2			17.2	17.2	50.6	50.6				
Total Split (s)	22.0	22.0			22.0	22.0	58.0	58.0				
Total Split (%)	27.5%	27.5%			27.5%	27.5%	72.5%	72.5%				
Maximum Green (s)	16.8	16.8			16.8	16.8	52.4	52.4				
Yellow Time (s)	3.2	3.2			3.2	3.2	3.2	3.2				
All-Red Time (s)	2.0	2.0			2.0	2.0	2.4	2.4				
Lost Time Adjust (s)	0.0	0.0			0.0	0.0		0.0				
Total Lost Time (s)	5.2	5.2			5.2	5.2		5.6				
Lead/Lag	•	•.=			•=	0.2		0.0				
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0	3.0	3.0	3.0				
Recall Mode	Max	Max			None	None	C-Max	C-Max				
Walk Time (s)	11.0	11.0			1.0	10	34.0	34.0				
Flash Dont Walk (s)	11.0	11.0			11.0	11.0	11.0	11.0				
Pedestrian Calls (#/hr)	0	0			0	0	0	0				
Act Effct Green (s)	16.8	16.8			16.8	16.8	Ŭ	52 4				
Actuated g/C Ratio	0.21	0.21			0.21	0.21		0.66				
v/c Ratio	0.09	0.10			0.52	0.16		0.63				
Control Delay	26.9	26.4			34.6	9.8		6.2				
Queue Delay	0.0	0.0			0.0	0.0		0.0				
Total Delay	26.9	26.4			34.6	9.8		6.3				
LOS	C	C			C	A		A				
Approach Delay	•	26.6			29.1			6.3				
Approach LOS		С			С			A				
90th %ile Green (s)	16.8	16.8			16.8	16.8	52.4	52.4				
90th %ile Term Code	Ped	Ped			Max	Max	Coord	Coord				
70th %ile Green (s)	16.8	16.8			16.8	16.8	52.4	52.4				
70th %ile Term Code	Ped	Ped			Hold	Hold	Coord	Coord				
50th %ile Green (s)	16.8	16.8			16.8	16.8	52.4	52.4				
50th %ile Term Code	Ped	Ped			Hold	Hold	Coord	Coord				
30th %ile Green (s)	16.8	16.8			16.8	16.8	52.4	52.4				
30th %ile Term Code	Ped	Ped			Hold	Hold	Coord	Coord				
10th %ile Green (s)	16.8	16.8			16.8	16.8	52.4	52.4				
10th %ile Term Code	Ped	Ped			Hold	Hold	Coord	Coord				
Stops (vph)	14	29			124	11	ooora	291				
Fuel Used(gal)	1 -1 0	0			27	0		201				
CO Emissions (a/br)	13	28			172	25		573				
NOx Emissions (g/hr)	3	5			33	5		111				
VOC Emissions (g/hr)	3	7			40	6		133				
Dilemma Vehicles (#)	0	0				0		0				
Oueue Length 50th (ft)	7	16			74	0		82				
	1	10			17	0		02				

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 24: S East St/N East St & E Front St

02/22/2024	ŀ
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)	23	38			126	24		92				
Internal Link Dist (ft)		176			630			553			242	
Turn Bay Length (ft)	90					100						
Base Capacity (vph)	198	393			322	299		1977				
Starvation Cap Reductn	0	0			0	0		0				
Spillback Cap Reductn	0	0			0	0		31				
Storage Cap Reductn	0	0			0	0		0				
Reduced v/c Ratio	0.09	0.10			0.52	0.16		0.64				
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 8	0											
Offset: 60 (75%), Referen	ced to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 80												
Control Type: Actuated-C	oordinated											
Maximum v/c Ratio: 0.63												
Intersection Signal Delay:	10.2			In	tersectior	n LOS: B						
Intersection Capacity Utili	zation 61.5%			IC	U Level o	of Service	В					
Analysis Period (min) 15												

Splits and Phases: 24: S East St/N East St & E Front St

Ø2 (R)	 Ø4
58 s	22 s
	<u></u> Ø8
	22 s

Lanes, Volumes, Timings 27: N Center St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,			ŧ						đ þ	
Traffic Volume (vph)	0	42	23	13	112	0	0	0	0	4	139	13
Future Volume (vph)	0	42	23	13	112	0	0	0	0	4	139	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt		0.952									0.988	
Flt Protected					0.995						0.999	
Satd. Flow (prot)	0	1536	0	0	1445	0	0	0	0	0	2874	0
Flt Permitted					0.995						0.999	
Satd. Flow (perm)	0	1536	0	0	1445	0	0	0	0	0	2874	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		247			266			264			413	
Travel Time (s)		5.6			6.0			6.0			9.4	
Confl. Peds. (#/hr)			23	23						6		6
Confl. Bikes (#/hr)			1									
Peak Hour Factor	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Heavy Vehicles (%)	6%	6%	6%	6%	6%	6%	6%	6%	6%	6%	6%	6%
Parking (#/hr)					0						0	
Adj. Flow (vph)	0	53	29	16	142	0	0	0	0	5	176	16
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	82	0	0	158	0	0	0	0	0	197	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.14	1.14	1.14	1.30	1.14	1.14	1.14	1.14	1.14	1.22	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: 0	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 27.1%			IC	CU Level of	of Service	Α					
Analysis Period (min) 15												

Lanes, Volumes, Timings 29: N Center St & W Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f.			ŧ						đ þ	
Traffic Volume (vph)	0	33	1	12	18	0	0	0	0	20	104	3
Future Volume (vph)	0	33	1	12	18	0	0	0	0	20	104	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt		0.997									0.996	
Flt Protected					0.981						0.992	
Satd. Flow (prot)	0	1408	0	0	1385	0	0	0	0	0	2798	0
Flt Permitted					0.981						0.992	
Satd. Flow (perm)	0	1408	0	0	1385	0	0	0	0	0	2798	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		253			255			306			278	
Travel Time (s)		5.8			5.8			7.0			6.3	
Confl. Peds. (#/hr)			7	7						13		9
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80
Heavy Vehicles (%)	9%	9%	9%	9%	9%	9%	9%	9%	9%	9%	9%	9%
Parking (#/hr)		0			0						0	
Adj. Flow (vph)	0	41	1	15	23	0	0	0	0	25	130	4
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	42	0	0	38	0	0	0	0	0	159	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.14	1.14	1.14	1.22	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: (CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 21.5% ICU Level of Service A											

Analysis Period (min) 15

Lanes, Volumes, Timings 30: N Center St & W Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		Į.		7	†						đ þ	
Traffic Volume (vph)	0	165	7	23	214	0	0	0	0	50	60	7
Future Volume (vph)	0	165	7	23	214	0	0	0	0	50	60	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	65		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		1.00		1.00							0.99	
Frt		0.994									0.991	
Flt Protected				0.950							0.979	
Satd. Flow (prot)	0	1416	0	1504	1425	0	0	0	0	0	2766	0
Flt Permitted				0.538							0.979	
Satd. Flow (perm)	0	1416	0	849	1425	0	0	0	0	0	2755	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		4									9	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		253			254			326			306	
Travel Time (s)		5.8			5.8			7.4			7.0	
Confl. Peds. (#/hr)			3	3						4		10
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80
Heavy Vehicles (%)	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%
Parking (#/hr)		0			0						0	
Adj. Flow (vph)	0	206	9	29	268	0	0	0	0	63	75	9
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	215	0	29	268	0	0	0	0	0	147	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.14	1.14	1.14	1.22	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		Cl+Ex	CI+Ex					CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		CI+Ex			Cl+Ex						CI+Ex	
Detector 2 Channel												

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 30: N Center St & W Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		23.0		23.0	23.0					23.0	23.0	
Total Split (s)		50.0		50.0	50.0					30.0	30.0	
Total Split (%)		62.5%		62.5%	62.5%					37.5%	37.5%	
Maximum Green (s)		45.0		45.0	45.0					25.0	25.0	
Yellow Time (s)		4.0		4.0	4.0					4.0	4.0	
All-Red Time (s)		1.0		1.0	1.0					1.0	1.0	
Lost Time Adjust (s)		0.0		0.0	0.0						0.0	
Total Lost Time (s)		5.0		5.0	5.0						5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		None	None					C-Max	C-Max	
Walk Time (s)		7.0		7.0	7.0					7.0	7.0	
Flash Dont Walk (s)		11.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		21.5		21.5	21.5						48.5	
Actuated g/C Ratio		0.27		0.27	0.27						0.61	
v/c Ratio		0.56		0.13	0.70						0.09	
Control Delay		18.9		3.1	12.4						7.9	
Queue Delay		0.0		0.0	0.1						0.0	
Total Delay		18.9		3.1	12.5						7.9	
LOS		В		А	В						А	
Approach Delay		18.9			11.6						7.9	
Approach LOS		В			В						А	
90th %ile Green (s)		31.0		31.0	31.0					39.0	39.0	
90th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
70th %ile Green (s)		25.0		25.0	25.0					45.0	45.0	
70th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
50th %ile Green (s)		21.3		21.3	21.3					48.7	48.7	
50th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
30th %ile Green (s)		17.7		17.7	17.7					52.3	52.3	
30th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
10th %ile Green (s)		12.6		12.6	12.6					57.4	57.4	
10th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
Stops (vph)		68		2	63						47	
Fuel Used(gal)		1		0	1						1	
CO Emissions (g/hr)		96		5	92						51	
NOx Emissions (a/hr)		19		1	18						10	
VOC Emissions (g/hr)		22		1	21						12	
Dilemma Vehicles (#)		0		0	0						0	
Queue Length 50th (ft)		45		2	22						13	
Queue Length 95th (ft)		49		m4	20						30	

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 30: N Center St & W Washington St

02/22/2024	ŀ
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		173			174			246			226	
Turn Bay Length (ft)				65								
Base Capacity (vph)		798		477	801						1673	
Starvation Cap Reductn		30		0	70						0	
Spillback Cap Reductn		0		0	45						0	
Storage Cap Reductn		0		0	0						0	
Reduced v/c Ratio		0.28		0.06	0.37						0.09	
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80)											
Offset: 19 (24%), Referen	ced to phase	2: and 6:	SBTL, Sta	art of Gre	en							
Natural Cycle: 50												
Control Type: Actuated-Co	oordinated											
Maximum v/c Ratio: 0.70												
Intersection Signal Delay:	13.1			In	tersectior	n LOS: B						
Intersection Capacity Utiliz	zation 48.5%			IC	U Level o	of Service	A					
Analysis Period (min) 15												
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m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 30: N Center St & W Washington St

	→ Ø4
	50 s
	+
🕈 Ø6 (R)	▼ Ø8
30 s	50 s

Lanes, Volumes, Timings 31: W Front St & N Center St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		¢Î,		٢	1			\$		٢	f,	
Traffic Volume (vph)	0	127	21	22	61	0	0	0	0	57	15	6
Future Volume (vph)	0	127	21	22	61	0	0	0	0	57	15	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	8	12	12	12	12
Storage Length (ft)	0		0	70		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.981									0.954	
Flt Protected				0.950						0.950		
Satd. Flow (prot)	0	1398	0	1504	1425	0	0	1235	0	1354	1359	0
Flt Permitted				0.950						0.950		
Satd. Flow (perm)	0	1398	0	1504	1425	0	0	1235	0	1354	1359	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		247			262			221			326	
Travel Time (s)		5.6			6.0			5.0			7.4	
Confl. Peds. (#/hr)			35	35			19		8	8		19
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%
Parking (#/hr)		0			0			0		0	0	
Adj. Flow (vph)	0	138	23	24	66	0	0	0	0	62	16	7
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	161	0	24	66	0	0	0	0	62	23	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		13			13			0			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane					Yes							
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.55	1.14	1.30	1.30	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: C	BD											
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 34.9%			IC	U Level o	of Service	А					
Analysis Period (min) 15												

Lanes, Volumes, Timings 34: N Main St & W Market St/E Market St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ŧ			ĥ			đ þ				
Traffic Volume (vph)	8	41	0	0	96	6	32	29	20	0	0	0
Future Volume (vph)	8	41	0	0	96	6	32	29	20	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.992			0.963				
Flt Protected		0.992						0.981				
Satd. Flow (prot)	0	1414	0	0	1414	0	0	2700	0	0	0	0
Flt Permitted		0.992						0.981				
Satd. Flow (perm)	0	1414	0	0	1414	0	0	2700	0	0	0	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		266			263			249			404	
Travel Time (s)		6.0			6.0			5.7			9.2	
Confl. Peds. (#/hr)	13					13	1		7			
Confl. Bikes (#/hr)						1						
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Heavy Vehicles (%)	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%
Parking (#/hr)		0			0			0				
Adj. Flow (vph)	10	53	0	0	125	8	42	38	26	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	63	0	0	133	0	0	106	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.22	1.14	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: 0	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 21.6%			IC	CU Level	of Service	А					
Analysis Period (min) 15												

Lanes, Volumes, Timings 36: N Main St & W Jefferson St/E Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ŧ			f,			đ þ				
Traffic Volume (vph)	16	16	0	0	17	11	26	67	11	0	0	0
Future Volume (vph)	16	16	0	0	17	11	26	67	11	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.946			0.984				
Flt Protected		0.976						0.988				
Satd. Flow (prot)	0	1366	0	0	1324	0	0	2728	0	0	0	0
Flt Permitted		0.976						0.988				
Satd. Flow (perm)	0	1366	0	0	1324	0	0	2728	0	0	0	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		255			268			306			272	
Travel Time (s)		5.8			6.1			7.0			6.2	
Confl. Peds. (#/hr)	11					11	2		24			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	10%	10%	10%	10%	10%	10%	10%	10%	10%	10%	10%	10%
Parking (#/hr)		0			0			0				
Adj. Flow (vph)	17	17	0	0	18	12	28	73	12	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	34	0	0	30	0	0	113	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.22	1.14	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type:	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	tion 24.2%			IC	CU Level	of Service	Α					

Analysis Period (min) 15

Lanes, Volumes, Timings	
37: N Main St & W Washington St/E Washington St	t

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	1			ĥ			é.	1			
Traffic Volume (vph)	1	194	0	0	253	30	6	25	10	0	0	0
Future Volume (vph)	1	194	0	0	253	30	6	25	10	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	65		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00				1.00			1.00	0.97			
Frt					0.986				0.850			
Flt Protected	0.950							0.990				
Satd. Flow (prot)	1504	1425	0	0	1403	0	0	1411	1211	0	0	0
Flt Permitted	0.386							0.990				
Satd. Flow (perm)	610	1425	0	0	1403	0	0	1406	1171	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					13				27			
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		254			262			326			306	
Travel Time (s)		5.8			6.0			7.4			7.0	
Confl. Peds. (#/hr)	3					3	7		6			
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80
Heavy Vehicles (%)	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%	8%
Parking (#/hr)		0			0			0	0			
Adj. Flow (vph)	1	243	0	0	316	38	8	31	13	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	1	243	0	0	354	0	0	39	13	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12	Ū		0	0		0	Ū
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.30	1.30	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2			2		1	2	1			
Detector Template	Left	Thru			Thru		Left	Thru	Right			
Leading Detector (ft)	20	100			100		20	100	20			
Trailing Detector (ft)	0	0			0		0	0	0			
Detector 1 Position(ft)	0	0			0		0	0	0			
Detector 1 Size(ft)	20	6			6		20	6	20			
Detector 1 Type	CI+Ex	Cl+Ex			CI+Ex		CI+Ex	CI+Ex	CI+Ex			
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			CI+Ex			Cl+Ex				
Detector 2 Channel												

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings	
37: N Main St & W Washington St/E Washington	St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA		Perm	NA	Perm			
Protected Phases		4			8			2				
Permitted Phases	4						2		2			
Detector Phase	4	4			8		2	2	2			
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0		5.0	5.0	5.0			
Minimum Split (s)	23.0	23.0			23.0		23.0	23.0	23.0			
Total Split (s)	52.0	52.0			52.0		28.0	28.0	28.0			
Total Split (%)	65.0%	65.0%			65.0%		35.0%	35.0%	35.0%			
Maximum Green (s)	47.0	47.0			47.0		23.0	23.0	23.0			
Yellow Time (s)	4.0	4.0			4.0		4.0	4.0	4.0			
All-Red Time (s)	1.0	1.0			1.0		1.0	1.0	1.0			
Lost Time Adjust (s)	0.0	0.0			0.0			0.0	0.0			
Total Lost Time (s)	5.0	5.0			5.0			5.0	5.0			
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0	3.0			
Recall Mode	None	None			None		C-Max	C-Max	C-Max			
Walk Time (s)	7.0	7.0			7.0		7.0	7.0	7.0			
Flash Dont Walk (s)	11.0	11.0			11.0		11.0	11.0	11.0			
Pedestrian Calls (#/hr)	0	0			0		0	0	0			
Act Effct Green (s)	26.9	26.9			26.9			43.1	43.1			
Actuated g/C Ratio	0.34	0.34			0.34			0.54	0.54			
v/c Ratio	0.00	0.51			0.74			0.05	0.02			
Control Delay	22.0	31.1			12.8			13.6	5.2			
Queue Delay	0.0	0.1			0.1			0.0	0.0			
Total Delay	22.0	31.3			12.8			13.6	5.2			
LOS	С	С			В			В	А			
Approach Delay		31.2			12.8			11.5				
Approach LOS		С			В			В				
90th %ile Green (s)	38.1	38.1			38.1		31.9	31.9	31.9			
90th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
70th %ile Green (s)	31.3	31.3			31.3		38.7	38.7	38.7			
70th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
50th %ile Green (s)	26.7	26.7			26.7		43.3	43.3	43.3			
50th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
30th %ile Green (s)	22.3	22.3			22.3		47.7	47.7	47.7			
30th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
10th %ile Green (s)	16.1	16.1			16.1		53.9	53.9	53.9			
10th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
Stops (vph)	1	128			78			22	5			
Fuel Used(gal)	0	2			2			0	0			
CO Emissions (g/hr)	1	162			122			20	4			
NOx Emissions (g/hr)	0	32			24			4	1			
VOC Emissions (g/hr)	0	38			28			5	1			
Dilemma Vehicles (#)	0	0			0			0	0			
Queue Length 50th (ft)	0	88			19			12	0			
Queue Length 95th (ft)	m1	86			m19			m30	m4			

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, T	imings	
37: N Main St & W	Washington St/E	Washington St

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Lane Group	FBI	FBT	FBR	WBI	WBT	WBR	NBI	NBT	NBR	SBI	SBT	SBR
Internal Link Dist (ft)		174	2011		182			246		002	226	0.0.1
Turn Bay Length (ft)	65											
Base Capacity (vph)	358	837			829			757	643			
Starvation Cap Reductn	0	129			49			0	0			
Spillback Cap Reductn	0	38			0			0	0			
Storage Cap Reductn	0	0			0			0	0			
Reduced v/c Ratio	0.00	0.34			0.45			0.05	0.02			
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 7 (9%), Referenced	to phase 2:1	VBTL and	6:, Start	of Green								
Natural Cycle: 50												
Control Type: Actuated-Coc	ordinated											
Maximum v/c Ratio: 0.74												
Intersection Signal Delay: 1	9.6			In	tersectior	n LOS: B						
Intersection Capacity Utiliza	ation 48.5%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
m Volume for 95th percer	ntile queue is	s metered	l by upstr	eam sign	al.							

Splits and Phases: 37: N Main St & W Washington St/E Washington St

[™] Ø2 (R)		
28 s	52 s	-
	← Ø8	
	52 s	

Lanes, Volumes, Timings 38: W Front St/E Front St & N Main St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	ţ,			\$			\$				
Traffic Volume (vph)	33	150	0	0	89	29	0	0	0	0	0	0
Future Volume (vph)	33	150	0	0	89	29	0	0	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	8	12	12	12	12
Storage Length (ft)	70		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.967							
Flt Protected	0.950											
Satd. Flow (prot)	1504	1425	0	0	1378	0	0	1308	0	0	0	0
Flt Permitted	0.950											
Satd. Flow (perm)	1504	1425	0	0	1378	0	0	1308	0	0	0	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		262			256			210			326	
Travel Time (s)		6.0			5.8			4.8			7.4	
Confl. Peds. (#/hr)	17		2	2		17	7		36			
Peak Hour Factor	0.83	0.83	0.92	0.92	0.83	0.83	0.92	0.92	0.92	0.83	0.92	0.83
Heavy Vehicles (%)	8%	8%	2%	2%	8%	8%	2%	2%	2%	8%	2%	8%
Parking (#/hr)		0			0			0				
Adj. Flow (vph)	40	181	0	0	107	35	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	40	181	0	0	142	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.55	1.14	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type: C	BD											
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 27.1%			IC	U Level o	of Service	A					
Analysis Period (min) 15												

	4	•	t	1	4	ţ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		1	≜ ‡⊅			
Traffic Volume (vph)	0	17	1022	7	0	0
Future Volume (vph)	0	17	1022	7	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	11	11	12	12
Grade (%)	0%		-2%			2%
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00
Frt		0.865	0.999			
Flt Protected						
Satd. Flow (prot)	0	1292	3077	0	0	0
Flt Permitted						
Satd. Flow (perm)	0	1292	3077	0	0	0
Link Speed (mph)	30		30			30
Link Distance (ft)	558		266			127
Travel Time (s)	12.7		6.0			2.9
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%
Parking (#/hr)		0				
Adj. Flow (vph)	0	21	1246	9	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	21	1255	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		0			0
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.14	1.30	1.18	1.18	1.16	1.16
Turning Speed (mph)	15	9		9	15	
Sign Control	Stop		Free			Stop
Intersection Summary						
Area Type: (CBD					
Control Type: Unsignalized						
Intersection Capacity Utilizat	ion 41.6%			IC	U Level c	of Service A

Analysis Period (min) 15

Lanes, Volumes, Timings 47: S Center St & W Olive St

02/22/2024	02	122	2	024
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1	1	7	•						41	1
Traffic Volume (vph)	0	60	37	3	37	0	0	0	0	44	726	57
Future Volume (vph)	0	60	37	3	37	0	0	0	0	44	726	57
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	11	10	12	12	12	12	12	12	12	12	12
Grade (%)		1%			-3%			3%			-1%	
Storage Length (ft)	0		80	0		0	0		0	0		50
Storage Lanes	0		1	1		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00
Frt			0.850									0.850
Flt Protected				0.950							0.997	
Satd, Flow (prot)	0	1792	1470	1796	1891	0	0	0	0	0	3546	1591
Flt Permitted				0.715							0.997	
Satd. Flow (perm)	0	1792	1470	1352	1891	0	0	0	0	0	3546	1591
Right Turn on Red	•	•=	Yes			Yes	· ·	•	Yes	•		Yes
Satd. Flow (RTOR)			40									56
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		777			330			770			660	
Travel Time (s)		17 7			7.5			17.5			15.0	
Peak Hour Factor	0.92	0.92	0 92	0 92	0.92	0 92	0.92	0.92	0.92	0 92	0.92	0.92
Adi Flow (vph)	0.02	65	40	3	40	0.02	0.02	0.02	0.02	48	789	62
Shared Lane Traffic (%)	Ū	00	10	Ū	10	Ū	Ŭ	Ŭ	Ŭ	10	100	02
Lane Group Flow (vph)	0	65	40	3	40	0	0	0	0	0	837	62
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12	J -		12	J -		0	J -		0	J -
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.01	1.05	1.10	0.98	0.98	0.98	1.02	1.02	1.02	0.99	0.99	0.99
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2	1	1	2					1	2	1
Detector Template		Thru	Right	Left	Thru					Left	Thru	Right
Leading Detector (ft)		100	20	20	100					20	100	20
Trailing Detector (ft)		0	0	0	0					0	0	0
Detector 1 Position(ft)		0	0	0	0					0	0	0
Detector 1 Size(ft)		6	20	20	6					20	6	20
Detector 1 Type		CI+Ex	Cl+Ex	Cl+Ex	Cl+Ex					Cl+Ex	Cl+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 1 Queue (s)		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 1 Delay (s)		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		CI+Ex			CI+Ex						Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA	Perm	Perm	NA					Perm	NA	Perm
Turn Type		NA	Perm	Perm	NA					Perm	NA	Perm

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 47: S Center St & W Olive St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases		4			8						6	
Permitted Phases			4	8						6		6
Detector Phase		4	4	8	8					6	6	6
Switch Phase												
Minimum Initial (s)		5.0	5.0	5.0	5.0					5.0	5.0	5.0
Minimum Split (s)		13.5	13.5	13.5	13.5					37.9	37.9	37.9
Total Split (s)		21.0	21.0	21.0	21.0					59.0	59.0	59.0
Total Split (%)		26.3%	26.3%	26.3%	26.3%					73.8%	73.8%	73.8%
Maximum Green (s)		15.5	15.5	15.5	15.5					53.1	53.1	53.1
Yellow Time (s)		3.2	3.2	3.2	3.2					3.2	3.2	3.2
All-Red Time (s)		2.3	2.3	2.3	2.3					2.7	2.7	2.7
Lost Time Adjust (s)		0.0	0.0	0.0	0.0						0.0	0.0
Total Lost Time (s)		5.5	5.5	5.5	5.5						5.9	5.9
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0	3.0	3.0	3.0					3.0	3.0	3.0
Recall Mode		Max	Max	None	None					C-Max	C-Max	C-Max
Walk Time (s)		1.0	1.0	1.0	1.0					21.0	21.0	21.0
Flash Dont Walk (s)		7.0	7.0	7.0	7.0					11.0	11.0	11.0
Pedestrian Calls (#/hr)		0	0	0	0					0	0	0
Act Effct Green (s)		15.5	15.5	15.5	15.5						53.1	53.1
Actuated g/C Ratio		0.19	0.19	0.19	0.19						0.66	0.66
v/c Ratio		0.19	0.13	0.01	0.11						0.36	0.06
Control Delay		28.7	10.4	25.7	25.8						1.6	0.2
Queue Delay		0.0	0.0	0.0	0.0						0.0	0.0
Total Delay		28.7	10.4	25.7	25.8						1.6	0.2
LOS		С	В	С	С						А	A
Approach Delay		21.7			25.8						1.5	
Approach LOS		С			С						А	
90th %ile Green (s)		15.5	15.5	15.5	15.5					53.1	53.1	53.1
90th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
70th %ile Green (s)		15.5	15.5	15.5	15.5					53.1	53.1	53.1
70th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
50th %ile Green (s)		15.5	15.5	15.5	15.5					53.1	53.1	53.1
50th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
30th %ile Green (s)		15.5	15.5	15.5	15.5					53.1	53.1	53.1
30th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
10th %ile Green (s)		15.5	15.5	15.5	15.5					53.1	53.1	53.1
10th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
Stops (vph)		49	11	5	32						84	0
Fuel Used(gal)		1	0	0	0						5	0
CO Emissions (g/hr)		69	25	4	33						326	21
NOx Emissions (g/hr)		13	5	1	6						64	4
VOC Emissions (g/hr)		16	6	1	8						76	5
Dilemma Vehicles (#)		0	0	0	0						0	0
Queue Length 50th (ft)		27	0	1	17						12	0
Queue Length 95th (ft)		61	25	m5	m38						15	m0
Internal Link Dist (ft)		697			250			690			580	
Turn Bay Length (ft)			80									50

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 47: S Center St & W Olive St

02/22/2024

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Lane Group	EBL	EBT	EBR	• WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Base Capacity (vph)		347	317	261	366					-	2353	1074
Starvation Cap Reductn		0	0	0	0						0	0
Spillback Cap Reductn		0	0	0	0						0	0
Storage Cap Reductn		0	0	0	0						0	0
Reduced v/c Ratio		0.19	0.13	0.01	0.11						0.36	0.06
Intersection Summary												
Area Type:	Other											
Cycle Length: 80												
Actuated Cycle Length: 80)											
Offset: 21 (26%), Reference	ced to phase	2: and 6:	SBTL, Sta	art of Gre	en							
Natural Cycle: 55												
Control Type: Actuated-Co	oordinated											
Maximum v/c Ratio: 0.36												
Intersection Signal Delay:	4.5			In	tersectior	n LOS: A						
Intersection Capacity Utiliz	zation 43.8%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
m Volume for 95th perce	entile queue i	s metereo	l by upstr	eam sign	al.							
Splits and Phases: 47:	S Center St 8	W Olive	St									
								95 - C				95



Lanes, Volumes, Timings 48: S East St & E Olive St

02/22/2024

Lane Configurations EBL EBT EBR WBL WBT WBR NBT NBT NBR SBL SBT SBR Lane Configurations 4 1 17 26 31 1025 11 0 0 0 Fuller Volume (vph) 58 52 0 0 17 26 31 1025 11 0		٠	-	7	1	+	*	1	Ť	1	1	ŧ	~
Lane Configurations 4 1 41 - 41 - Traffic Volume (vph) 58 52 0 0 17 28 31 1025 11 0 <td< th=""><th>Lane Group</th><th>EBL</th><th>EBT</th><th>EBR</th><th>WBL</th><th>WBT</th><th>WBR</th><th>NBL</th><th>NBT</th><th>NBR</th><th>SBL</th><th>SBT</th><th>SBR</th></td<>	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph) 58 52 0 0 17 26 31 1025 11 0 0 0 ideal Flow (vphpt) 1900 100 10	Lane Configurations		÷.			≜ î≽			đ î je				
Future (vph) 58 52 0 0 17 26 31 1025 11 0 0 0 ideal Flow (vph) 1900	Traffic Volume (vph)	58	52	0	0	17	26	31	1025	11	0	0	0
ideal Flow (rphp) 1900 100	Future Volume (vph)	58	52	0	0	17	26	31	1025	11	0	0	0
Lane Width (ft) 12 14 12 12 12 12 12 12 12 12 12 12 12 12 12	Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%) 2% 4% 4% 4% 4% Storage Length (ft) 0 <	Lane Width (ft)	12	14	12	12	12	12	12	12	12	12	12	12
Shorage Length (ft) 0	Grade (%)		2%			-4%			4%			-4%	
Storage Lanes 0 <	Storage Length (ft)	0		0	0		0	300		0	0		0
Tape Length (ft) 25 25 25 25 25 Lane Uli, Factor 1.00 1.00 1.00 0.95 0.95 0.95 0.95 1.00 0	Storage Lanes	0		0	0		0	0		0	0		0
Lane Util. Factor 1.00 1.00 1.00 1.00 0.95 0.95 0.95 0.95 1.00 <td>Taper Length (ft)</td> <td>25</td> <td></td> <td></td> <td>25</td> <td></td> <td></td> <td>25</td> <td></td> <td></td> <td>25</td> <td></td> <td></td>	Taper Length (ft)	25			25			25			25		
Frt 0.909 0.999 0.999 FIP crotected 0.974 0.999 Satd. Flow (prot) 0 1675 0 0 3188 0 0 3359 0 0 0 0 FIP protected 0.809 0 0 3188 0 3359 0 0 0 0 Satd. Flow (prot) 0 1391 0 0 3188 0 3359 0 0 0 0 Satd. Flow (prot) 0 1391 0 0 3188 0 3359 0 0 0 0 Satd. Flow (prot) 30 28 78 Yes Yes <td>Lane Util. Factor</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>0.95</td> <td>0.95</td> <td>0.95</td> <td>0.95</td> <td>0.95</td> <td>1.00</td> <td>1.00</td> <td>1.00</td>	Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00
Fit Protected 0.974 0.999 Satd. Flow (prot) 0 1675 0 0 3188 0 0 3359 0 0 0 0 Right Turn on Red Yes 100	Frt					0.909			0.998				
Satid. Flow (prot) 0 1675 0 0 3188 0 0 3359 0 0 0 FILPermitted 0.809 0 3188 0 0 3359 0 0 0 Satid. Flow (perm) 0 1391 0 0 3188 0 0 3359 0 0 0 0 0 Repending (mph) 30 1331 0 0 3188 0 0 3359 0 0 0 0 0 Link Speed (mph) 30 28 75 14.7 155 14.4 14 2 0.92	Flt Protected		0.974						0.999				
Fit Permitted 0.809 0.999 Satd. Flow (perm) 0 1391 0 0 3188 0 0.3559 0 0 0 0 Satd. Flow (perm) 0 1391 0 0 3188 0 0 3559 0	Satd. Flow (prot)	0	1675	0	0	3188	0	0	3359	0	0	0	0
Satd. Flow (perm) 0 1391 0 0 3188 0 0 3359 0 0 0 0 Right Turn on Red Yes Yes <td>Flt Permitted</td> <td></td> <td>0.809</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>0.999</td> <td></td> <td></td> <td></td> <td></td>	Flt Permitted		0.809						0.999				
Right Turn on Red Yes Yes Yes Yes Yes Yes Satt. Flow (RTOR) 28 3 30 30 30 30 11 Link Speed (mph) 30 30 447 682 633 144 Ink Distance (t) 330 647 682 633 144 Peak Hour Factor 0.92<	Satd, Flow (perm)	0	1391	0	0	3188	0	0	3359	0	0	0	0
Satt. Flow (RTOR) 28 3 Link Speed (mph) 30 30 30 30 Link Distance (ft) 330 647 682 633 Travel Time (s) 7.5 14.7 15.5 14.4 Peak Hour Factor 0.92	Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph) 30 30 30 30 30 Link Distance (ft) 330 647 662 633 Travel Time (s) 7.5 14.7 15.5 14.4 Peak Hour Factor 0.92	Satd. Flow (RTOR)					28			3				
Link Distance (ft) 330 647 682 633 Travel Time (s) 7.5 14.7 15.5 14.4 Peak Hour Factor 0.92	Link Speed (mph)		30			30			30			30	
Travel Time (s) 7.5 14.7 15.5 14.4 Peak Hour Factor 0.92 0.	Link Distance (ft)		330			647			682			633	
Peak Hour Factor 0.92 <th0.92< th=""> 0.93 0.93</th0.92<>	Travel Time (s)		7.5			14.7			15.5			14.4	
Heavy Vehicles (%) 5% 5	Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Index (r) Ord	Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%
Adi, Flow (vph) 63 57 0 0 18 28 34 1114 12 0 0 0 Shared Lane Traffic (%) Lane Group Flow (vph) 0 120 0 0 46 0 0 1160 0 <td>Parking (#/hr)</td> <td>•,•</td> <td>0</td> <td>• / •</td> <td>• / •</td> <td>• / •</td> <td>• / •</td> <td>0,0</td> <td>• / •</td>	Parking (#/hr)	•,•	0	• / •	• / •	• / •	• / •	0,0	• / •	• / •	• / •	• / •	• / •
Shared Lane Traffic (%) Left Right Left Left Left Right Left Left Left Right Left Left Left Right Left Left Left Left Right Left Left Left Right Left Left Left Left Right Left Left Left Left Left Right Left Left Right Left Left Left Right Left Left <thleft< th=""> Left Left</thleft<>	Adi, Flow (vph)	63	57	0	0	18	28	34	1114	12	0	0	0
Detector Lane Group Flow (vph) 0 120 0 0 46 0 0 1160 0 0 0 0 Eane Group Flow (vph) 0 No No </td <td>Shared Lane Traffic (%)</td> <td></td> <td>01</td> <td>Ū</td> <td>•</td> <td>10</td> <td>20</td> <td>01</td> <td></td> <td></td> <td>•</td> <td>Ū</td> <td>U</td>	Shared Lane Traffic (%)		01	Ū	•	10	20	01			•	Ū	U
Enter Blocked Intersection No	Lane Group Flow (vph)	0	120	0	0	46	0	0	1160	0	0	0	0
Left Left Left Right Left Right <th< td=""><td>Enter Blocked Intersection</td><td>No</td><td>No</td><td>No</td><td>No</td><td>No</td><td>No</td><td>No</td><td>No</td><td>No</td><td>No</td><td>No</td><td>No</td></th<>	Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Land right Lon Lon <thlon< th=""> Lon <thlon< th=""> <thlon< td=""><td>Lane Alignment</td><td>Left</td><td>Left</td><td>Right</td><td>Left</td><td>Left</td><td>Right</td><td>Left</td><td>Left</td><td>Right</td><td>Left</td><td>Left</td><td>Right</td></thlon<></thlon<></thlon<>	Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Inix Offset(fi) 0 0 0 0 0 Crosswalk Width(ft) 16 16 16 16 16 Two way Left Turn Lane	Median Width(ft)		0			0			0		_0.1	0	
Crosswalk Width(ft) 16 16 16 Two way Left Turn Lane Image: Construct of the structure	Link Offset(ft)		0			0			0			0	
Two way Left Turn Lane 1.01 1.07 1.01 0.97 0.97 1.03 1.03 0.97 0.97 0.97 Turning Speed (mph) 15 9 15 15 10 10 100	Crosswalk Width(ft)		16			16			16			16	
Headway Factor 1.01 1.07 1.01 0.97 0.97 0.97 1.03 1.03 0.97 0.97 0.97 Turning Speed (mph) 15 9 15 15 15 10 15 15 10 10 10 100 100 100 100 100 100 100 100 15 10 15 10 15 10 15 10 15 10 15 10 15 10	Two way Left Turn Lane												
Turning Speed (mph) 15 9 15 16<	Headway Factor	1.01	1.07	1.01	0.97	0.97	0.97	1.03	1.03	1.03	0.97	0.97	0.97
Number of Detectors 1 2 1 2 1 2 Detector Template Left Thru Thru Left Thru Leading Detector (ft) 20 100 100 20 100 Trailing Detector (ft) 20 100 0 0 0 Detector 1 Position(ft) 0 0 0 0 0 Detector 1 Size(ft) 20 6 6 20 6 Detector 1 Channel	Turning Speed (mph)	15		9	15		9	15		9	15		9
Detector Template Left Thru Thru Left Thru Leading Detector (ft) 20 100 100 20 100 Trailing Detector (ft) 0 0 0 0 0 Detector 1 Position(ft) 0 0 0 0 0 Detector 1 Size(ft) 20 6 6 20 6 Detector 1 Size(ft) 20 6 Cl+Ex Cl+Ex Cl+Ex Detector 1 Channel	Number of Detectors	1	2	, , , , , , , , , , , , , , , , , , ,		2	•	1	2	•			•
Leading Detector (ft) 20 100 100 20 100 Trailing Detector (ft) 0 0 0 0 0 Detector 1 Position(ft) 0 0 0 0 0 Detector 1 Size(ft) 20 6 6 20 6 Detector 1 Size(ft) 20 6 6 20 6 Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 1 Channel	Detector Template	Left	Thru			Thru		l eft	Thru				
Trailing Detector (ft) 0 0 0 0 0 Detector 1 Position(ft) 0 0 0 0 0 Detector 1 Size(ft) 20 6 6 20 6 Detector 1 Size(ft) 20 6 6 20 6 Detector 1 Size(ft) 20 6 6 20 6 Detector 1 Type Cl+Ex Cl+Ex Cl+Ex Cl+Ex Detector 1 Channel	Leading Detector (ft)	20	100			100		20	100				
Detector 1 Position(ft) 0 0 0 0 0 0 Detector 1 Size(ft) 20 6 6 20 6 Detector 1 Size(ft) 20 6 6 20 6 Detector 1 Size(ft) 20 6 6 20 6 Detector 1 Type Cl+Ex Cl+Ex Cl+Ex Cl+Ex Cl+Ex Detector 1 Channel Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 94 94 94 94 Detector 2 Size(ft) 6 6 6 6 6 6 1 <th1< th=""> <th1< th=""> 1</th1<></th1<>	Trailing Detector (ft)		0			0		0	0				
Detector 1 Size(ft) 20 6 6 20 6 Detector 1 Size(ft) 20 6 6 20 6 Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex Detector 1 Channel 0.0 0.0 0.0 0.0 Detector 1 Extend (s) 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 94 Detector 2 Size(ft) 6 6 6 Detector 2 Type CI+Ex CI+Ex CI+Ex Detector 2 Type CI+Ex CI+Ex CI+Ex	Detector 1 Position(ft)	0	0			0		0	0				
Detector 1 Type Cl+Ex	Detector 1 Size(ft)	20	6			6		20	6				
Detector 1 Channel 0.0 0.0 0.0 0.0 0.0 Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 94 Detector 2 Size(ft) 6 6 6 Detector 2 Type CI+Ex CI+Ex CI+Ex	Detector 1 Type	CI+Ex	Cl+Ex			Cl+Ex		CI+Ex	Cl+Ex				
Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 94 94 Detector 2 Size(ft) 6 6 6 Detector 2 Type Cl+Ex Cl+Ex Cl+Ex	Detector 1 Channel	OI · EX	OI LA			OFFER		OFFER	OI' LA				
Detector 1 Queue (s) 0.0	Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 94 Detector 2 Size(ft) 6 6 6 Detector 2 Type CI+Ex CI+Ex CI+Ex	Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0				
Detector 2 Position(ft)9494Detector 2 Size(ft)66Detector 2 TypeCI+ExDetector 2 ChannelCI+Ex	Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0				
Detector 2 Size(ft) 6 6 Detector 2 Type CI+Ex CI+Ex Detector 2 Channel CI+Ex CI+Ex	Detector 2 Position(ft)	0.0	Q/			94		0.0	Q/				
Detector 2 Type CI+Ex CI+Ex CI+Ex	Detector 2 Size(ft)		6			6			6				
Detector 2 Channel	Detector 2 Type		CI+Ex			Cl+Ev			CI+Ex				
	Detector 2 Channel												

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 48: S East St & E Olive St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA		Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4				-		2					
Detector Phase	4	4			8		2	2				
Switch Phase	-				-							
Minimum Initial (s)	50	50			50		50	50				
Minimum Split (s)	17.5	17.5			17.5		60.6	60.6				
Total Split (s)	18.0	18.0			18.0		62.0	62.0				
Total Split (%)	22.5%	22.5%			22.5%		77.5%	77.5%				
Maximum Green (s)	12.5	12.5			12.5		56.4	56.4				
Yellow Time (s)	3.2	3.2			3.2		32	3.2				
All-Red Time (s)	2.3	2.3			2.3		2.4	2.4				
Lost Time Adjust (s)	2.0	0.0			0.0		2.1	0.0				
Total Lost Time (s)		5.5			5.5			5.6				
Lead/Lag		0.0			0.0			0.0				
Lead-Lag Ontimize?												
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0				
Recall Mode	May	Max			None		C-Max	C-Max				
Walk Time (s)	1 0	1 0			1 0							
Flach Dont Walk (c)	11.0	11.0			11.0		11.0	11.0				
Pedestrian Calls (#/br)	0.11	0			0		0	0				
Act Effet Green (s)	0	12.5			12.5		0	56 /				
Actuated a/C Patio		0.16			0.16			0.70				
Actualed g/C Ratio		0.10			0.10			0.70				
Control Dolay		12.3			16.6			0.49				
		43.3			10.0			0.1				
Total Delay		13.3			16.6			0.0				
		43.5			10.0 D			0.1				
LUS Approach Dolov		12.2			16.6			A 6.1				
Approach LOS		43.3			10.0 D			0.1				
Approach LOS	10 5	12.5			12.5		56 A	56 A				
90th %ile Green (S)	IZ.J MayD	IZ.0 MaxD			IZ.U		Coord	00.4				
							C0010					
70th %ile Green (S)	IZ.J	IZ.3			C.2I		50.4 Coord	00.4				
Foth %ile Crean (a)								C0010				
50th %ile Green (S)	IZ.3	IZ.5			12.5		50.4 Coord	50.4 Coord				
					100							
30th %ile Green (s)	12.5 MaxiD	12.5 MaxiD			12.5		50.4	56.4				
30th %ile Term Code					HOID							
10th %ile Green (s)	12.5 MaxiD	12.5 MaxiD			12.5		50.4	56.4				
10th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
Stops (vpn)		107			21			430				
Fuel Used(gal)		2			0			9				
CO Emissions (g/hr)		129			33			656				
NUX Emissions (g/hr)		25			6			128				
VUC Emissions (g/hr)		30			8			152				
Dilemma Vehicles (#)		0			0			0				
Queue Length 50th (ft)		65			4			113				
Queue Length 95th (ft)		#120			18			150				

2044 AM Downtown Bloomington 7:06 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 48: S East St & E Olive St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		250			567			602			553	
Turn Bay Length (ft)												
Base Capacity (vph)		217			521			2368				
Starvation Cap Reductn		0			0			0				
Spillback Cap Reductn		0			0			0				
Storage Cap Reductn		0			0			0				
Reduced v/c Ratio		0.55			0.09			0.49				
Intersection Summary												
Area Type: C	other											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 55 (69%), Referenced	to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 80												
Control Type: Actuated-Coord	dinated											
Maximum v/c Ratio: 0.55												
Intersection Signal Delay: 9.9	1			In	tersectior	n LOS: A						
Intersection Capacity Utilization	on 51.4%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
# 95th percentile volume ex	ceeds cap	bacity, qu	eue may	be longer								
Queue shown is maximum	n after two	cycles.										
0.111 1.01 (0.05		<u></u>										

Splits and Phases: 48: S East St & E Olive St

Ø2 (R)	 A ₀₄
62 s	18 s
	← Ø8
	18 s

02/22/2024

Lanes, Volumes, Timings 3: N Madison St/N Center St & W Locust St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		† 1 ₂			ŧ						-fî†	7
Traffic Volume (vph)	0	491	69	4	63	0	0	0	0	216	1306	182
Future Volume (vph)	0	491	69	4	63	0	0	0	0	216	1306	182
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	0		0	0		150
Storage Lanes	0		0	0		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00
Ped Bike Factor		1.00									1.00	
Frt		0.982										0.850
Flt Protected					0.997						0.993	
Satd. Flow (prot)	0	3468	0	0	1857	0	0	0	0	0	3514	1583
Flt Permitted					0.965						0.993	
Satd. Flow (perm)	0	3468	0	0	1798	0	0	0	0	0	3507	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd, Flow (RTOR)		21										194
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		659			244			860			526	
Travel Time (s)		15.0			5.5			19.5			12.0	
Confl. Peds. (#/hr)			4		0.0					12		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adi, Flow (vph)	0	522	73	4	67	0	0	0	0	230	1389	194
Shared Lane Traffic (%)	•			•	•.	•	•	•	•			
Lane Group Flow (vph)	0	595	0	0	71	0	0	0	0	0	1619	194
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2	-	1	2	-			-	1	2	1
Detector Template		Thru		Left	Thru					Left	Thru	Right
Leading Detector (ft)		100		20	100					20	100	20
Trailing Detector (ft)		0		0	0					0	0	0
Detector 1 Position(ft)		0		0	0					0	0	0
Detector 1 Size(ft)		6		20	6					20	6	20
Detector 1 Type		Cl+Ex		CI+Ex	CI+Ex					CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel		0 . 1 .		0/	•					•	• . • .	0. 2.0
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	0.0
Detector 2 Position(ft)		94		0.0	94					0.0	94	0.0
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		CI+Ex			Cl+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	Perm
				i Giiii								

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 3: N Madison St/N Center St & W Locust St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases		4			8						6	
Permitted Phases				8						6		6
Detector Phase		4		8	8					6	6	6
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	5.0
Minimum Split (s)		23.4		22.5	22.5					23.4	23.4	23.4
Total Split (s)		23.6		22.5	22.5					46.4	46.4	46.4
Total Split (%)		33.7%		32.1%	32.1%					66.3%	66.3%	66.3%
Maximum Green (s)		18.2		18.0	18.0					41.0	41.0	41.0
Yellow Time (s)		3.2		3.5	3.5					3.2	3.2	3.2
All-Red Time (s)		2.2		1.0	1.0					2.2	2.2	2.2
Lost Time Adjust (s)		0.0			0.0						0.0	0.0
Total Lost Time (s)		5.4			4.5						5.4	5.4
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	3.0
Recall Mode		None		None	None					C-Max	C-Max	C-Max
Walk Time (s)		7.0		7.0	7.0					7.0	7.0	7.0
Flash Dont Walk (s)		11.0		11.0	11.0					11.0	11.0	11.0
Pedestrian Calls (#/hr)		0		0	0					0	0	0
Act Effct Green (s)		16.4			17.3						42.8	42.8
Actuated g/C Ratio		0.23			0.25						0.61	0.61
v/c Ratio		0.72			0.16						0.75	0.19
Control Delay		29.0			13.8						13.2	1.6
Queue Delay		0.8			0.0						2.3	0.0
Total Delay		29.8			13.8						15.5	1.6
LOS		С			В						В	А
Approach Delay		29.8			13.8						14.0	
Approach LOS		С			В						В	
90th %ile Green (s)		18.2		19.1	19.1					41.0	41.0	41.0
90th %ile Term Code		Max		Hold	Hold					Coord	Coord	Coord
70th %ile Green (s)		18.2		19.1	19.1					41.0	41.0	41.0
70th %ile Term Code		Max		Hold	Hold					Coord	Coord	Coord
50th %ile Green (s)		17.5		18.4	18.4					41.7	41.7	41.7
50th %ile Term Code		Gap		Hold	Hold					Coord	Coord	Coord
30th %ile Green (s)		15.7		16.6	16.6					43.5	43.5	43.5
30th %ile Term Code		Gap		Hold	Hold					Coord	Coord	Coord
10th %ile Green (s)		12.3		13.2	13.2					46.9	46.9	46.9
10th %ile Term Code		Gap		Hold	Hold					Coord	Coord	Coord
Stops (vph)		475			29						1044	14
Fuel Used(gal)		9			0						16	1
CO Emissions (g/hr)		615			33						1125	62
NOx Emissions (g/hr)		120			6						219	12
VOC Emissions (g/hr)		142			8						261	14
Dilemma Vehicles (#)		0			0						0	0
Queue Length 50th (ft)		117			8						245	0
Queue Length 95th (ft)		167			m18						342	22
Internal Link Dist (ft)		579			164			780			446	
Turn Bay Length (ft)												150

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 3: N Madison St/N Center St & W Locust St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Base Capacity (vph)		917			490						2145	1043
Starvation Cap Reductn		0			0						0	0
Spillback Cap Reductn		117			0						377	0
Storage Cap Reductn		0			0						0	0
Reduced v/c Ratio		0.74			0.14						0.92	0.19
Intersection Summary												
Area Type:	Other											
Cycle Length: 70												
Actuated Cycle Length: 70												
Offset: 45 (64%), Referenc	ed to phase	2: and 6:	SBTL, Sta	art of Gre	en							
Natural Cycle: 60												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.75												
Intersection Signal Delay: 1	17.8			In	tersectior	n LOS: B						
Intersection Capacity Utiliza	ation 67.2%			IC	U Level o	of Service	С					
Analysis Period (min) 15												
m Volume for 95th perce	ntile queue is	s metereo	l by upstr	eam sign	al.							

Splits and Phases: 3: N Madison St/N Center St & W Locust St



02/22/2024

Lanes, Volumes, Timings <u>6: N East St/N Main St & W Locust St/E Locust St</u>

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		.att						412				
Traffic Volume (vph)	113	599	0	0	0	0	67	1043	231	0	0	0
Future Volume (vph)	113	599	0	0	0	0	67	1043	231	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	0.95	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor		1.00						1.00				
Frt								0.974				
Flt Protected		0.992						0.998				
Satd. Flow (prot)	0	3511	0	0	0	0	0	3431	0	0	0	0
Flt Permitted		0.992						0.998				
Satd. Flow (perm)	0	3508	0	0	0	0	0	3431	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)								41				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		244			514			878			525	
Travel Time (s)		5.5			11.7			20.0			11.9	
Confl. Peds. (#/hr)	5								3			
Confl. Bikes (#/hr)									1			
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	122	644	0	0	0	0	72	1122	248	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	766	0	0	0	0	0	1442	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0	Ū		0	Ŭ		0	Ū		0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2					1	2				
Detector Template	Left	Thru					Left	Thru				
Leading Detector (ft)	20	100					20	100				
Trailing Detector (ft)	0	0					0	0				
Detector 1 Position(ft)	0	0					0	0				
Detector 1 Size(ft)	20	6					20	6				
Detector 1 Type	Cl+Ex	Cl+Ex					CI+Ex	CI+Ex				
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0				
Detector 1 Queue (s)	0.0	0.0					0.0	0.0				
Detector 1 Delay (s)	0.0	0.0					0.0	0.0				
Detector 2 Position(ft)		94						94				
Detector 2 Size(ft)		6						6				
Detector 2 Type		CI+Ex						CI+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0				
Turn Type	Perm	NA					Perm	NA				
Protected Phases		4						2				
Permitted Phases	4						2					

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings	
6: N East St/N Main St & W Locust St/E Locus	st St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4					2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0					5.0	5.0				
Minimum Split (s)	23.5	23.5					23.5	23.5				
Total Split (s)	24.0	24.0					46.0	46.0				
Total Split (%)	34.3%	34.3%					65.7%	65.7%				
Maximum Green (s)	18.5	18.5					40.5	40.5				
Yellow Time (s)	32	32					32	32				
All-Red Time (s)	2.3	2.3					2.3	2.3				
Lost Time Adjust (s)	2.0	0.0					2.0	0.0				
Total Lost Time (s)		5.5						5.5				
Lead/Lag		0.0						0.0				
Lead-Lag Optimize?												
Vehicle Extension (s)	30	3.0					3.0	3.0				
Recall Mode	None	None					C-Max	C-Max				
Walk Time (s)	7.0	7.0					7.0	7.0				
Flash Dont Walk (s)	11.0	11.0					11.0	11.0				
Pedestrian Calls (#/hr)	0	0					0	0				
Act Effet Green (s)	U	18.1					U	40.9				
Actuated a/C Ratio		0.26						0.58				
v/c Ratio		0.20						0.50				
Control Delay		34.4						7.6				
		50.0						7.0				
Total Delay		84.4						7.6				
		04.4 E						7.0				
LUG Approach Dolay		Q/ /						76				
Approach LOS		04.4 E						7.0				
Approach LOS	10 5	10 E					40 F	40 F				
90th %ile Green (s)	TO.0	TO.0					40.5	40.5				
70th %ile Croon (c)	19.5	19.5					40.5	40 F				
70th %ile Green (S)	10.0 Mov	TO.0 Mox					40.5	40.0				
Foth %ile Croop (a)	10 5	10 5					40.5	40.5				
50th %ile Green (S)	10.0 Mov	TO.0 Mox					40.5	40.0				
20th %ile Croop (a)							40 5	40 5				
20th %ile Green (S)	10.0 Mov	TO.U Max					40.5 Coord	40.0 Coord				
10th %ile Croop (a)							42.4	42.4				
10th %ile Green (S)	10.0 Con	10.0					4Z.4	42.4				
Stone (uph)	Gap	Gap					Coord	614				
		000						014				
Fuel Used(gal)		10						1005				
CO Emissions (g/nr)		101						1025				
NOX Emissions (g/hr)		130						199				
VUC Emissions (g/nr)		162						237				
Dilemma Venicles (#)		475						0				
		1/5						11				
Queue Length 95th (ft)		#257			40.4			104			445	
Internal LINK Dist (ft)		164			434			798			445	
Turn Bay Length (ft)		007						0000				
Base Capacity (vph)		927						2020				
Starvation Cap Reductn		262						0				

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE
Lanes, Volumes, Timings
6: N East St/N Main St & W Locust St/E Locust St

02/22/2024	
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Spillback Cap Reductn		0						0				
Storage Cap Reductn		0						0				
Reduced v/c Ratio		1.15						0.71				
Intersection Summary												
Area Type: C	Other											
Cycle Length: 70												
Actuated Cycle Length: 70												
Offset: 10 (14%), Referenced	I to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 60												
Control Type: Actuated-Coor	dinated											
Maximum v/c Ratio: 0.84												
Intersection Signal Delay: 34	.3			In	tersectior	n LOS: C						
Intersection Capacity Utilizati	on 67.2%			IC	U Level o	of Service	С					
Analysis Period (min) 15												
# 95th percentile volume ex	ceeds ca	bacity, qu	eue may	be longer								
Queue shown is maximun	n after two	cycles.										
Splits and Phases: 6: N Ea	ast St/N Ma	ain St & V	/ Locust \$	St/E Locu	st St							

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46 s	24 s	

Lanes, Volumes, Timings 9: N Madison St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1.		٦	†						4 P	
Traffic Volume (vph)	0	137	119	27	119	0	0	0	0	13	1029	162
Future Volume (vph)	0	137	119	27	119	0	0	0	0	13	1029	162
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	11	12	12	11	12	12	12	12	12	12	12
Grade (%)		1%			-2%			-6%			4%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		0.99		1.00							1.00	
Frt		0.937									0.980	
Flt Protected				0.950							0.999	
Satd. Flow (prot)	0	1501	0	1609	1637	0	0	0	0	0	3046	0
Flt Permitted				0.403							0.999	
Satd. Flow (perm)	0	1501	0	682	1637	0	0	0	0	0	3046	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		60									44	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		705			247			400			860	
Travel Time (s)		16.0			5.6			9.1			19.5	
Confl. Peds. (#/hr)			1	1								4
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Parking (#/hr)										20		20
Adj. Flow (vph)	0	146	127	29	127	0	0	0	0	14	1095	172
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	273	0	29	127	0	0	0	0	0	1281	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.15	1.20	1.15	1.13	1.18	1.13	1.10	1.10	1.10	1.17	1.17	1.17
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		Cl+Ex	CI+Ex					Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		Cl+Ex			Cl+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 9: N Madison St & W Market St

02/22/2024	02	122	2	024
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		17.4		17.4	17.4					45.2	45.2	
Total Split (s)		23.0		23.0	23.0					47.0	47.0	
Total Split (%)		32.9%		32.9%	32.9%					67.1%	67.1%	
Maximum Green (s)		17.6		17.6	17.6					41.8	41.8	
Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)		2.2		2.2	2.2					2.0	2.0	
Lost Time Adjust (s)		0.0		0.0	0.0						0.0	
Total Lost Time (s)		5.4		5.4	5.4						5.2	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		None	None					C-Max	C-Max	
Walk Time (s)		1.0		1.0	1.0					29.0	29.0	
Flash Dont Walk (s)		11.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		14.3		14.3	14.3						45.1	
Actuated g/C Ratio		0.20		0.20	0.20						0.64	
v/c Ratio		0.77		0.21	0.38						0.65	
Control Delay		34.9		25.3	26.3						1.7	
Queue Delay		0.0		0.0	0.0						0.0	
Total Delay		34.9		25.3	26.3						1.7	
LOS		С		С	С						А	
Approach Delay		34.9			26.1						1.7	
Approach LOS		С			С						А	
90th %ile Green (s)		17.6		17.6	17.6					41.8	41.8	
90th %ile Term Code		Max		Hold	Hold					Coord	Coord	
70th %ile Green (s)		17.6		17.6	17.6					41.8	41.8	
70th %ile Term Code		Max		Hold	Hold					Coord	Coord	
50th %ile Green (s)		15.5		15.5	15.5					43.9	43.9	
50th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
30th %ile Green (s)		12.6		12.6	12.6					46.8	46.8	
30th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
10th %ile Green (s)		8.3		8.3	8.3					51.1	51.1	
10th %ile Term Code		Gap		Hold	Hold					Coord	Coord	
Stops (vph)		186		23	97						48	
Fuel Used(gal)		4		0	1						9	
CO Emissions (g/hr)		298		22	98						612	
NOx Emissions (g/hr)		58		4	19						119	
VOC Emissions (g/hr)		69		5	23						142	
Dilemma Vehicles (#)		0		0	0						0	
Queue Length 50th (ft)		85		10	46						12	
Queue Length 95th (ft)		158		31	88						14	
Internal Link Dist (ft)		625			167			320			780	
Turn Bay Length (ft)												

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 9: N Madison St & W Market St

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			•	•		10)	22.63	1		•	100
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Base Capacity (vph)		422		171	411						1977	
Starvation Cap Reductn		0		0	0						0	
Spillback Cap Reductn		0		0	0						0	
Storage Cap Reductn		0		0	0						0	
Reduced v/c Ratio		0.65		0.17	0.31						0.65	
Intersection Summary												
Area Type:	CBD											
Cycle Length: 70												
Actuated Cycle Length: 7	0											
Offset: 64 (91%), Referen	ced to phase	2: and 6:	SBTL, Sta	art of Gre	en							
Natural Cycle: 65												
Control Type: Actuated-C	oordinated											
Maximum v/c Ratio: 0.77												
Intersection Signal Delay:	9.2			In	tersection	n LOS: A						
Intersection Capacity Utili	zation 71.5%			IC	U Level	of Service	С					
Analysis Period (min) 15												

Splits and Phases: 9: N Madison St & W Market St

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	23 s
Ø6 (R)	₹Ø8
47 s	23 s

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	3		HUE	41		
Traffic Volume (vph)	152	0	139	1114	0	0
Future Volume (vph)	152	0	139	1114	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	13	12	12	11	12	12
Grade (%)	0%	14	12	_2%	2%	12
Lane I Itil Eactor	1 00	1 00	0.05	0.05	1 00	1 00
Pad Pike Easter	0.08	1.00	0.95	1.00	1.00	1.00
	0.90			1.00		
Fit Protected	0 050			0 001		
Satd Elow (prot)	1/01	0	0	0.994	0	0
Salu. Flow (pill)	1401	U	U	2/02	U	U
	0.950	0	0	0.994	0	0
Satu. Flow (perm)	1447	U	U	2782	U	U
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)	~~			~~	~~	
Link Speed (mph)	30			30	30	
Link Distance (ft)	263			127	878	
Travel Time (s)	6.0			2.9	20.0	
Confl. Peds. (#/hr)	11		2			
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Parking (#/hr)	0		20	20		
Adj. Flow (vph)	162	0	148	1185	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	162	0	0	1333	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	13	0		0	0	0
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane	10			10	10	
Headway Eactor	1 25	1 14	1 13	1 34	1 16	1 16
Turning Speed (mph)	1.20	1.1 4 Q	1.10	1.04	1.10	0
Number of Detectors	1	9	1	0		9
Detector Template	l off		l off	2 Thru		
	20		20	100		
Trailing Detector (II)	20		20	100		
Detector (II)	0		0	0		
Detector 1 Position(II)	0		U	0		
Detector 1 Size(tt)	20		20	6		
Detector 1 lype	CI+Ex		CI+Ex	CI+Ex		
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0		
Detector 1 Queue (s)	0.0		0.0	0.0		
Detector 1 Delay (s)	0.0		0.0	0.0		
Detector 2 Position(ft)				94		
Detector 2 Size(ft)				6		
Detector 2 Type				CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)				0.0		
Turn Type	Prot		Perm	NA		

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Protected Phases	4			2		0.511
Permitted Phases	7		2	2		
Detector Phase	Δ		2	2		
Switch Phase	т		2	2		
Minimum Initial (c)	5.0		5.0	5.0		
Minimum Calit (s)	J.U		5.0 47.5	5.0 47 E		
Minimum Spiit (S)	15.4		47.5	47.5		
	21.0		49.0	49.0		
Total Split (%)	30.0%		70.0%	70.0%		
Maximum Green (s)	15.6		43.5	43.5		
Yellow I ime (s)	3.2		3.2	3.2		
All-Red Time (s)	2.2		2.3	2.3		
Lost Time Adjust (s)	0.0			0.0		
Total Lost Time (s)	5.4			5.5		
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0		3.0	3.0		
Recall Mode	None		C-Max	C-Max		
Walk Time (s)	1.0		31.0	31.0		
Flash Dont Walk (s)	9.0		11.0	11.0		
Pedestrian Calls (#/br)	0		0	0		
Act Effct Green (s)	12.3		Ű	46.8		
Actuated g/C Ratio	0.18			0.67		
v/c Ratio	0.10			0.07		
Control Delay	37.0			10.72		
	0.0			0.0		
Total Dolay	37.0			10.0		
	37.0			10.9		
LUS Anna ach Dalau	D			D		
Approach Delay	37.0			10.9		
Approach LUS	D		10 -	B		
90th %ile Green (s)	15.6		43.5	43.5		
90th %ile Term Code	Max		Coord	Coord		
70th %ile Green (s)	15.0		44.1	44.1		
70th %ile Term Code	Gap		Coord	Coord		
50th %ile Green (s)	12.8		46.3	46.3		
50th %ile Term Code	Gap		Coord	Coord		
30th %ile Green (s)	10.6		48.5	48.5		
30th %ile Term Code	Gap		Coord	Coord		
10th %ile Green (s)	7.5		51.6	51.6		
10th %ile Term Code	Gap		Coord	Coord		
Stops (vph)	135			772		
Fuel Used(gal)	2			8		
CO Emissions (a/hr)	154			580		
NOv Emissions (g/hr)	30			113		
VOC Emissions (g/hr)	36			12/		
Dilomma Vohiolog (4)				134		
Ouque Length E0th (#)	65			165		
	00			105		
Queue Length 95th (ft)	117			2/8	700	
Internal Link Dist (ft)	183			47	798	
Turn Bay Length (ft)						

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

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			-				
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Base Capacity (vph)	330			1859			
Starvation Cap Reductn	0			0			
Spillback Cap Reductn	0			0			
Storage Cap Reductn	0			0			
Reduced v/c Ratio	0.49			0.72			
Intersection Summary							
Area Type:	CBD						
Cycle Length: 70							
Actuated Cycle Length: 70)						
Offset: 64 (91%), Referen	ced to phase	2:NBTL a	nd 6:, Sta	art of Gree	en		
Natural Cycle: 65							
Control Type: Actuated-C	oordinated						
Maximum v/c Ratio: 0.72							
Intersection Signal Delay:	13.7			Int	tersectior	n LOS: B	
Intersection Capacity Utiliz	zation 57.1%			IC	U Level o	of Service B	
Analysis Period (min) 15							

Splits and Phases: 11: N East St & E Market St

	, 1941 - 1949	▶ Ø4	
49 s		21 s	

Lanes, Volumes, Timings 12: N Madison St & W Monroe St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ħ			÷.						đ î ja	
Traffic Volume (vph)	0	10	0	35	28	0	0	0	0	3	1168	7
Future Volume (vph)	0	10	0	35	28	0	0	0	0	3	1168	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	11	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			-3%			6%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt											0.999	
Flt Protected					0.973							
Satd. Flow (prot)	0	1621	0	0	1631	0	0	0	0	0	3087	0
Flt Permitted					0.973							
Satd. Flow (perm)	0	1621	0	0	1631	0	0	0	0	0	3087	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		699			209			307			400	
Travel Time (s)		15.9			4.8			7.0			9.1	
Confl. Peds. (#/hr)										4		2
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Adj. Flow (vph)	0	11	0	39	31	0	0	0	0	3	1312	8
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	11	0	0	70	0	0	0	0	0	1323	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.19	1.14	1.14	1.14	1.14	1.12	1.12	1.12	1.19	1.19	1.19
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Free	
Intersection Summary												
Area Type:	CBD											
Control Type: Unsignalized												

Intersection Capacity Utilization 53.3%

ICU Level of Service A

Analysis Period (min) 15

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Lane Group	EBI	EBR	NBI	NBT	SBT	SBR
Lane Configurations	3			At⊾		
Traffic Volume (vph)	54	0	21	1165	0	0
Future Volume (vph)	54	0	21	1165	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	11	12	12
Grade (%)	-2%	12	14	-3%	4%	12
Lane I Itil Factor	1.00	1.00	0.95	0.05	1 00	1 00
Ped Rike Factor	0.00	1.00	0.30	1.00	1.00	1.00
Frt	0.90			1.00		
Fit Protected	0.050			0 000		
Cated Flow (prot)	1440	0	0	0.999	٥	0
Salu. Flow (prot)	1440	U	U	2010	U	U
Fit Permitted	0.950	^	^	0.999	^	^
Satd. Flow (perm)	1426	0	0	2810	0	0
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)						
Link Speed (mph)	30			30	30	
Link Distance (ft)	221			318	266	
Travel Time (s)	5.0			7.2	6.0	
Confl. Peds. (#/hr)	5		7			
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Parking (#/hr)	0		20	20		
Adi, Flow (vph)	56	0	22	1201	0	0
Shared Lane Traffic (%)		v			v	v
Lane Group Flow (vph)	56	0	0	1223	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alianment	Loft	Right	Loff	Loft	Loft	Right
	10	Night	Leit			Ngnt
link Offect(ft)	12			0	0	
	0			0	0	
Crosswalk Width(ft)	16			16	16	
I wo way Left Turn Lane						
Headway Factor	1.29	1.13	1.12	1.34	1.17	1.17
Turning Speed (mph)	15	9	15			9
Number of Detectors	1		1	2		
Detector Template	Left		Left	Thru		
Leading Detector (ft)	20		20	100		
Trailing Detector (ft)	0		0	0		
Detector 1 Position(ft)	0		0	0		
Detector 1 Size(ft)	20		20	6		
Detector 1 Type			CI+Ev	CI+Ev		
Detector 1 Channel						
Detector 1 Extend (a)	0.0		0.0	0.0		
Detector 1 Extend (S)	0.0		0.0	0.0		
Detector I Queue (s)	0.0		0.0	0.0		
Detector 1 Delay (s)	0.0		0.0	0.0		
Detector 2 Position(ft)				94		
Detector 2 Size(ft)				6		
Detector 2 Type				CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)				0.0		
Turn Type	Prot		Perm	NA		

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

	٠	11	†	+	1	
Lane Group	FBL	EBR NBL	NBT	SBT	SBR	
Protected Phases	4		2			
Permitted Phases		2	-			
Detector Phase	4	2	2			
Switch Phase	•	-	-			
Minimum Initial (s)	5.0	50	5.0			
Minimum Split (s)	13.2	49.2	49.2			
Total Split (s)	17.0	63.0	63.0			
Total Split (%)	21.3%	78.8%	78.8%			
Maximum Green (s)	11.8	57.8	57.8			
Yellow Time (s)	32	32	3.2			
All-Red Time (s)	2.0	2.0	2.0			
Lost Time Adjust (s)	0.0	2.0	0.0			
Total Lost Time (s)	5.0		5.2			
Lead/Lag	0.2		0.2			
Lead-Lag Ontimize?						
Vehicle Extension (s)	3.0	3 0	3.0			
Recall Mode	None	C-May	C-May			
Walk Time (s)		22.0	33.0			
Flash Dont Walk (s)	7.0	11 0	11.0			
Pedestrian Calls (#/hr)	0.7	0	0			
Act Effet Green (s)	85	0	67.8			
Actuated a/C Ratio	0.0		01.0			
v/c Ratio	0.11		0.00			
Control Delay	39.1		1.2			
	0.0		0.0			
Total Delay	39.1		1.2			
	00.1 D		Δ			
Approach Delay	39.1		12			
Approach LOS	00.1 D		Δ			
90th %ile Green (s)	11.8	57.8	57.8			
90th %ile Term Code	Max	Coord	Coord			
70th %ile Green (s)	9.8	50.8	59.8			
70th %ile Term Code	Gan	Coord	Coord			
50th %ile Green (s)	8.4	61 2	61.2			
50th %ile Term Code	Gan	Coord	Coord			
30th %ile Green (s)	0.0	74.8	74.8			
30th %ile Term Code	Skin	Coord	Coord			
10th %ile Green (s)	0.0	74 8	74.8			
10th %ile Term Code	Skip	- Coord	Coord			
Stops (vph)	49	00010	44			
Fuel Used(gal)	1		3			
CO Emissions (a/hr)	55		242			
NOx Emissions (g/hr)	11		47			
VOC Emissions (g/hr)	13		56			
Dilemma Vehicles (#)	0		0			
Oueue Length 50th (ft)			16			
Queue Length 95th (ft)	50		21			
Internal Link Dist (ft)	141		238	186		
Turn Bay Length (ft)	171		200	100		

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Base Capacity (vph)	213			2380			
Starvation Cap Reductn	0			0			
Spillback Cap Reductn	0			0			
Storage Cap Reductn	0			0			
Reduced v/c Ratio	0.26			0.51			
Intersection Summary							
Area Type:	CBD						
Cycle Length: 80							
Actuated Cycle Length: 80)						
Offset: 59 (74%), Referen	ced to phase	2:NBTL a	nd 6:, Sta	art of Gre	en		
Natural Cycle: 65							
Control Type: Actuated-Co	oordinated						
Maximum v/c Ratio: 0.51							
Intersection Signal Delay:	2.8			In	tersectior	n LOS: A	
Intersection Capacity Utiliz	zation 49.3%			IC	U Level o	of Service A	
Analysis Period (min) 15							

Splits and Phases: 13: N East St & Monroe St

Lanes, Volumes, Timings 15: N Madison St & W Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1.			र्स						412	
Traffic Volume (vph)	0	21	10	28	23	0	0	0	0	20	1156	19
Future Volume (vph)	0	21	10	28	23	0	0	0	0	20	1156	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	16	12	12	14	12	12	12	12	12	12	12
Grade (%)		4%			-3%			2%			2%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		0.99			0.99						1.00	
Frt		0.956									0.998	
Flt Protected					0.973						0.999	
Satd. Flow (prot)	0	1768	0	0	1589	0	0	0	0	0	3144	0
Flt Permitted					0.832						0.999	
Satd. Flow (perm)	0	1768	0	0	1352	0	0	0	0	0	3144	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		12									4	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		692			253			310			307	
Travel Time (s)		15.7			5.8			7.0			7.0	
Confl. Peds. (#/hr)			4	4						2		
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Parking (#/hr)					0					20		20
Adi, Flow (vph)	0	25	12	33	27	0	0	0	0	24	1360	22
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	37	0	0	60	0	0	0	0	0	1406	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0	Ŭ		0	J		12	Ŭ		12	Ū
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.17	1.00	1.17	1.12	1.18	1.12	1.16	1.16	1.16	1.16	1.16	1.16
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		CI+Ex	CI+Ex					Cl+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		CI+Ex			CI+Ex						CI+Ex	
Detector 2 Channel											<u>-</u>	
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	
		1.0.1										

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 15: N Madison St & W Jefferson St

02/22/	2024
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase				-						-		
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		15.1		15.4	15.4					47.4	47.4	
Total Split (s)		20.0		20.0	20.0					60.0	60.0	
Total Split (%)		25.0%		25.0%	25.0%					75.0%	75.0%	
Maximum Green (s)		14.9		14.6	14.6					54.6	54.6	
Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)		1.9		2.2	2.2					2.2	2.2	
Lost Time Adjust (s)		0.0			0.0						0.0	
Total Lost Time (s)		5.1			5.4						5.4	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		Max		None	None					C-Max	C-Max	
Walk Time (s)		1.0		1.0	1.0					31.0	31.0	
Flash Dont Walk (s)		9.0		9.0	9.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		14.9		-	14.6					-	54.6	
Actuated g/C Ratio		0.19			0.18						0.68	
v/c Ratio		0.11			0.24						0.65	
Control Delay		21.4			31.0						9.1	
Queue Delav		0.0			0.0						0.0	
Total Delay		21.4			31.0						9.1	
LOS		С			С						A	
Approach Delay		21.4			31.0						9.1	
Approach LOS		С			С						A	
90th %ile Green (s)		14.9		14.6	14.6					54.6	54.6	
90th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
70th %ile Green (s)		14.9		14.6	14.6					54.6	54.6	
70th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
50th %ile Green (s)		14.9		14.6	14.6					54.6	54.6	
50th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
30th %ile Green (s)		14.9		14.6	14.6					54.6	54.6	
30th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
10th %ile Green (s)		14.9		14.6	14.6					54.6	54.6	
10th %ile Term Code		MaxR		Hold	Hold					Coord	Coord	
Stops (vph)		21			44						629	
Fuel Used(gal)		0			1						9	
CO Emissions (g/hr)		29			47						598	
NOx Emissions (g/hr)		6			9						116	
VOC Emissions (g/hr)		7			11						139	
Dilemma Vehicles (#)		0			0						0	
Queue Length 50th (ft)		10			26						180	
Queue Length 95th (ft)		33			56						215	
Internal Link Dist (ft)		612			173			230			227	
Turn Bay Length (ft)												

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 15: N Madison St & W Jefferson St

02/22/2024	
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Lane Group	EBL	ERI	EBK	VVBL	WRI	WBK	INBL	INR I	NBK	SBL	SBI	SBR
Base Capacity (vph)		339			246						2147	
Starvation Cap Reductn		0			0						0	
Spillback Cap Reductn		0			0						0	
Storage Cap Reductn		0			0						0	
Reduced v/c Ratio		0.11			0.24						0.65	
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 0 (0%), Referenced	I to phase 2:	and 6:SB	TL, Start	of Green								
Natural Cycle: 65												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.65												
Intersection Signal Delay:	10.3			In	tersectior	n LOS: B						
Intersection Capacity Utiliz	ation 55.6%			IC	U Level o	of Service	В					
Analysis Period (min) 15												

Splits and Phases: 15: N Madison St & W Jefferson St

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	20 s
Ø6 (R)	₹ø8
60 s	20 s

Lanes, Volumes, Timings 16: N East St & E Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			T.			4 P				
Traffic Volume (vph)	29	28	0	0	25	23	23	1141	7	0	0	0
Future Volume (vph)	29	28	0	0	25	23	23	1141	7	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	11	11	12	12	12
Grade (%)		-3%			3%			0%			3%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor		0.97			0.97			1.00				
Frt					0.935			0.999				
Flt Protected		0.975						0.999				
Satd, Flow (prot)	0	1493	0	0	1541	0	0	3073	0	0	0	0
Flt Permitted		0.839						0.999				
Satd, Flow (perm)	0	1248	0	0	1541	0	0	3072	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd, Flow (RTOR)					24			2				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		268			710			308			318	
Travel Time (s)		61			16.1			7 0			72	
Confl Peds (#/hr)	25	0.1				25	17		4			
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Parking (#/hr)	0.00	0.00	0.00	0.00	0.00	0.00	20	0.00	20	0.00	0.00	0.00
Adi Flow (vph)	30	29	0	0	26	24	24	1189	7	0	0	0
Shared Lane Traffic (%)	00	20	Ū	Ū	20	21	21	1100		v	v	Ŭ
Lane Group Flow (vph)	0	59	0	0	50	0	0	1220	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)	Lon	0	rugitt	Lon	0	rtigitt	Lon	0	rugitt	Lon	0	rugin
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		10			10			10			10	
Headway Eactor	1 12	1 28	1 12	1 17	1 12	1 17	1 14	1 19	1 19	1 17	1 17	1 17
Turning Speed (mph)	15	1.20	9	15	1.12	9	15	1.10	9	15	1.17	9
Number of Detectors	1	2	0	10	2	0	1	2	0	10		5
Detector Template	Left	Thru			Thru		Left	Thru				
Leading Detector (ft)	20	100			100		20	100				
Trailing Detector (ft)	0	0			0		0	0				
Detector 1 Position(ft)	0	0			0		0	0				
Detector 1 Size(ft)	20	6			6		20	6				
Detector 1 Type	CI+Ex	CI+Ex			CI+Ex		CI+Ex	CI+Ex				
Detector 1 Channel		OILX						OILX				
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0				
Detector 2 Position(ft)	0.0	0.0 Q/			0.0 Q/		0.0	0.0 Q/				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type												
Detector 2 Channel												
Detector 2 Extend (a)		0.0			0.0			0.0				
	Derm	0.0			0.0		Derm	0.0				
rum rype	Perm	NA			NA		Perm	NA				

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 16: N East St & E Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases		4			8			2				
Permitted Phases	4						2					
Detector Phase	4	4			8		2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0		5.0	5.0				
Minimum Split (s)	15.2	15.2			15.2		47.2	47.2				
Total Split (s)	19.0	19.0			19.0		61.0	61.0				
Total Split (%)	23.8%	23.8%			23.8%		76.3%	76.3%				
Maximum Green (s)	13.8	13.8			13.8		55.8	55.8				
Yellow Time (s)	3.2	3.2			3.2		3.2	3.2				
All-Red Time (s)	2.0	2.0			2.0		2.0	2.0				
Lost Time Adjust (s)		0.0			0.0			0.0				
Total Lost Time (s)		5.2			5.2			5.2				
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0				
Recall Mode	Max	Max			None		C-Max	C-Max				
Walk Time (s)	1.0	1.0			1.0		31.0	31.0				
Flash Dont Walk (s)	9.0	9.0			9.0		11.0	11.0				
Pedestrian Calls (#/hr)	0	0			0		0	0				
Act Effct Green (s)		13.8			13.8			55.8				
Actuated g/C Ratio		0.17			0.17			0.70				
v/c Ratio		0.27			0.18			0.57				
Control Delay		32.7			19.4			2.7				
Queue Delay		0.0			0.0			0.0				
Total Delay		32.7			19.4			2.7				
LOS		С			В			А				
Approach Delay		32.7			19.4			2.7				
Approach LOS		С			В			А				
90th %ile Green (s)	13.8	13.8			13.8		55.8	55.8				
90th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
70th %ile Green (s)	13.8	13.8			13.8		55.8	55.8				
70th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
50th %ile Green (s)	13.8	13.8			13.8		55.8	55.8				
50th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
30th %ile Green (s)	13.8	13.8			13.8		55.8	55.8				
30th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
10th %ile Green (s)	13.8	13.8			13.8		55.8	55.8				
10th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
Stops (vph)		50			26			127				
Fuel Used(gal)		1			1			4				
CO Emissions (g/hr)		54			42			290				
NOx Emissions (g/hr)		11			8			56				
VOC Emissions (g/hr)		13			10			67				
Dilemma Vehicles (#)		0			0			0				
Queue Length 50th (ft)		26			11			33				
Queue Length 95th (ft)		61			41			50				
Internal Link Dist (ft)		188			630			228			238	
Turn Bay Length (ft)												

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 16: N East St & E Jefferson St

02/22/2024	
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Lane Group	FRI	FBT	FBR	• WRI	WRT	WBR	NRI	NBT	NBR	SBI	SBT	SBR
Base Capacity (vph)	LDL	215	LDIX	WDL	285	WBI	NDL	2143	NBR	ODL	001	
Starvation Cap Reductn		0			0			76				
Spillback Cap Reductn		0			0			0				
Storage Cap Reductn		0			0			0				
Reduced v/c Ratio		0.27			0.18			0.59				
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 55 (69%), Reference	ed to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 65												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.57												
Intersection Signal Delay: 4	4.7			In	tersectior	n LOS: A						
Intersection Capacity Utiliz	ation 54.8%			IC	U Level o	of Service	A					
Analysis Period (min) 15												

Splits and Phases: 16: N East St & E Jefferson St

Ø2 (R)	 Ø4
61 s	19 s
	← Ø8
	19 s

Lanes, Volumes, Timings 19: N Madison St & W Washington St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1.		٦	†					٦	† Ъ	
Traffic Volume (vph)	0	139	9	66	252	0	0	0	0	64	1117	45
Future Volume (vph)	0	139	9	66	252	0	0	0	0	64	1117	45
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	13	12	12	13	12	12	12	12	12	12	12
Grade (%)		3%			-3%			2%			-2%	
Storage Length (ft)	0		0	65		0	0		0	150		0
Storage Lanes	0		0	1		0	0		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95
Ped Bike Factor		1.00		0.99						1.00	1.00	
Frt		0.992									0.994	
Flt Protected				0.950						0.950		
Satd. Flow (prot)	0	1522	0	1617	1758	0	0	0	0	1367	3195	0
Flt Permitted				0.641						0.950		
Satd. Flow (perm)	0	1522	0	1084	1758	0	0	0	0	1364	3195	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		4									10	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		690			253			324			310	
Travel Time (s)		15.7			5.8			7.4			7.0	
Confl. Peds. (#/hr)			4	4						2		3
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Parking (#/hr)		0								10		10
Adj. Flow (vph)	0	146	9	69	265	0	0	0	0	67	1176	47
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	155	0	69	265	0	0	0	0	67	1223	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
I wo way Left I urn Lane					Yes							
Headway Factor	1.17	1.28	1.1/	1.12	1.08	1.12	1.16	1.16	1.16	1.38	1.13	1.13
Turning Speed (mph)	15	•	9	15	•	9	15		9	15	•	9
Number of Detectors		2		1	2					1	2	
Detector Template		l hru		Left	l hru					Left	I hru	_
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		CI+Ex		CI+EX	CI+EX					CI+Ex	CI+Ex	
Detector 1 Channel		0.0		0.0	0.0					0.0	0.0	
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(tt)		94			94						94	
Detector 2 Size(tt)		6			6						6	
Detector 2 Type		CI+Ex			CI+Ex						CI+EX	

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 19: N Madison St & W Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		17.4		27.4	27.4					35.6	35.6	
Total Split (s)		23.0		23.0	23.0					57.0	57.0	
Total Split (%)		28.8%		28.8%	28.8%					71.3%	71.3%	
Maximum Green (s)		17.6		17.6	17.6					51.4	51.4	
Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)		2.2		2.2	2.2					2.4	2.4	
Lost Time Adjust (s)		0.0		0.0	0.0					0.0	0.0	
Total Lost Time (s)		5.4		5.4	5.4					5.6	5.6	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		Max	Max					C-Max	C-Max	
Walk Time (s)		1.0		11.0	11.0					19.0	19.0	
Flash Dont Walk (s)		11.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		17.6		17.6	17.6					51.4	51.4	
Actuated g/C Ratio		0.22		0.22	0.22					0.64	0.64	
v/c Ratio		0.46		0.29	0.69					0.08	0.59	
Control Delay		31.5		14.1	25.1					3.4	4.4	
Queue Delay		0.0		0.0	16.6					0.0	0.2	
Total Delay		31.5		14.1	41.7					3.4	4.6	
LOS		С		В	D					А	А	
Approach Delay		31.5			36.0						4.6	
Approach LOS		С			D						А	
90th %ile Green (s)		17.6		17.6	17.6					51.4	51.4	
90th %ile Term Code		Max		Ped	Ped					Coord	Coord	
70th %ile Green (s)		17.6		17.6	17.6					51.4	51.4	
70th %ile Term Code		Hold		Ped	Ped					Coord	Coord	
50th %ile Green (s)		17.6		17.6	17.6					51.4	51.4	
50th %ile Term Code		Hold		Ped	Ped					Coord	Coord	
30th %ile Green (s)		17.6		17.6	17.6					51.4	51.4	
30th %ile Term Code		Hold		Ped	Ped					Coord	Coord	
10th %ile Green (s)		17.6		17.6	17.6					51.4	51.4	
10th %ile Term Code		Hold		Ped	Ped					Coord	Coord	
Stops (vph)		121		55	246					10	169	
Fuel Used(gal)		2		1	3					0	5	
CO Emissions (g/hr)		168		44	220					18	335	
NOx Emissions (g/hr)		33		8	43					3	65	
VOC Emissions (g/hr)		39		10	51					4	78	
Dilemma Vehicles (#)		0		0	0					0	0	
Queue Length 50th (ft)		66		28	145					6	52	

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 19: N Madison St & W Washington St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)		123		44	#229					m9	62	
Internal Link Dist (ft)		610			173			244			230	
Turn Bay Length (ft)				65						150		
Base Capacity (vph)		337		238	386					876	2056	
Starvation Cap Reductn		0		0	107					0	203	
Spillback Cap Reductn		0		0	0					57	0	
Storage Cap Reductn		0		0	0					0	0	
Reduced v/c Ratio		0.46		0.29	0.95					0.08	0.66	
Intersection Summary												
Area Type: C	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 0 (0%), Referenced to	o phase 2:	and 6:SB	TL, Start	of Green								
Natural Cycle: 65												
Control Type: Actuated-Coor	dinated											
Maximum v/c Ratio: 0.69												
Intersection Signal Delay: 12	.8			In	tersectior	n LOS: B						
Intersection Capacity Utilizati	ion 62.7%			IC	U Level o	of Service	В					
Analysis Period (min) 15												
# 95th percentile volume ex	xceeds cap	acity, qu	eue may	be longer								
Queue shown is maximur	n after two	cycles.										
m Volume for 95th percent	ile queue is	s metered	l by upstr	eam sign	al.							
Splits and Phases: 19: N M	/ladison St	& W Was	shington S	St								

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Lanes, Volumes, Timings 20: N East St & E Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٢	1			1	1	7	† 1 ₂				
Traffic Volume (vph)	38	246	0	0	315	96	35	1001	76	0	0	0
Future Volume (vph)	38	246	0	0	315	96	35	1001	76	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	11	10	11	11	11	12	12	12
Grade (%)		-3%			2%			-3%			1%	
Storage Length (ft)	65		0	0		80	80		0	0		0
Storage Lanes	1		0	0		1	1		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	1.00					0.98	1.00	1.00				
Frt						0.850		0.989				
Flt Protected	0.950						0.950					
Satd. Flow (prot)	1617	1702	0	0	1604	1317	1250	3085	0	0	0	0
Flt Permitted	0.308						0.950					
Satd. Flow (perm)	523	1702	0	0	1604	1292	1245	3085	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)						77		18				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		262			724			322			308	
Travel Time (s)		6.0			16.5			7.3			7.0	
Confl. Peds. (#/hr)	4					4	4		9			
Confl. Bikes (#/hr)						1						
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Parking (#/hr)							20		20			
Adj. Flow (vph)	41	265	0	0	339	103	38	1076	82	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	41	265	0	0	339	103	38	1158	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			11			11	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
I wo way Left I urn Lane	4.40	Yes	4.40	4.40	Yes	4.00						
Headway Factor	1.12	1.12	1.12	1.16	1.21	1.26	1.54	1.17	1.17	1.15	1.15	1.15
Turning Speed (mph)	15	_	9	15	_	9	15	_	y	15		g
Number of Detectors	1	2			2	1	1	2				
Detector Template	Left	l hru			I hru	Right	Left	I hru				
Leading Detector (ft)	20	100			100	20	20	100				
Trailing Detector (ft)	0	0			0	0	0	0				
Detector 1 Position(ft)	0	0			0	0	0	0				
Detector 1 Size(ft)	20	6			6	20	20	6				
Detector 1 Type	CI+EX	CI+EX			CI+EX	CI+EX	CI+EX	CI+EX				
Detector 1 Channel	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Extend (S)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Delay (S)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 20: N East St & E Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA	Perm	Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4					8	2					
Detector Phase	4	4			8	8	2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0	5.0	2.0	2.0				
Minimum Split (s)	31.4	31.4			21.4	21.4	47.0	47.0				
Total Split (s)	26.0	26.0			26.0	26.0	54.0	54.0				
Total Split (%)	32.5%	32.5%			32.5%	32.5%	67.5%	67.5%				
Maximum Green (s)	20.6	20.6			20.6	20.6	48.0	48.0				
Yellow Time (s)	3.2	3.2			3.2	3.2	3.2	3.2				
All-Red Time (s)	2.2	2.2			2.2	2.2	2.8	2.8				
Lost Time Adjust (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Total Lost Time (s)	5.4	5.4			5.4	5.4	6.0	6.0				
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0	3.0	3.0	3.0				
Recall Mode	None	None			None	None	C-Max	C-Max				
Walk Time (s)	15.0	15.0			5.0	5.0	30.0	30.0				
Flash Dont Walk (s)	11.0	11.0			11.0	11.0	11.0	11.0				
Pedestrian Calls (#/hr)	0	0			0	0	0	0				
Act Effct Green (s)	19.4	19.4			19.4	19.4	49.2	49.2				
Actuated g/C Ratio	0.24	0.24			0.24	0.24	0.62	0.62				
v/c Ratio	0.32	0.64			0.87	0.28	0.05	0.61				
Control Delay	15.9	15.6			52.8	11.1	4.0	5.4				
Queue Delay	0.0	0.7			0.0	0.0	0.0	0.2				
Total Delay	15.9	16.3			52.8	11.1	4.0	5.6				
LOS	В	В			D	В	А	А				
Approach Delay		16.3			43.1			5.6				
Approach LOS		В			D			А				
90th %ile Green (s)	20.6	20.6			20.6	20.6	48.0	48.0				
90th %ile Term Code	Max	Max			Max	Max	Coord	Coord				
70th %ile Green (s)	20.6	20.6			20.6	20.6	48.0	48.0				
70th %ile Term Code	Hold	Hold			Max	Max	Coord	Coord				
50th %ile Green (s)	20.6	20.6			20.6	20.6	48.0	48.0				
50th %ile Term Code	Hold	Hold			Max	Max	Coord	Coord				
30th %ile Green (s)	19.9	19.9			19.9	19.9	48.7	48.7				
30th %ile Term Code	Hold	Hold			Gap	Gap	Coord	Coord				
10th %ile Green (s)	15.5	15.5			15.5	15.5	53.1	53.1				
10th %ile Term Code	Hold	Hold			Gap	Gap	Coord	Coord				
Stops (vph)	14	95			277	29	6	218				
Fuel Used(gal)	0	2			7	1	0	5				
CO Emissions (g/hr)	19	127			468	64	10	356				
NOx Emissions (g/hr)	4	25			91	13	2	69				
VOC Emissions (g/hr)	5	29			108	15	2	83				
Dilemma Vehicles (#)	0	0			0	0	0	0				

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 20: N East St & E Washington St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)	2	14			160	10	4	62				
Queue Length 95th (ft)	14	80			#297	48	m7	77				
Internal Link Dist (ft)		182			644			242			228	
Turn Bay Length (ft)	65					80	80					
Base Capacity (vph)	134	438			413	389	765	1902				
Starvation Cap Reductn	0	36			0	0	0	174				
Spillback Cap Reductn	0	0			0	0	0	0				
Storage Cap Reductn	0	0			0	0	0	0				
Reduced v/c Ratio	0.31	0.66			0.82	0.26	0.05	0.67				
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 60 (75%), Reference	d to phase	2:NBTL a	ind 6:, Sta	art of Gre	en							
Natural Cycle: 80												
Control Type: Actuated-Coo	rdinated											
Maximum v/c Ratio: 0.87												
Intersection Signal Delay: 15	5.8			In	tersectior	n LOS: B						
Intersection Capacity Utiliza	tion 70.8%			IC	U Level o	of Service	С					
Analysis Period (min) 15												
# 95th percentile volume e	exceeds cap	bacity, qu	eue may	be longer								
Queue shown is maximu	m after two	cycles.										
m Volume for 95th percen	tile queue i	s metered	l by upstr	eam sign	al.							
Splits and Phases: 20: N	East St & E	Washing	ton St									
							1	- A	_			253

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54 s	26 s
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	26 s

Lanes, Volumes, Timings 23: S Center St/N Madison St & W Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,		5	+						đ î þ	
Traffic Volume (vph)	0	39	15	58	71	0	0	0	0	25	1134	30
Future Volume (vph)	0	39	15	58	71	0	0	0	0	25	1134	30
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	13	12	12	12	12	12	12	12	12
Grade (%)		3%			-3%			1%			-1%	
Storage Length (ft)	0		0	60		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	0		0
Taper Length (ft)	25			0			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		0.99		0.98							1.00	
Frt		0.963									0.996	
Flt Protected				0.950							0.999	
Satd. Flow (prot)	0	1419	0	1670	1531	0	0	0	0	0	3183	0
Flt Permitted				0.721							0.999	
Satd. Flow (perm)	0	1419	0	1245	1531	0	0	0	0	0	3182	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		15									7	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		684			247			660			324	
Travel Time (s)		15.5			5.6			15.0			7.4	
Confl. Peds. (#/hr)			8	8						13		4
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Parking (#/hr)		0			0					20		20
Adj. Flow (vph)	0	40	15	60	73	0	0	0	0	26	1169	31
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	55	0	60	73	0	0	0	0	0	1226	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		13			13			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.17	1.33	1.17	1.08	1.28	1.12	1.15	1.15	1.15	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		Cl+Ex	CI+Ex					Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		Cl+Ex			Cl+Ex						Cl+Ex	

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Lanes, Volumes, Timings 23: S Center St/N Madison St & W Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8					-	6	
Permitted Phases				8						6		
Detector Phase		4		8	8					6	6	
Switch Phase												
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		15.5		25.5	25.5					37.7	37.7	
Total Split (s)		22.0		22.0	22.0					58.0	58.0	
Total Split (%)		27.5%		27.5%	27.5%					72.5%	72.5%	
Maximum Green (s)		16.5		16.5	16.5					52.3	52.3	
Yellow Time (s)		3.2		3.2	3.2					3.2	3.2	
All-Red Time (s)		2.3		2.3	2.3					2.5	2.5	
Lost Time Adjust (s)		0.0		0.0	0.0						0.0	
Total Lost Time (s)		5.5		5.5	5.5						5.7	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		None	None					C-Max	C-Max	
Walk Time (s)		1.0		9.0	9.0					21.0	21.0	
Flash Dont Walk (s)		9.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		9.3		9.3	9.3						62.8	
Actuated g/C Ratio		0.12		0.12	0.12						0.78	
v/c Ratio		0.31		0.41	0.41						0.49	
Control Delay		28.8		40.4	38.7						2.3	
Queue Delay		0.0		0.0	0.0						0.0	
Total Delay		28.8		40.4	38.7						2.3	
LOS		С		D	D						А	
Approach Delay		28.8			39.5						2.3	
Approach LOS		С			D						А	
90th %ile Green (s)		13.4		13.4	13.4					55.4	55.4	
90th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
70th %ile Green (s)		10.9		10.9	10.9					57.9	57.9	
70th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
50th %ile Green (s)		9.2		9.2	9.2					59.6	59.6	
50th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
30th %ile Green (s)		7.6		7.6	7.6					61.2	61.2	
30th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
10th %ile Green (s)		0.0		0.0	0.0					74.3	74.3	
10th %ile Term Code		Skip		Skip	Skip					Coord	Coord	
Stops (vph)		38		53	63						132	
Fuel Used(gal)		1		1	1						4	
CO Emissions (g/hr)		56		62	73						300	
NOx Emissions (g/hr)		11		12	14						58	
VOC Emissions (g/hr)		13		14	17						69	
Dilemma Vehicles (#)		0		0	0						0	
Queue Length 50th (ft)		19		28	35						46	

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 23: S Center St/N Madison St & W Front St

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Lane Group	FRI	FRT	FRR	• WRI	WRT	WRR	NRI	NRT	NRR	SBI	SBT	SBR
Oueue Length 95th (ft)		49	LDIX	62	70	WBI	NDL		NDIX	ODL	53	
Internal Link Dist (ft)		604		02	167			580			244	
Turn Bay Length (ft)		•••		60								
Base Capacity (vph)		304		256	315						2500	
Starvation Cap Reductn		0		0	0						79	
Spillback Cap Reductn		0		0	0						0	
Storage Cap Reductn		0		0	0						0	
Reduced v/c Ratio		0.18		0.23	0.23						0.51	
Intersection Summary												
Area Type: Cl	BD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 0 (0%), Referenced to	phase 2:	and 6:SB	TL, Start	of Green								
Natural Cycle: 65												
Control Type: Actuated-Coord	linated											
Maximum v/c Ratio: 0.49												
Intersection Signal Delay: 6.8				In	tersectior	LOS: A						
Intersection Capacity Utilization	on 56.3%			IC	U Level o	of Service	В					
Analysis Period (min) 15												

Splits and Phases: 23: S Center St/N Madison St & W Front St

	→ Ø4
	22 s
₩ Ø6 (R)	₩ Ø8
58 s	22 s

02/22/2024

Lanes, Volumes, Timings 24: S East St/N East St & E Front St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲	•			1	1		đ þ				
Traffic Volume (vph)	90	127	0	0	54	25	48	1005	32	0	0	0
Future Volume (vph)	90	127	0	0	54	25	48	1005	32	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	16	12	12	10	10	13	11	11	12	12	12
Grade (%)		-1%			0%			-1%			2%	
Storage Length (ft)	90		0	0		100	0		0	0		0
Storage Lanes	1		0	0		1	0		0	0		0
Taper Length (ft)	0			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor	0.97					0.95		1.00				
Frt						0.850		0.996				
Flt Protected	0.950							0.998				
Satd. Flow (prot)	1547	1910	0	0	1565	1330	0	3074	0	0	0	0
Flt Permitted	0.717							0.998				
Satd. Flow (perm)	1129	1910	0	0	1565	1267	0	3071	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)						35		8				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		256			710			633			322	
Travel Time (s)		5.8			16.1			14.4			7.3	
Confl. Peds. (#/hr)	18					18	28		1			
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	103	146	0	0	62	29	55	1155	37	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	103	146	0	0	62	29	0	1247	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		11			11			11			11	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.19	0.97	1.14	1.14	1.25	1.25	1.09	1.19	1.19	1.16	1.16	1.16
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2			2	1	1	2				
Detector Template	Left	Thru			Thru	Right	Left	Thru				
Leading Detector (ft)	20	100			100	20	20	100				
Trailing Detector (ft)	0	0			0	0	0	0				
Detector 1 Position(ft)	0	0			0	0	0	0				
Detector 1 Size(ft)	20	6			6	20	20	6				
Detector 1 Type	CI+Ex	Cl+Ex			CI+Ex	Cl+Ex	CI+Ex	CI+Ex				
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0	0.0	0.0	0.0				
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex				
Detector 2 Channel												

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 24: S East St/N East St & E Front St

02/22/2024

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Lane Group E	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type Po	erm	NA			NA	Perm	Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4					8	2					
Detector Phase	4	4			8	8	2	2				
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0	5.0	5.0	5.0				
Minimum Split (s) 2	27.2	27.2			17.2	17.2	50.6	50.6				
Total Split (s) 2	22.0	22.0			22.0	22.0	58.0	58.0				
Total Split (%) 27	.5% 2	7.5%			27.5%	27.5%	72.5%	72.5%				
Maximum Green (s) 1	16.8	16.8			16.8	16.8	52.4	52.4				
Yellow Time (s)	3.2	3.2			3.2	3.2	3.2	3.2				
All-Red Time (s)	2.0	2.0			2.0	2.0	2.4	2.4				
Lost Time Adjust (s)	0.0	0.0			0.0	0.0		0.0				
Total Lost Time (s)	5.2	5.2			5.2	5.2		5.6				
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0	3.0	3.0	3.0				
Recall Mode N	one	None			Max	Max	C-Max	C-Max				
Walk Time (s) 1	11.0	11.0			1.0	1.0	34.0	34.0				
Flash Dont Walk (s) 1	11.0	11.0			11.0	11.0	11.0	11.0				
Pedestrian Calls (#/hr)	0	0			0	0	0	0				
Act Effct Green (s)	16.8	16.8			16.8	16.8		52.4				
Actuated g/C Ratio 0).21	0.21			0.21	0.21		0.66				
v/c Ratio).43	0.36			0.19	0.10		0.62				
Control Delay 3	34.2	30.1			27.9	9.1		5.5				
Queue Delay	0.0	0.0			0.0	0.0		0.0				
Total Delay	34.2	30.1			27.9	9.1		5.5				
LOS	С	С			С	А		А				
Approach Delay		31.8			21.9			5.5				
Approach LOS		С			С			А				
90th %ile Green (s) 1	16.8	16.8			16.8	16.8	52.4	52.4				
90th %ile Term Code	Max	Max			MaxR	MaxR	Coord	Coord				
70th %ile Green (s) 1	16.8	16.8			16.8	16.8	52.4	52.4				
70th %ile Term Code	lold	Hold			MaxR	MaxR	Coord	Coord				
50th %ile Green (s) 1	16.8	16.8			16.8	16.8	52.4	52.4				
50th %ile Term Code	lold	Hold			MaxR	MaxR	Coord	Coord				
30th %ile Green (s) 1	16.8	16.8			16.8	16.8	52.4	52.4				
30th %ile Term Code	lold	Hold			MaxR	MaxR	Coord	Coord				
10th %ile Green (s) 1	16.8	16.8			16.8	16.8	52.4	52.4				
10th %ile Term Code	lold	Hold			MaxR	MaxR	Coord	Coord				
Stops (vph)	77	105			44	7		267				
Fuel Used(gal)	1	2			1	0		8				
CO Emissions (a/hr)	86	113			59	16		562				
NOx Emissions (a/hr)	17	22			12	3		109				
VOC Emissions (a/hr)	20	26			14	4		130				
Dilemma Vehicles (#)	0	0			0	0		0				
Queue Length 50th (ft)	45	63			26	0		72				
Queue Length 95th (ft)	89	111			56	17		85				

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 24: S East St/N East St & E Front St

02/22/2024	ŀ
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		176			630			553			242	
Turn Bay Length (ft)	90					100						
Base Capacity (vph)	237	401			328	293		2014				
Starvation Cap Reductn	0	0			0	0		0				
Spillback Cap Reductn	0	0			0	0		0				
Storage Cap Reductn	0	0			0	0		0				
Reduced v/c Ratio	0.43	0.36			0.19	0.10		0.62				
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 58 (73%), Reference	ed to phase	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 80												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.62												
Intersection Signal Delay: 7	10.6			In	tersectior	n LOS: B						
Intersection Capacity Utiliz	ation 66.4%			IC	U Level o	of Service	С					
Analysis Period (min) 15												
Splits and Phases: 24: S	East St/N E	ast St & E	E Front St	t								

Ø2 (R)	3 224	<u>⊿</u> _{Ø4}	- 25
58 s		22 s	
		4 [⊕] Ø8	
		22 s	

Lanes, Volumes, Timings 27: N Center St & W Market St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ţ,			र्स						đ þ	
Traffic Volume (vph)	0	148	31	37	119	0	0	0	0	37	134	17
Future Volume (vph)	0	148	31	37	119	0	0	0	0	37	134	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt		0.977									0.986	
Flt Protected					0.988						0.990	
Satd. Flow (prot)	0	1606	0	0	1462	0	0	0	0	0	2897	0
Flt Permitted					0.988						0.990	
Satd. Flow (perm)	0	1606	0	0	1462	0	0	0	0	0	2897	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		247			266			264			413	
Travel Time (s)		5.6			6.0			6.0			9.4	
Confl. Peds. (#/hr)			30	30						6		5
Confl. Bikes (#/hr)			1									
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)					0						0	
Adj. Flow (vph)	0	164	34	41	132	0	0	0	0	41	149	19
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	198	0	0	173	0	0	0	0	0	209	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.14	1.14	1.14	1.30	1.14	1.14	1.14	1.14	1.14	1.22	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: 0	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 38.9%			IC	CU Level	of Service	А					
Analysis Period (min) 15												

Lanes, Volumes, Timings 29: N Center St & W Jefferson St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		Þ			र्स						4 P	
Traffic Volume (vph)	0	39	4	10	44	0	0	0	0	39	106	11
Future Volume (vph)	0	39	4	10	44	0	0	0	0	39	106	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt		0.986									0.989	
Flt Protected					0.991						0.988	
Satd. Flow (prot)	0	1459	0	0	1466	0	0	0	0	0	2900	0
Flt Permitted					0.991						0.988	
Satd. Flow (perm)	0	1459	0	0	1466	0	0	0	0	0	2900	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		253			255			306			278	
Travel Time (s)		5.8			5.8			7.0			6.3	
Confl. Peds. (#/hr)			5	5						20		8
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)		0			0						0	
Adj. Flow (vph)	0	45	5	12	51	0	0	0	0	45	123	13
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	50	0	0	63	0	0	0	0	0	181	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.14	1.14	1.14	1.22	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: (CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 23.4%			IC	CU Level of	of Service	А					

Analysis Period (min) 15

Lanes, Volumes, Timings 30: N Center St & W Washington St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ĥ		5	+						đ î þ	
Traffic Volume (vph)	0	213	6	9	316	0	0	0	0	68	47	8
Future Volume (vph)	0	213	6	9	316	0	0	0	0	68	47	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	65		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95
Ped Bike Factor		1.00		0.99							0.98	
Frt		0.996									0.991	
Flt Protected				0.950							0.973	
Satd. Flow (prot)	0	1473	0	1562	1480	0	0	0	0	0	2853	0
Flt Permitted				0.542							0.973	
Satd. Flow (perm)	0	1473	0	883	1480	0	0	0	0	0	2804	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		3									8	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		253			254			326			306	
Travel Time (s)		5.8			5.8			7.4			7.0	
Confl. Peds. (#/hr)			10	10						14		12
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)		0			0						0	
Adj. Flow (vph)	0	222	6	9	329	0	0	0	0	71	49	8
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	228	0	9	329	0	0	0	0	0	128	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.14	1.14	1.14	1.22	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors		2		1	2					1	2	
Detector Template		Thru		Left	Thru					Left	Thru	
Leading Detector (ft)		100		20	100					20	100	
Trailing Detector (ft)		0		0	0					0	0	
Detector 1 Position(ft)		0		0	0					0	0	
Detector 1 Size(ft)		6		20	6					20	6	
Detector 1 Type		Cl+Ex		Cl+Ex	CI+Ex					CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Queue (s)		0.0		0.0	0.0					0.0	0.0	
Detector 1 Delay (s)		0.0		0.0	0.0					0.0	0.0	
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		Cl+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 30: N Center St & W Washington St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA					Perm	NA	
Protected Phases		4			8						6	
Permitted Phases		•		8	•					6	, T	
Detector Phase		4		8	8					6	6	
Switch Phase		-		-	-					-	-	
Minimum Initial (s)		5.0		5.0	5.0					5.0	5.0	
Minimum Split (s)		23.0		23.0	23.0					23.0	23.0	
Total Split (s)		52.0		52.0	52.0					28.0	28.0	
Total Split (%)		65.0%		65.0%	65.0%					35.0%	35.0%	
Maximum Green (s)		47.0		47.0	47.0					23.0	23.0	
Yellow Time (s)		4.0		4.0	4.0					4.0	4.0	
All-Red Time (s)		1.0		1.0	1.0					1.0	1.0	
Lost Time Adjust (s)		0.0		0.0	0.0						0.0	
Total Lost Time (s)		5.0		5.0	5.0						5.0	
Lead/Lag				0.0	0.0						0.0	
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0		3.0	3.0					3.0	3.0	
Recall Mode		None		None	None					C-Max	C-Max	
Walk Time (s)		7.0		7.0	7.0					7 0	7.0	
Flash Dont Walk (s)		11.0		11.0	11.0					11.0	11.0	
Pedestrian Calls (#/hr)		0		0	0					0	0	
Act Effct Green (s)		24.8		24.8	24.8					Ű	45.2	
Actuated g/C Ratio		0.31		0.31	0.31						0.56	
v/c Ratio		0.50		0.03	0.72						0.08	
Control Delay		37.7		9.7	21.0						9.6	
Queue Delay		0.2		0.0	0.1						0.0	
Total Delay		37.9		9.7	21.1						9.6	
LOS		D		A	C						A	
Approach Delay		37.9			20.8						9.6	
Approach LOS		D			С						A	
90th %ile Green (s)		35.1		35.1	35.1					34.9	34.9	
90th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
70th %ile Green (s)		28.7		28.7	28.7					41.3	41.3	
70th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
50th %ile Green (s)		24.6		24.6	24.6					45.4	45.4	
50th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
30th %ile Green (s)		20.6		20.6	20.6					49.4	49.4	
30th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
10th %ile Green (s)		15.0		15.0	15.0					55.0	55.0	
10th %ile Term Code		Hold		Gap	Gap					Coord	Coord	
Stops (vph)		194		2	117						55	
Fuel Used(gal)		3		0	3						1	
CO Emissions (a/hr)		223		3	183						59	
NOx Emissions (a/hr)		43		1	36						11	
VOC Emissions (g/hr)		52		1	43						14	
Dilemma Vehicles (#)		0		0	0						0	
Queue Length 50th (ft)		115		1	46						13	
Queue Length 95th (ft)		156		m3	54						34	

2044 PM Downtown Bloomington 7:12 am 02/27/2023 2 lane option BSE

Lanes, Volumes, Timings 30: N Center St & W Washington St

02/22/2024	ŀ
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	FDI									CDI	CDT	000
	EDL		EDK	VVDL		VVDR	INDL		NDR	SDL		SDK
Internal Link Dist (ft)		1/3			1/4			246			226	
Turn Bay Length (ft)				65								
Base Capacity (vph)		866		518	869						1587	
Starvation Cap Reductn		205		0	38						0	
Spillback Cap Reductn		0		0	54						0	
Storage Cap Reductn		0		0	0						0	
Reduced v/c Ratio		0.34		0.02	0.40						0.08	
Intersection Summary												
Area Type: Cl	BD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 2 (3%), Referenced to	phase 2:	and 6:SB	TL, Start	of Green								
Natural Cycle: 50												
Control Type: Actuated-Coord	linated											
Maximum v/c Ratio: 0.72												
Intersection Signal Delay: 24.3	3			In	tersectior	n LOS: C						
Intersection Capacity Utilization	on 53.3%			IC	U Level o	of Service	A					
Analysis Period (min) 15												
m Volume for 95th percentil	e queue is	s meterec	by upstr	eam sign	al.							

Splits and Phases: 30: N Center St & W Washington St

	→ Ø4
	52 s
Ø6 (R)	₩ Ø8
28 s	52 s

Lanes, Volumes, Timings 31: W Front St & N Center St

02/22/202	24
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ħ		7	1			\$		7	ħ	
Traffic Volume (vph)	0	59	6	6	122	0	0	0	1	51	3	10
Future Volume (vph)	0	59	6	6	122	0	0	0	1	51	3	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	8	12	12	12	12
Storage Length (ft)	0		0	70		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.988						0.865			0.887	
Flt Protected				0.950						0.950		
Satd. Flow (prot)	0	1395	0	1490	1412	0	0	1176	0	1341	1252	0
Flt Permitted				0.950						0.950		
Satd. Flow (perm)	0	1395	0	1490	1412	0	0	1176	0	1341	1252	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		247			262			221			326	
Travel Time (s)		5.6			6.0			5.0			7.4	
Confl. Peds. (#/hr)			48	48			25		13	13		25
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	9%	9%	9%	9%	9%	9%	9%	9%	9%	9%	9%	9%
Parking (#/hr)		0			0					0	0	
Adj. Flow (vph)	0	72	7	7	149	0	0	0	1	62	4	12
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	79	0	7	149	0	0	1	0	62	16	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		13			13			0			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane					Yes							
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.37	1.14	1.30	1.30	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type: C	BD											
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 30.0%			IC	U Level o	of Service	А					
Analysis Period (min) 15												

Lanes, Volumes, Timings 34: N Main St & W Market St/E Market St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		÷.			Į.			đ î de				
Traffic Volume (vph)	36	144	0	0	127	9	48	64	28	0	0	0
Future Volume (vph)	36	144	0	0	127	9	48	64	28	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.991			0.970				
Flt Protected		0.990						0.983				
Satd. Flow (prot)	0	1479	0	0	1481	0	0	2857	0	0	0	0
Flt Permitted		0.990						0.983				
Satd. Flow (perm)	0	1479	0	0	1481	0	0	2857	0	0	0	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		266			263			249			404	
Travel Time (s)		6.0			6.0			5.7			9.2	
Confl. Peds. (#/hr)	19					19	20		18			
Confl. Bikes (#/hr)						1			1			
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Parking (#/hr)		0			0			0				
Adj. Flow (vph)	42	167	0	0	148	10	56	74	33	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	209	0	0	158	0	0	163	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.22	1.14	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type:	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 39.9%			IC	CU Level o	of Service	А					
Analysis Period (min) 15												
Lanes, Volumes, Timings 36: N Main St & W Jefferson St/E Jefferson St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ŧ			ĥ			đ î ja				
Traffic Volume (vph)	37	39	0	0	31	17	16	83	9	0	0	0
Future Volume (vph)	37	39	0	0	31	17	16	83	9	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.953			0.987				
Flt Protected		0.976						0.993				
Satd. Flow (prot)	0	1404	0	0	1371	0	0	2827	0	0	0	0
Flt Permitted		0.976						0.993				
Satd. Flow (perm)	0	1404	0	0	1371	0	0	2827	0	0	0	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		255			268			306			272	
Travel Time (s)		5.8			6.1			7.0			6.2	
Confl. Peds. (#/hr)	11					11	3		22			
Confl. Bikes (#/hr)									2			
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83
Heavy Vehicles (%)	7%	7%	7%	7%	7%	7%	7%	7%	7%	7%	7%	7%
Parking (#/hr)		0			0			0				
Adj. Flow (vph)	45	47	0	0	37	20	19	100	11	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	92	0	0	57	0	0	130	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.22	1.14	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type:	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 26.5%			IC	CU Level of	of Service	А					
Analysis Period (min) 15												

Lanes, Volumes, Timings	
37: N Main St & W Washington St/E Washington St	t

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	•			ţ,			é.	1			
Traffic Volume (vph)	19	266	0	0	321	41	12	29	17	0	0	0
Future Volume (vph)	19	266	0	0	321	41	12	29	17	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	65		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99				1.00			0.99	0.93			
Frt					0.985				0.850			
Flt Protected	0.950							0.986				
Satd. Flow (prot)	1562	1480	0	0	1454	0	0	1459	1258	0	0	0
Flt Permitted	0.368							0.986				
Satd. Flow (perm)	602	1480	0	0	1454	0	0	1451	1168	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					14				27			
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		254			262			326			306	
Travel Time (s)		5.8			6.0			7.4			7.0	
Confl. Peds. (#/hr)	8					8	9		23			
Confl. Bikes (#/hr)						1			1			
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Heavy Vehicles (%)	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%	4%
Parking (#/hr)		0			0			0	0			
Adj. Flow (vph)	19	271	0	0	328	42	12	30	17	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	19	271	0	0	370	0	0	42	17	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12	Ŭ		12	Ŭ		0	Ŭ		0	Ŭ
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.30	1.30	1.14	1.14	1.14
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2			2		1	2	1			
Detector Template	Left	Thru			Thru		Left	Thru	Right			
Leading Detector (ft)	20	100			100		20	100	20			
Trailing Detector (ft)	0	0			0		0	0	0			
Detector 1 Position(ft)	0	0			0		0	0	0			
Detector 1 Size(ft)	20	6			6		20	6	20			
Detector 1 Type	CI+Ex	CI+Ex			CI+Ex		CI+Ex	CI+Ex	CI+Ex			
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0	0.0			
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			CI+Ex			Cl+Ex				

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Lanes, Volumes, Timings	
37: N Main St & W Washington St/E Washington	St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA		Perm	NA	Perm			
Protected Phases		4			8			2				
Permitted Phases	4						2		2			
Detector Phase	4	4			8		2	2	2			
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0		5.0	5.0	5.0			
Minimum Split (s)	23.0	23.0			23.0		23.0	23.0	23.0			
Total Split (s)	52.0	52.0			52.0		28.0	28.0	28.0			
Total Split (%)	65.0%	65.0%			65.0%		35.0%	35.0%	35.0%			
Maximum Green (s)	47.0	47.0			47.0		23.0	23.0	23.0			
Yellow Time (s)	4.0	4.0			4.0		4.0	4.0	4.0			
All-Red Time (s)	1.0	1.0			1.0		1.0	1.0	1.0			
Lost Time Adjust (s)	0.0	0.0			0.0			0.0	0.0			
Total Lost Time (s)	5.0	5.0			5.0			5.0	5.0			
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0	3.0			
Recall Mode	None	None			None		C-Max	C-Max	C-Max			
Walk Time (s)	7.0	7.0			7.0		7.0	7.0	7.0			
Flash Dont Walk (s)	11.0	11.0			11.0		11.0	11.0	11.0			
Pedestrian Calls (#/hr)	0	0			0		0	0	0			
Act Effct Green (s)	27.1	27.1			27.1			42.9	42.9			
Actuated g/C Ratio	0.34	0.34			0.34			0.54	0.54			
v/c Ratio	0.09	0.54			0.74			0.05	0.03			
Control Delay	9.2	12.9			23.7			12.1	4.2			
Queue Delay	0.0	0.1			0.0			0.0	0.0			
Total Delay	9.2	13.0			23.7			12.1	4.2			
LOS	А	В			С			В	А			
Approach Delay		12.7			23.7			9.8				
Approach LOS		В			С			А				
90th %ile Green (s)	38.1	38.1			38.1		31.9	31.9	31.9			
90th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
70th %ile Green (s)	31.4	31.4			31.4		38.6	38.6	38.6			
70th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
50th %ile Green (s)	26.9	26.9			26.9		43.1	43.1	43.1			
50th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
30th %ile Green (s)	22.5	22.5			22.5		47.5	47.5	47.5			
30th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
10th %ile Green (s)	16.4	16.4			16.4		53.6	53.6	53.6			
10th %ile Term Code	Hold	Hold			Gap		Coord	Coord	Coord			
Stops (vph)	6	76			291			24	4			
Fuel Used(gal)	0	2			4			0	0			
CO Emissions (g/hr)	7	115			286			24	6			
NOx Emissions (g/hr)	1	22			56			5	1			
VOC Emissions (g/hr)	2	27			66			5	1			
Dilemma Vehicles (#)	0	0			0			0	0			
Queue Length 50th (ft)	3	38			70			11	0			

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Lanes, Volumes, Timings 37: N Main St & W Washington St/E Washington St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (ft)	10	72			m17			m33	m9			
Internal Link Dist (ft)		174			182			246			226	
Turn Bay Length (ft)	65											
Base Capacity (vph)	353	869			860			778	639			
Starvation Cap Reductn	0	71			22			0	0			
Spillback Cap Reductn	0	3			0			0	0			
Storage Cap Reductn	0	0			0			0	0			
Reduced v/c Ratio	0.05	0.34			0.44			0.05	0.03			
Intersection Summary												
Area Type:	CBD											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 78 (98%), Reference	ed to phase	2:NBTL a	ind 6:, Sta	art of Gre	en							
Natural Cycle: 50												
Control Type: Actuated-Cod	ordinated											
Maximum v/c Ratio: 0.74												
Intersection Signal Delay: 1	8.1			In	tersectior	LOS: B						
Intersection Capacity Utiliza	Intersection Capacity Utilization 53.3% ICU Level of Service A											
Analysis Period (min) 15												
m Volume for 95th percer	ntile queue is	s metered	l by upstr	eam sign	al.							
			_									

Splits and Phases: 37: N Main St & W Washington St/E Washington St

1 Ø2 (R)		
28 s	52 s	
	←	
	52 s	

02/22/2024

Lanes, Volumes, Timings 38: W Front St/E Front St & N Main St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	ţ,			\$			\$				
Traffic Volume (vph)	23	87	0	0	119	23	0	0	0	0	0	0
Future Volume (vph)	23	87	0	0	119	23	0	0	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	8	12	12	12	12
Storage Length (ft)	70		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.978							
Flt Protected	0.950											
Satd. Flow (prot)	1504	1425	0	0	1394	0	0	1308	0	0	0	0
Flt Permitted	0.950											
Satd. Flow (perm)	1504	1425	0	0	1394	0	0	1308	0	0	0	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		262			256			210			326	
Travel Time (s)		6.0			5.8			4.8			7.4	
Confl. Peds. (#/hr)	13		7	7		13	4		32			
Peak Hour Factor	0.82	0.82	0.92	0.92	0.82	0.82	0.92	0.92	0.92	0.82	0.92	0.82
Heavy Vehicles (%)	8%	8%	2%	2%	8%	8%	2%	2%	2%	8%	2%	8%
Parking (#/hr)		0			0			0				
Adj. Flow (vph)	28	106	0	0	145	28	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	28	106	0	0	173	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.14	1.30	1.14	1.14	1.30	1.14	1.14	1.55	1.14	1.14	1.14	1.14
Turning Speed (mph)	15		60	60		9	60		60	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type: 0	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilizati	ion 37.0%			IC	CU Level o	of Service	А					
Analysis Period (min) 15												

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations		1	† 1>					
Traffic Volume (vph)	0	29	1224	7	0	0		
Future Volume (vph)	0	29	1224	7	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Lane Width (ft)	12	12	11	11	12	12		
Grade (%)	0%		-2%			2%		
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00		
Frt		0.865	0.999					
Flt Protected								
Satd. Flow (prot)	0	1305	3107	0	0	0		
Flt Permitted								
Satd. Flow (perm)	0	1305	3107	0	0	0		
Link Speed (mph)	30		30			30		
Link Distance (ft)	558		266			127		
Travel Time (s)	12.7		6.0			2.9		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94		
Parking (#/hr)		0						
Adj. Flow (vph)	0	31	1302	7	0	0		
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	31	1309	0	0	0		
Enter Blocked Intersection	No	No	No	No	No	No		
Lane Alignment	Left	Right	Left	Right	Left	Left		
Median Width(ft)	0		0			0		
Link Offset(ft)	0		0			0		
Crosswalk Width(ft)	16		16			16		
Two way Left Turn Lane								
Headway Factor	1.14	1.30	1.18	1.18	1.16	1.16		
Turning Speed (mph)	15	9		9	15			
Sign Control	Stop		Free			Stop		
Intersection Summary								
Area Type: 0	CBD							
Control Type: Unsignalized								
Intersection Capacity Utilizat	ion 47.8%			IC	U Level o	of Service	γA	
Analysis Period (min) 15								

Lanes, Volumes, Timings 47: S Center St & W Olive St

02/22/2024	02	122	2	024
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		*	1	7	•						-fî†	1
Traffic Volume (vph)	0	89	48	10	32	0	0	0	0	54	1145	51
Future Volume (vph)	0	89	48	10	32	0	0	0	0	54	1145	51
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	11	10	12	12	12	12	12	12	12	12	12
Grade (%)		1%			-3%			3%			-1%	
Storage Length (ft)	0		80	0		0	0		0	0		50
Storage Lanes	0		1	1		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00
Frt			0.850									0.850
Flt Protected				0.950							0.998	
Satd. Flow (prot)	0	1792	1470	1796	1891	0	0	0	0	0	3550	1591
Flt Permitted				0.694							0.998	
Satd. Flow (perm)	0	1792	1470	1312	1891	0	0	0	0	0	3550	1591
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			52									36
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		777			330			770			660	
Travel Time (s)		17.7			7.5			17.5			15.0	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adi, Flow (vph)	0	97	52	11	35	0	0	0	0	59	1245	55
Shared Lane Traffic (%)	-		-			-					-	
Lane Group Flow (vph)	0	97	52	11	35	0	0	0	0	0	1304	55
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12	J -		12	J •		0	J -		0	5
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.01	1.05	1.10	0.98	0.98	0.98	1.02	1.02	1.02	0.99	0.99	0.99
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors		2	1	1	2					1	2	1
Detector Template		Thru	Right	Left	Thru					Left	Thru	Right
Leading Detector (ft)		100	20	20	100					20	100	20
Trailing Detector (ft)		0	0	0	0					0	0	0
Detector 1 Position(ft)		0	0	0	0					0	0	0
Detector 1 Size(ft)		6	20	20	6					20	6	20
Detector 1 Type		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex					Cl+Ex	Cl+Ex	CI+Ex
Detector 1 Channel												-
Detector 1 Extend (s)		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 1 Queue (s)		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 1 Delay (s)		0.0	0.0	0.0	0.0					0.0	0.0	0.0
Detector 2 Position(ft)		94			94						94	
Detector 2 Size(ft)		6			6						6	
Detector 2 Type		Cl+Fx			Cl+Ex						Cl+Fx	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA	Perm	Perm	NA					Perm	NA	Perm
типттуре					IN/A							

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Lanes, Volumes, Timings 47: S Center St & W Olive St

02/22/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases		4			8						6	
Permitted Phases			4	8						6		6
Detector Phase		4	4	8	8					6	6	6
Switch Phase												
Minimum Initial (s)		5.0	5.0	5.0	5.0					5.0	5.0	5.0
Minimum Split (s)		13.5	13.5	13.5	13.5					37.9	37.9	37.9
Total Split (s)		18.0	18.0	18.0	18.0					62.0	62.0	62.0
Total Split (%)		22.5%	22.5%	22.5%	22.5%					77.5%	77.5%	77.5%
Maximum Green (s)		12.5	12.5	12.5	12.5					56.1	56.1	56.1
Yellow Time (s)		3.2	3.2	3.2	3.2					3.2	3.2	3.2
All-Red Time (s)		2.3	2.3	2.3	2.3					2.7	2.7	2.7
Lost Time Adjust (s)		0.0	0.0	0.0	0.0						0.0	0.0
Total Lost Time (s)		5.5	5.5	5.5	5.5						5.9	5.9
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)		3.0	3.0	3.0	3.0					3.0	3.0	3.0
Recall Mode		Max	Max	None	None					C-Max	C-Max	C-Max
Walk Time (s)		1.0	1.0	1.0	1.0					21.0	21.0	21.0
Flash Dont Walk (s)		7.0	7.0	7.0	7.0					11.0	11.0	11.0
Pedestrian Calls (#/hr)		0	0	0	0					0	0	0
Act Effct Green (s)		12.5	12.5	12.5	12.5						56.1	56.1
Actuated g/C Ratio		0.16	0.16	0.16	0.16						0.70	0.70
v/c Ratio		0.35	0.19	0.05	0.12						0.52	0.05
Control Delay		34.1	11.1	32.7	32.3						3.1	0.6
Queue Delay		0.0	0.0	0.0	0.0						0.0	0.0
Total Delay		34.1	11.1	32.7	32.3						3.1	0.6
LOS		С	В	С	С						А	A
Approach Delay		26.1			32.4						3.0	
Approach LOS		С			С						А	
90th %ile Green (s)		12.5	12.5	12.5	12.5					56.1	56.1	56.1
90th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
70th %ile Green (s)		12.5	12.5	12.5	12.5					56.1	56.1	56.1
70th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
50th %ile Green (s)		12.5	12.5	12.5	12.5					56.1	56.1	56.1
50th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
30th %ile Green (s)		12.5	12.5	12.5	12.5					56.1	56.1	56.1
30th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
10th %ile Green (s)		12.5	12.5	12.5	12.5					56.1	56.1	56.1
10th %ile Term Code		MaxR	MaxR	Hold	Hold					Coord	Coord	Coord
Stops (vph)		78	14	12	31						213	3
Fuel Used(gal)		2	0	0	0						8	0
CO Emissions (g/hr)		111	33	11	32						567	20
NOx Emissions (g/hr)		22	6	2	6						110	4
VOC Emissions (g/hr)		26	8	3	8						131	5
Dilemma Vehicles (#)		0	0	0	0						0	0
Queue Length 50th (ft)		44	0	5	16						73	1
Queue Length 95th (ft)		88	30	m17	m41						78	m0
Internal Link Dist (ft)		697			250			690			580	
Turn Bay Length (ft)			80									50

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Lanes, Volumes, Timings 47: S Center St & W Olive St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Base Capacity (vph)		280	273	205	295						2489	1126
Starvation Cap Reductn		0	0	0	0						0	0
Spillback Cap Reductn		0	0	0	0						0	0
Storage Cap Reductn		0	0	0	0						0	0
Reduced v/c Ratio		0.35	0.19	0.05	0.12						0.52	0.05
Intersection Summary												
Area Type:	Other											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 16 (20%), Reference	d to phase	2: and 6:	SBTL, Sta	art of Gre	en							
Natural Cycle: 55												
Control Type: Actuated-Coo	rdinated											
Maximum v/c Ratio: 0.52												
Intersection Signal Delay: 6.	1			In	tersectior	n LOS: A						
Intersection Capacity Utilizat	tion 55.6%			IC	U Level o	of Service	В					
Analysis Period (min) 15												
m Volume for 95th percent	tile queue i	s metered	l by upstr	eam sign	al.							
Splits and Phases: 47: S	Center St 8	W Olive	St									
									1	2		35



Lanes, Volumes, Timings 48: S East St & E Olive St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ŧ			≜ 1≽			đ î ja				
Traffic Volume (vph)	87	67	0	0	39	30	18	917	16	0	0	0
Future Volume (vph)	87	67	0	0	39	30	18	917	16	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	12	12	12	12	12	12	12	12	12
Grade (%)		2%			-4%			4%			-4%	
Storage Length (ft)	0		0	0		0	300		0	0		0
Storage Lanes	0		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00
Frt					0.934			0.998				
Flt Protected		0.973						0.999				
Satd. Flow (prot)	0	1673	0	0	3275	0	0	3359	0	0	0	0
Flt Permitted		0.781						0.999				
Satd. Flow (perm)	0	1343	0	0	3275	0	0	3359	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					33			4				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		330			647			682			633	
Travel Time (s)		7.5			14.7			15.5			14.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%	5%
Parking (#/hr)		0										
Adj. Flow (vph)	95	73	0	0	42	33	20	997	17	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	168	0	0	75	0	0	1034	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.01	1.07	1.01	0.97	0.97	0.97	1.03	1.03	1.03	0.97	0.97	0.97
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors	1	2			2		1	2				
Detector Template	Left	Thru			Thru		Left	Thru				
Leading Detector (ft)	20	100			100		20	100				
Trailing Detector (ft)	0	0			0		0	0				
Detector 1 Position(ft)	0	0			0		0	0				
Detector 1 Size(ft)	20	6			6		20	6				
Detector 1 Type	Cl+Ex	Cl+Ex			CI+Ex		CI+Ex	CI+Ex				
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0				
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0				
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex				
Detector 2 Channel												

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Lanes, Volumes, Timings 48: S East St & E Olive St

02/22/	2024
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA			NA		Perm	NA				
Protected Phases		4			8			2				
Permitted Phases	4				-		2					
Detector Phase	4	4			8		2	2				
Switch Phase	•	•			•		_	_				
Minimum Initial (s)	50	50			50		50	50				
Minimum Split (s)	19.5	19.5			19.5		58.6	58.6				
Total Split (s)	25.0	25.0			25.0		55.0	55.0				
Total Split (%)	31.3%	31.3%			31.3%		68.8%	68.8%				
Maximum Green (s)	19.5	19.5			19.5		49.4	49 A				
Vellow Time (s)	3.0	3.0			3.0		3.7	3.7				
All Ped Time (s)	0.Z	2.2			2.2		0.Z	2.4				
Lost Time Adjust (s)	2.0	2.5			2.5		2.4	2.4				
Total Lost Time (s)		0.0			5.5			5.6				
		0.0			0.0			5.0				
Leau/Lay												
	3.0	3.0			3.0		3.0	3.0				
Pocall Mode	J.U May	J.U Max			J.U Nono		C Max	C Max				
	2.0	2.0			2.0		0-IVIAX	12 0				
Valk Tille (S)	3.0 11.0	11.0			3.0		42.0	42.0				
Flash Done walk (S)	0	11.0			11.0		11.0	11.0				
Act Effet Croop (a)	U	10 5			10.5		0	40.4				
Act Elici Green (S)		19.5			19.0			49.4				
Actualed g/C Ratio		0.24			0.24			0.62				
V/C Ratio		0.51			0.09			0.50				
Control Delay		31.0			15.4			9.4				
		0.0			0.0			0.0				
l otal Delay		31.0			15.4			9.4				
LUS		U 01.0			B			A				
Approach Delay		31.0			15.4			9.4				
Approach LOS	40 5	C 40 5			B		40.4	A				
90th %ile Green (s)	19.5	19.5			19.5		49.4	49.4				
90th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
70th %ile Green (s)	19.5	19.5			19.5		49.4	49.4				
70th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
50th %ile Green (s)	19.5	19.5			19.5		49.4	49.4				
50th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
30th %ile Green (s)	19.5	19.5			19.5		49.4	49.4				
30th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
10th %ile Green (s)	19.5	19.5			19.5		49.4	49.4				
10th %ile Term Code	MaxR	MaxR			Hold		Coord	Coord				
Stops (vph)		142			33			483				
Fuel Used(gal)		2			1			10				
CO Emissions (g/hr)		151			52			668				
NOx Emissions (g/hr)		29			10			130				
VOC Emissions (g/hr)		35			12			155				
Dilemma Vehicles (#)		0			0			0				
Queue Length 50th (ft)		81			8			133				
Queue Length 95th (ft)		144			25			178				

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Lanes, Volumes, Timings 48: S East St & E Olive St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		250			567			602			553	
Turn Bay Length (ft)												
Base Capacity (vph)		327			823			2075				
Starvation Cap Reductn		0			0			0				
Spillback Cap Reductn		0			0			0				
Storage Cap Reductn		0			0			0				
Reduced v/c Ratio		0.51			0.09			0.50				
Intersection Summary												
Area Type:	Other											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 54 (68%), Reference	ed to phase 2	2:NBTL a	nd 6:, Sta	art of Gre	en							
Natural Cycle: 80												
Control Type: Actuated-Coo	ordinated											
Maximum v/c Ratio: 0.51												
Intersection Signal Delay: 1	2.6			In	tersectior	n LOS: B						
Intersection Capacity Utilization	ation 50.6%			IC	U Level o	of Service	А					
Analysis Period (min) 15												
Splits and Phases: 48: S	East St & E	Olive St						_				

Ø2 (R)	<u> ≁</u> _{Ø4}
55 s	25 s
	← Ø8
	25 s

02/22/2024